

STORMWATER CALCULATIONS

TOWN OF ZEBULON
WAKE COUNTY, NC



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The project area of 1.38 ac (60,200 ft²) is an undeveloped, grassed, and wooded lot. The development proposes building 12 residential lots. This will also include the necessary infrastructure (i.e., roadways, sidewalks, and public utilities) needed to facilitate the development.

The Town of Zebulon requires new residential development to meet NCDEQ stormwater regulations and requires that “Structural and non-structural BMPs shall be used to ensure there is no net increase in peak flow leaving the site from the pre-development conditions for the one-year, 24-hour storm”. A stormwater wet pond, SCM #1, has been proposed to meet the requirements.

The proposed disturbed area for this project is 45,981 ft² (1.06 ac) with a total of 21,121 ft² (0.48 ac) of impervious areas. 46,412 ft² of the site, including all the impervious areas, is being captured by SCM #1. Pre onsite drainage goes to POI#1 while post onsite drainage goes to both POI#1 and POI#2.

The simple method and the SA/DA method were used for sizing the pond. AutoCAD Civil 3D and Storm Water Management Model (SWMM) were used for the design and routing of the pond.

For stormwater control measure sizing, we were more conservative in a way that instead of 35%, we assumed there is 50% of impervious area after site development. One reason to make this conservative approach is to consider 70% of each residential as the impervious area (rooftop and driveway).

Table 1 – SCM #1 Volumes

| | Required | Provided |
|-----------------------|----------|----------|
| Water Quality Volume | 2,539 cf | 3,183 cf |
| Permanent Pool Volume | 3,269 cf | 3,309 cf |
| Forebay Volume | 654 cf | 678 cf |

Summary

The post development discharge from the site at POI #1 and #2 for 1-year & 24-hour storm event is 17% less than the pre-development discharge at POI #1 meeting the requirements set for by both the Town of Zebulon and NCDEQ. The emergency spillway is not triggered during the 1-year& 24-hour storm event. Phosphorus and nitrogen have been reduced for this site.

Stormwater Control Measure Sizing

Project: True Homes - Zebulon - 712 Arendell
 SCM SCM 1 (Wet Pond)
 Date: 9/23/2021
 Rev.:
 Rev.2:

DETERMINE SCM TYPE AND DIMENSIONS

Simple Method: Pond Runoff Volume

Simple Method - $V = 3630 * R_v * R_d * A$

Rd - Rainfall Depth (inches) =

| |
|---|
| 1 |
|---|

$R_v = 0.05 + 0.9 * I_a$

Rv = Runoff Coefficient

$I_a = \text{Impervious fraction} = \text{Impervious drainage area} / \text{Total drainage area}$

A = Drainage area (ac.) =

| | |
|------|----|
| 1.38 | ac |
| 0.70 | ac |

Impervious portion of drainage area (ac.) =

$I_a =$

| |
|-------|
| 0.507 |
|-------|

$R_v = 0.05 + 0.9 * I_a$

Rv =

| |
|-------|
| 0.506 |
|-------|

Water Quality Volume Required:

$WQV = DV = 3630 * R_v * R_d * A =$

| | |
|-------|----|
| 2,539 | cf |
| 3,183 | cf |

Water Quality Volume Provided:

MDC WET POND Sizing:

Option 1: HRT Method (Main Pool Hydraulic Retention Time)

$V_{mp} = 0.87 * HRT / T_s * DV$

Vmp

Volume of main pool

HRT

Hydraulic Residence Time (14 days)

Ts

Time between storm events (5 days)

DV

Design Volume (cf)

Design Volume from simple method

$V_{mp} = 0.87 * (HRT / T_s) * WQV$

| | | |
|-------|-------|----|
| WQV = | 2,539 | cf |
| Vmp = | 6,185 | cf |

Determine Min. SA of Main Pool:

$D_{avg} = V_{pp} / SA = V_{mp} / A_{mp}$

Amp =

V_{mp} / D_{avg}

| | | |
|--------------------|-------|----|
| Amp = | 2,062 | sf |
| Plus 20% Forebay = | 412 | sf |
| App = | 2,474 | sf |

Choose Depth =

| | |
|---|----|
| 3 | ft |
|---|----|

| |
|--|
| $D_{avg} = \frac{V_{PP}}{SA}$ |
| Where: D_{avg} = Average depth (feet) |
| V_{PP} = Main pool volume at permanent pool elevation (feet ³) |
| SA = Main pool area at permanent pool elevation (feet ²) |

(Per MDC, when calculating the average depth, only the main pool is considered. The forebay is excluded from the calculation.)

Option 2: SA/DA and Average Depth Method

Impervious Ratio =

| | |
|-------|--|
| 0.507 | |
|-------|--|

 Choose Permanent Pool Avg Depth, Davg =

| | |
|---|----|
| 3 | ft |
|---|----|

From MDC 1, Table 1: Piedmont and Mountain SA/DA Table
 SA/DA (%) =

| | |
|------|---|
| 1.81 | % |
|------|---|

 DA (acres) =

| | |
|------|----|
| 1.38 | ac |
|------|----|

Therefore, SA = DA * coefficient
 SA =

| | |
|-------|----|
| 0.025 | ac |
|-------|----|

 SA (main pool) =

| | |
|-------|----|
| 1,090 | sf |
|-------|----|

 Plus 20% Forebay =

| | |
|-----|----|
| 218 | sf |
|-----|----|

 SA (total) =

| | |
|-------|----|
| 1,308 | sf |
|-------|----|

Find Volume of Permanent Pool
 Davg = Vpp / SA = Vmp / Amp
 Vmp = Davg * Amp
 Vmp =

| | |
|-------|----|
| 3,269 | cf |
|-------|----|

 Plus 20% Forebay =

| | |
|-----|----|
| 654 | cf |
|-----|----|

 Vpp =

| | |
|-------|----|
| 3,923 | cf |
|-------|----|

SUMMARY: Determine SCM Type and Volume/Area/Depth

| | | | | | | |
|--------------------------|------------|-------|------------|-------|--------|---|
| Wet Ponc HRT Method 1: | Vpp (cf) = | 6,185 | App (sf) = | 2,474 | Davg = | 3 |
| Wet Ponc SA/DA Method 2: | Vpp (cf) = | 3,923 | App (sf) = | 1,308 | Davg = | 3 |



Table 1: Piedmont and Mountain SA/DA Table (Adapted from Driscoll, 1986)

| Percent Impervious Cover | Permanent Pool Average Depth (ft) | | | | | |
|--------------------------|-----------------------------------|------|------|------|------|------|
| | 3.0 | 4.0 | 5.0 | 6.0 | 7.0 | 8.0 |
| 10% | 0.51 | 0.43 | 0.37 | 0.30 | 0.27 | 0.25 |
| 20% | 0.84 | 0.69 | 0.61 | 0.51 | 0.44 | 0.40 |
| 30% | 1.17 | 0.81 | 0.84 | 0.72 | 0.61 | 0.56 |
| 40% | 1.51 | 1.22 | 1.09 | 0.91 | 0.78 | 0.71 |
| 50% | 1.79 | 1.47 | 1.31 | 1.13 | 0.95 | 0.87 |
| 60% | 2.09 | 1.73 | 1.49 | 1.31 | 1.12 | 1.03 |
| 70% | 2.51 | 2.04 | 1.80 | 1.56 | 1.34 | 1.17 |
| 80% | 2.92 | 2.36 | 2.07 | 1.82 | 1.62 | 1.40 |
| 90% | 3.25 | 2.64 | 2.31 | 2.04 | 1.84 | 1.59 |

Desired Depth =

| |
|---|
| 3 |
|---|

 ft

Lower Limit =

| | |
|------|-------|
| Imp. | Ratio |
| 0.5 | 1.79 |

 Upper Limit =

| | |
|-----|------|
| 0.6 | 2.09 |
|-----|------|

Actual Surface Area Ratio = **1.81**

| Temporary pool | |
|--|------|
| depth (From BMP Orifice Sizing excel file) | 1 ft |
| Main pool | |
| depth (From SCM Sizing Calcs-WP excel file) | 3 ft |

| Elevation | |
|--|-----------|
| Ground elv | 311.9 ft |
| Distance to pond | 130 ft |
| slope | 0.5% |
| dif elv | 0.65 ft |
| SSMN | |
| Rim | 313 ft |
| Inv in | 309.22 ft |
| Inv out | 309.03 ft |
| Forebay | |
| main pool elv | 310 ft |
| Depth | 3.0 ft |
| Bottem elv | 307.0 ft |
| Sediment storage in Forebay | |
| Sediment storage depth | 12 inch |
| Sediment storage bottom elv (Entrance) | 306 ft |
| Sediment storage bottom elv (Exit) | 306.5 ft |
| Main pool | |
| main pool elv | 310 ft |
| Depth | 3 ft |
| bottom elv | 307 ft |
| Sediment storage depth | 6 inch |
| sediment storage bottom elv | 306.5 ft |

| Forebay | | | | | | | |
|---------------------------|------------|-----------|-------|-----------|--------|-----------------|------|
| Designed forebay (SCM #1) | | | | | | Design Criteria | PASS |
| | Area (ft2) | Area (ac) | depth | elevation | Volume | volume | |
| Bottom | 73 | 0.0017 | 3 | 307 | 678 | 654 | |
| Top | 429 | 0.0098 | | 310 | | | |

| Main Pool | | | | | | | |
|-----------------------------|------------|-----------|-------|-----------|--------|-----------------|------|
| Designed main pool (SCM #1) | | | | | | Design Criteria | PASS |
| | Area (ft2) | Area (ac) | depth | elevation | Volume | Volume | |
| Bottom | 436 | 0.0100 | 3 | 307 | 3309 | 3,269 | |
| Top | 1951 | 0.0448 | | 310 | | | |

| Temporary pool | | | | | | | |
|----------------------------------|------------|-----------|-------|-----------|--------|-----------------|------|
| Designed temporary pool (SCM #1) | | | | | | Design Criteria | PASS |
| | Area (ft2) | Area (ac) | depth | elevation | Volume | Volume | |
| Bottom | 2380 | 0.0546 | 1 | 310 | 3183 | 2,539 | |
| Top | 4061 | 0.0932 | | 311 | | | |

SCM Orifice Sizing

Orifice Sizing - BMP A WETPOND

$$Q = C_d \times A \times \text{sqrt}(2gh)$$

Total Volume to be discharged from Ponds (Q) = 2,539 cf
Depth of Storage (H) = 1.0

2 days to Discharge - 172,800 seconds

$$Q_2 = 0.014693 \text{ cfs}$$

$$A_2 = \frac{Q_2}{C_d \times \text{sqrt}(2gh)}$$

$$C_d = 0.60$$

$$g = 32.2$$

$$H/3 = h = 0.33$$

$$A_2 = 0.0053 \text{ sf} \quad A = \pi R^2 = \pi D^2/4$$
$$0.761 \text{ sq in}$$

$$\text{diameter } \mathbf{0.984} \text{ inches} \quad D = \text{sqrt}(4A/\pi)$$

5 Days to Discharge - 432,000 seconds

$$Q_5 = 0.005877 \text{ cfs}$$

$$A_5 = \frac{Q_5}{C_d \times \text{sqrt}(2gh)}$$

$$C_d = 0.60$$

$$g = 32.2$$

$$H/3 = h = 0.33$$

$$A_5 = 0.002 \text{ sf}$$
$$0.304 \text{ sq in}$$

$$\text{diameter } \mathbf{0.623} \text{ inches}$$

Orifice Chosen

$$\text{diameter} = \mathbf{0.90} \text{ inches}$$

$$A = 0.0044 \text{ sf}$$

$$Q = C_d \times A \times \text{sqrt}(2 \times g \times h)$$

$$Q = 0.0123 \text{ cfs}$$

$$\text{time} = 206736.30 \text{ seconds}$$

$$\text{Time} = \mathbf{2.39} \text{ days}$$

NOAA Rainfall Data



NOAA Atlas 14, Volume 2, Version 3
Location name: Zebulon, North Carolina, USA*
Latitude: 35.8281°, Longitude: -78.3171°
Elevation: 312.79 ft**



* source: ESRI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

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PF tabular

| PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹ | | | | | | | | | | |
|--|-------------------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|-----------------------|
| Duration | Average recurrence interval (years) | | | | | | | | | |
| | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | 0.405 (0.369-0.445) | 0.468 (0.428-0.513) | 0.531 (0.486-0.581) | 0.601 (0.549-0.658) | 0.671 (0.609-0.733) | 0.729 (0.659-0.796) | 0.780 (0.701-0.852) | 0.828 (0.740-0.905) | 0.881 (0.781-0.964) | 0.932 (0.819-1.02) |
| 10-min | 0.647 (0.590-0.710) | 0.749 (0.685-0.820) | 0.851 (0.778-0.930) | 0.962 (0.877-1.05) | 1.07 (0.971-1.17) | 1.16 (1.05-1.27) | 1.24 (1.12-1.35) | 1.31 (1.17-1.43) | 1.39 (1.24-1.53) | 1.47 (1.29-1.61) |
| 15-min | 0.808 (0.738-0.888) | 0.941 (0.861-1.03) | 1.08 (0.984-1.18) | 1.22 (1.11-1.33) | 1.36 (1.23-1.48) | 1.47 (1.33-1.61) | 1.57 (1.41-1.71) | 1.66 (1.48-1.81) | 1.75 (1.56-1.92) | 1.84 (1.62-2.02) |
| 30-min | 1.11 (1.01-1.22) | 1.30 (1.19-1.42) | 1.53 (1.40-1.67) | 1.76 (1.61-1.93) | 2.01 (1.82-2.19) | 2.21 (2.00-2.42) | 2.40 (2.16-2.62) | 2.58 (2.30-2.82) | 2.79 (2.48-3.06) | 2.98 (2.62-3.27) |
| 60-min | 1.38 (1.26-1.52) | 1.63 (1.49-1.79) | 1.96 (1.79-2.14) | 2.30 (2.09-2.51) | 2.67 (2.43-2.92) | 3.00 (2.71-3.28) | 3.31 (2.97-3.61) | 3.62 (3.23-3.95) | 4.00 (3.55-4.38) | 4.36 (3.83-4.78) |
| 2-hr | 1.62 (1.46-1.79) | 1.91 (1.74-2.10) | 2.33 (2.12-2.56) | 2.77 (2.50-3.04) | 3.28 (2.95-3.59) | 3.75 (3.36-4.10) | 4.20 (3.74-4.60) | 4.67 (4.13-5.12) | 5.30 (4.64-5.80) | 5.88 (5.10-6.45) |
| 3-hr | 1.71 (1.55-1.90) | 2.03 (1.85-2.25) | 2.48 (2.25-2.74) | 2.96 (2.68-3.27) | 3.55 (3.19-3.90) | 4.09 (3.66-4.50) | 4.63 (4.11-5.09) | 5.21 (4.58-5.72) | 5.99 (5.21-6.58) | 6.72 (5.79-7.41) |
| 6-hr | 2.05 (1.87-2.27) | 2.43 (2.22-2.68) | 2.97 (2.70-3.27) | 3.56 (3.23-3.91) | 4.28 (3.86-4.69) | 4.95 (4.43-5.42) | 5.63 (5.00-6.15) | 6.35 (5.59-6.94) | 7.35 (6.38-8.03) | 8.30 (7.11-9.09) |
| 12-hr | 2.42 (2.20-2.66) | 2.87 (2.62-3.15) | 3.52 (3.21-3.86) | 4.23 (3.85-4.64) | 5.12 (4.63-5.60) | 5.97 (5.35-6.51) | 6.83 (6.06-7.44) | 7.77 (6.81-8.46) | 9.07 (7.83-9.88) | 10.3 (8.78-11.3) |
| 24-hr | 2.86 (2.65-3.09) | 3.46 (3.21-3.74) | 4.39 (4.07-4.75) | 5.14 (4.76-5.56) | 6.21 (5.71-6.71) | 7.08 (6.49-7.65) | 8.01 (7.30-8.66) | 9.01 (8.15-9.75) | 10.4 (9.35-11.3) | 11.6 (10.3-12.6) |
| 2-day | 3.31 (3.07-3.57) | 3.99 (3.71-4.30) | 5.02 (4.66-5.42) | 5.86 (5.42-6.32) | 7.03 (6.48-7.59) | 7.99 (7.33-8.62) | 9.00 (8.22-9.73) | 10.1 (9.15-10.9) | 11.6 (10.4-12.6) | 12.9 (11.5-14.1) |
| 3-day | 3.51 (3.27-3.78) | 4.22 (3.94-4.55) | 5.29 (4.92-5.69) | 6.15 (5.71-6.61) | 7.36 (6.80-7.91) | 8.34 (7.67-8.97) | 9.37 (8.58-10.1) | 10.5 (9.52-11.3) | 12.0 (10.8-13.0) | 13.3 (11.9-14.4) |
| 4-day | 3.71 (3.47-3.98) | 4.46 (4.16-4.79) | 5.56 (5.19-5.96) | 6.44 (6.00-6.90) | 7.68 (7.11-8.23) | 8.69 (8.01-9.32) | 9.74 (8.94-10.5) | 10.9 (9.90-11.7) | 12.4 (11.2-13.4) | 13.7 (12.3-14.8) |
| 7-day | 4.32 (4.03-4.62) | 5.16 (4.83-5.53) | 6.36 (5.94-6.80) | 7.32 (6.82-7.83) | 8.65 (8.03-9.26) | 9.72 (9.00-10.4) | 10.8 (9.99-11.6) | 12.0 (11.0-12.9) | 13.7 (12.4-14.7) | 15.0 (13.5-16.2) |
| 10-day | 4.93 (4.62-5.25) | 5.87 (5.50-6.26) | 7.13 (6.68-7.60) | 8.12 (7.59-8.65) | 9.48 (8.84-10.1) | 10.6 (9.82-11.3) | 11.7 (10.8-12.5) | 12.8 (11.8-13.8) | 14.5 (13.2-15.5) | 15.7 (14.3-16.9) |
| 20-day | 6.61 (6.22-7.03) | 7.83 (7.36-8.33) | 9.34 (8.78-9.94) | 10.5 (9.89-11.2) | 12.2 (11.4-13.0) | 13.5 (12.6-14.4) | 14.8 (13.8-15.8) | 16.2 (15.0-17.3) | 18.1 (16.6-19.3) | 19.5 (17.9-21.0) |
| 30-day | 8.21 (7.74-8.71) | 9.68 (9.13-10.3) | 11.4 (10.7-12.1) | 12.7 (11.9-13.5) | 14.4 (13.5-15.3) | 15.8 (14.8-16.8) | 17.2 (16.0-18.2) | 18.5 (17.2-19.7) | 20.4 (18.9-21.8) | 21.8 (20.1-23.4) |
| 45-day | 10.4 (9.91-11.0) | 12.3 (11.6-12.9) | 14.2 (13.4-15.0) | 15.6 (14.8-16.5) | 17.6 (16.6-18.6) | 19.1 (18.0-20.1) | 20.5 (19.3-21.7) | 22.0 (20.6-23.3) | 24.0 (22.3-25.5) | 25.4 (23.6-27.1) |
| 60-day | 12.5 (11.9-13.2) | 14.7 (13.9-15.4) | 16.8 (15.9-17.6) | 18.4 (17.4-19.3) | 20.5 (19.4-21.6) | 22.1 (20.8-23.3) | 23.6 (22.2-24.9) | 25.1 (23.6-26.6) | 27.1 (25.4-28.8) | 28.6 (26.7-30.4) |

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical



NOAA Atlas 14, Volume 2, Version 3
Location name: Zebulon, North Carolina, USA*
Latitude: 35.8281°, Longitude: -78.3171°
Elevation: 312.79 ft**
 * source: ESRI Maps
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PF tabular

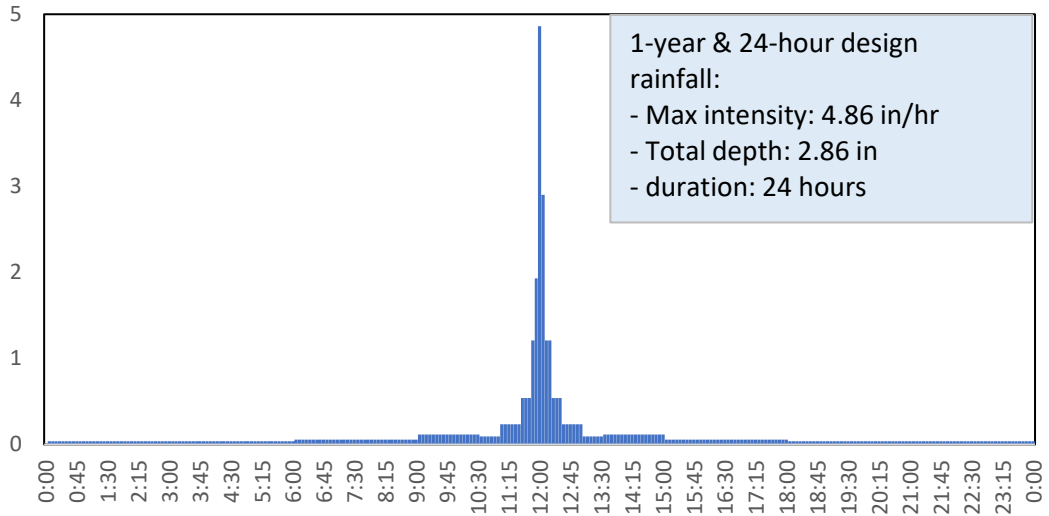
| PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)¹ | | | | | | | | | | |
|---|-------------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|
| Duration | Average recurrence interval (years) | | | | | | | | | |
| | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | 4.86 (4.43-5.34) | 5.62 (5.14-6.16) | 6.37 (5.83-6.97) | 7.21 (6.59-7.90) | 8.05 (7.31-8.80) | 8.75 (7.91-9.55) | 9.36 (8.41-10.2) | 9.94 (8.88-10.9) | 10.6 (9.37-11.6) | 11.2 (9.83-12.3) |
| 10-min | 3.88 (3.54-4.26) | 4.49 (4.11-4.92) | 5.11 (4.67-5.58) | 5.77 (5.26-6.31) | 6.41 (5.83-7.01) | 6.96 (6.30-7.61) | 7.44 (6.69-8.12) | 7.87 (7.03-8.60) | 8.36 (7.42-9.16) | 8.81 (7.74-9.66) |
| 15-min | 3.23 (2.95-3.55) | 3.76 (3.44-4.12) | 4.30 (3.94-4.71) | 4.86 (4.44-5.32) | 5.42 (4.92-5.92) | 5.88 (5.32-6.42) | 6.27 (5.64-6.84) | 6.62 (5.92-7.24) | 7.02 (6.22-7.68) | 7.37 (6.48-8.08) |
| 30-min | 2.22 (2.02-2.43) | 2.60 (2.38-2.85) | 3.06 (2.80-3.34) | 3.53 (3.22-3.86) | 4.01 (3.65-4.38) | 4.43 (4.00-4.84) | 4.80 (4.32-5.24) | 5.15 (4.61-5.63) | 5.58 (4.95-6.11) | 5.97 (5.24-6.54) |
| 60-min | 1.38 (1.26-1.52) | 1.63 (1.49-1.79) | 1.96 (1.79-2.14) | 2.30 (2.09-2.51) | 2.67 (2.43-2.92) | 3.00 (2.71-3.28) | 3.31 (2.97-3.61) | 3.62 (3.23-3.95) | 4.00 (3.55-4.38) | 4.36 (3.83-4.78) |
| 2-hr | 0.808 (0.732-0.895) | 0.957 (0.872-1.05) | 1.16 (1.06-1.28) | 1.38 (1.25-1.52) | 1.64 (1.48-1.80) | 1.87 (1.68-2.05) | 2.10 (1.87-2.30) | 2.34 (2.07-2.56) | 2.65 (2.32-2.90) | 2.94 (2.55-3.23) |
| 3-hr | 0.570 (0.517-0.634) | 0.676 (0.616-0.748) | 0.825 (0.749-0.912) | 0.987 (0.893-1.09) | 1.18 (1.06-1.30) | 1.36 (1.22-1.50) | 1.54 (1.37-1.70) | 1.73 (1.53-1.90) | 1.99 (1.74-2.19) | 2.24 (1.93-2.47) |
| 6-hr | 0.343 (0.312-0.379) | 0.406 (0.371-0.448) | 0.496 (0.452-0.547) | 0.594 (0.539-0.653) | 0.714 (0.644-0.783) | 0.826 (0.741-0.905) | 0.940 (0.834-1.03) | 1.06 (0.933-1.16) | 1.23 (1.07-1.34) | 1.39 (1.19-1.52) |
| 12-hr | 0.201 (0.183-0.221) | 0.238 (0.218-0.261) | 0.292 (0.267-0.321) | 0.351 (0.319-0.386) | 0.425 (0.384-0.465) | 0.495 (0.444-0.540) | 0.567 (0.503-0.618) | 0.645 (0.565-0.702) | 0.753 (0.650-0.820) | 0.857 (0.729-0.934) |
| 24-hr | 0.119 (0.111-0.129) | 0.144 (0.134-0.156) | 0.183 (0.170-0.198) | 0.214 (0.198-0.232) | 0.259 (0.238-0.279) | 0.295 (0.270-0.319) | 0.334 (0.304-0.361) | 0.375 (0.340-0.406) | 0.435 (0.390-0.472) | 0.484 (0.430-0.526) |
| 2-day | 0.069 (0.064-0.074) | 0.083 (0.077-0.090) | 0.105 (0.097-0.113) | 0.122 (0.113-0.132) | 0.146 (0.135-0.158) | 0.166 (0.153-0.180) | 0.188 (0.171-0.203) | 0.210 (0.191-0.227) | 0.242 (0.217-0.263) | 0.268 (0.239-0.293) |
| 3-day | 0.049 (0.045-0.052) | 0.059 (0.055-0.063) | 0.073 (0.068-0.079) | 0.085 (0.079-0.092) | 0.102 (0.094-0.110) | 0.116 (0.107-0.125) | 0.130 (0.119-0.140) | 0.145 (0.132-0.157) | 0.167 (0.150-0.181) | 0.185 (0.165-0.201) |
| 4-day | 0.039 (0.036-0.041) | 0.047 (0.043-0.050) | 0.058 (0.054-0.062) | 0.067 (0.062-0.072) | 0.080 (0.074-0.086) | 0.090 (0.083-0.097) | 0.101 (0.093-0.109) | 0.113 (0.103-0.122) | 0.129 (0.117-0.140) | 0.143 (0.128-0.154) |
| 7-day | 0.026 (0.024-0.027) | 0.031 (0.029-0.033) | 0.038 (0.035-0.040) | 0.044 (0.041-0.047) | 0.051 (0.048-0.055) | 0.058 (0.054-0.062) | 0.065 (0.059-0.069) | 0.072 (0.066-0.077) | 0.081 (0.074-0.088) | 0.089 (0.080-0.096) |
| 10-day | 0.021 (0.019-0.022) | 0.024 (0.023-0.026) | 0.030 (0.028-0.032) | 0.034 (0.032-0.036) | 0.040 (0.037-0.042) | 0.044 (0.041-0.047) | 0.049 (0.045-0.052) | 0.054 (0.049-0.057) | 0.060 (0.055-0.065) | 0.066 (0.060-0.071) |
| 20-day | 0.014 (0.013-0.015) | 0.016 (0.015-0.017) | 0.019 (0.018-0.021) | 0.022 (0.021-0.023) | 0.025 (0.024-0.027) | 0.028 (0.026-0.030) | 0.031 (0.029-0.033) | 0.034 (0.031-0.036) | 0.038 (0.035-0.040) | 0.041 (0.037-0.044) |
| 30-day | 0.011 (0.011-0.012) | 0.013 (0.013-0.014) | 0.016 (0.015-0.017) | 0.018 (0.017-0.019) | 0.020 (0.019-0.021) | 0.022 (0.021-0.023) | 0.024 (0.022-0.025) | 0.026 (0.024-0.027) | 0.028 (0.026-0.030) | 0.030 (0.028-0.032) |
| 45-day | 0.010 (0.009-0.010) | 0.011 (0.011-0.012) | 0.013 (0.012-0.014) | 0.014 (0.014-0.015) | 0.016 (0.015-0.017) | 0.018 (0.017-0.019) | 0.019 (0.018-0.020) | 0.020 (0.019-0.022) | 0.022 (0.021-0.024) | 0.024 (0.022-0.025) |
| 60-day | 0.009 (0.008-0.009) | 0.010 (0.010-0.011) | 0.012 (0.011-0.012) | 0.013 (0.012-0.013) | 0.014 (0.013-0.015) | 0.015 (0.014-0.016) | 0.016 (0.015-0.017) | 0.017 (0.016-0.018) | 0.019 (0.018-0.020) | 0.020 (0.019-0.021) |

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

Rainfall Intensity (in/hr)



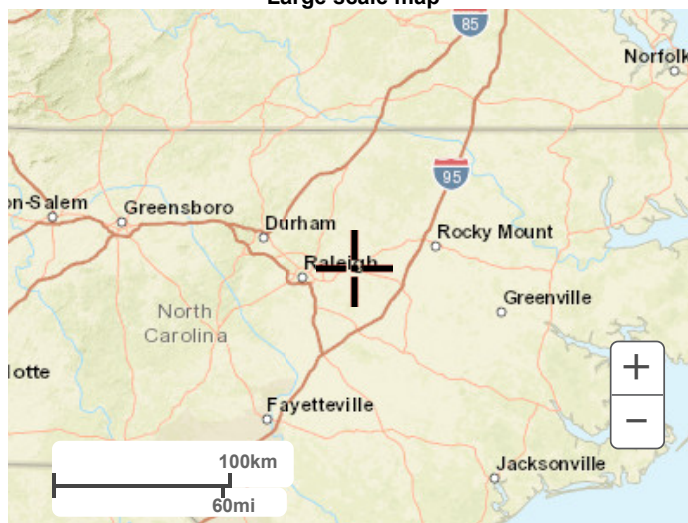
Drainage Area Maps



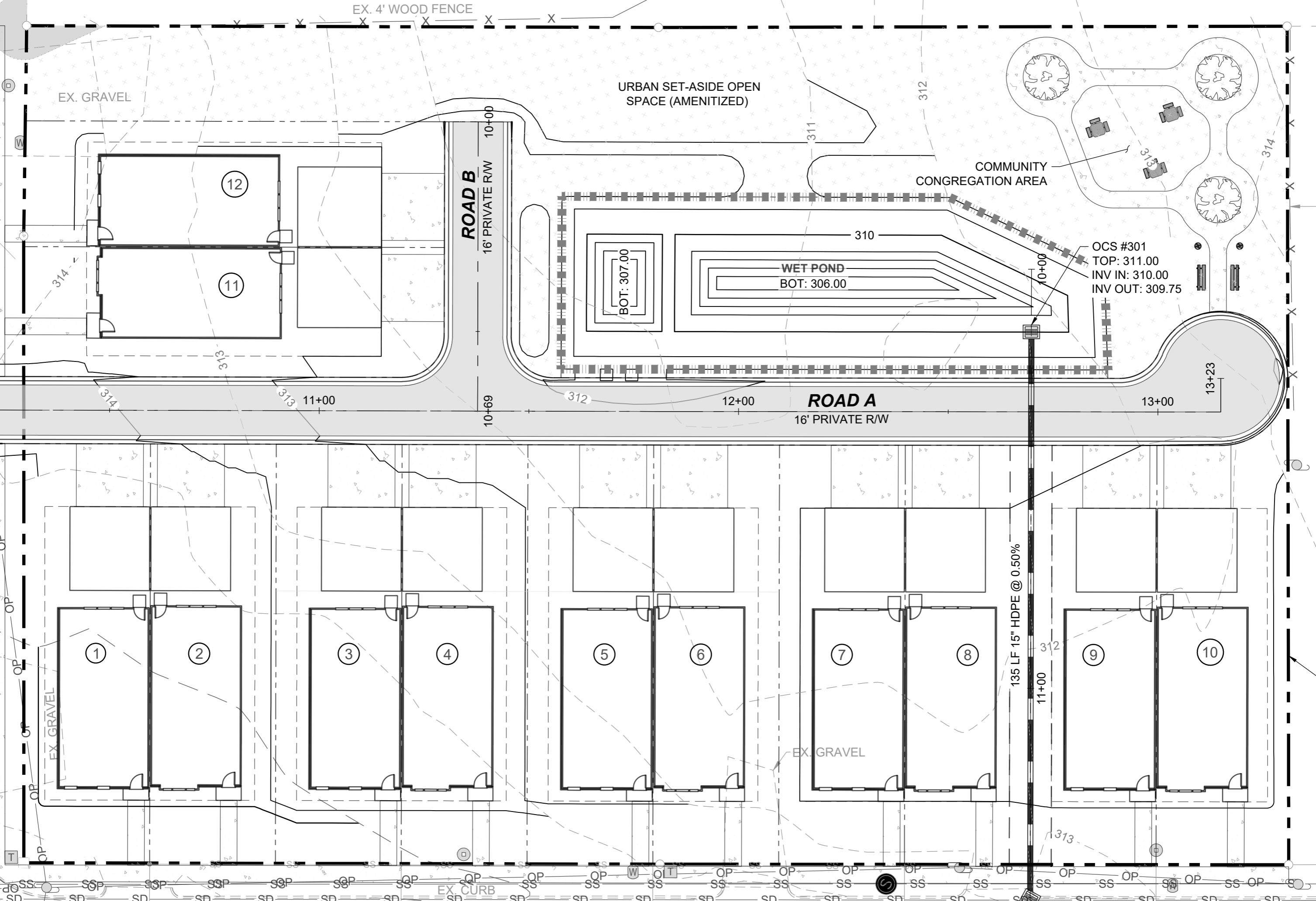
Large scale terrain



Large scale map



Large scale aerial



URBAN SET-ASIDE OPEN SPACE (AMENITIZED)

ROAD B
16' PRIVATE RW

ROAD A
16' PRIVATE RW

WET POND
BOT: 306.00
TOP: 307.00

COMMUNITY CONGREGATION AREA

OCS #301
TOP: 311.00
INV IN: 310.00
INV OUT: 309.75

EX. GRAVEL

EX. GRAVEL

EX. GRAVEL

EX. CURB

1

2

3

4

5

6

7

8

9

10

11

12

11+00

10+69

312

12+00

13+00

13+23

312

311

310

313

314

314

313

314

313

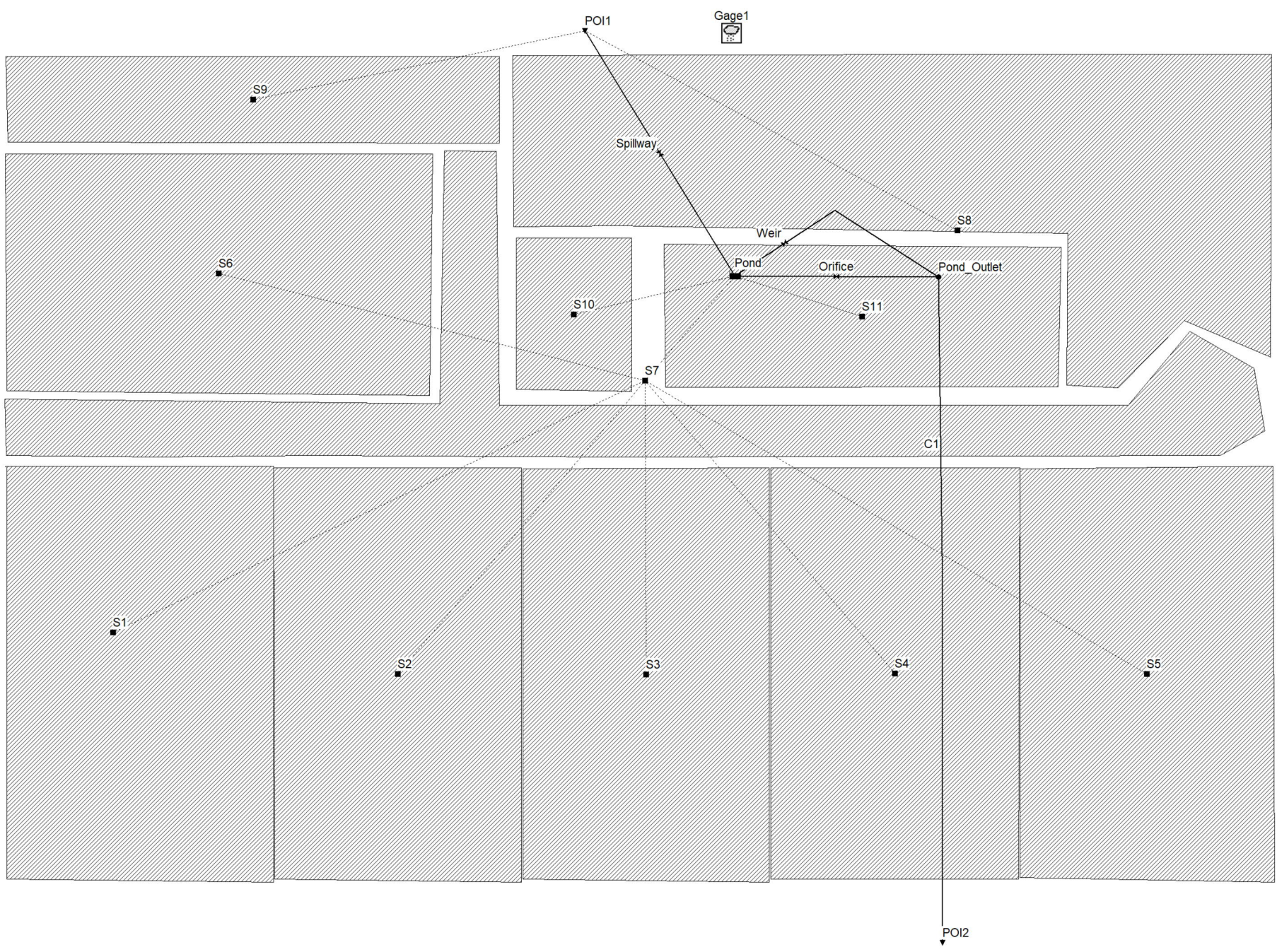
312

11+00

313

EX. 4' WOOD FENCE

135 LF 15" HDPE @ 0.50%



Pre/Post Development Runoff

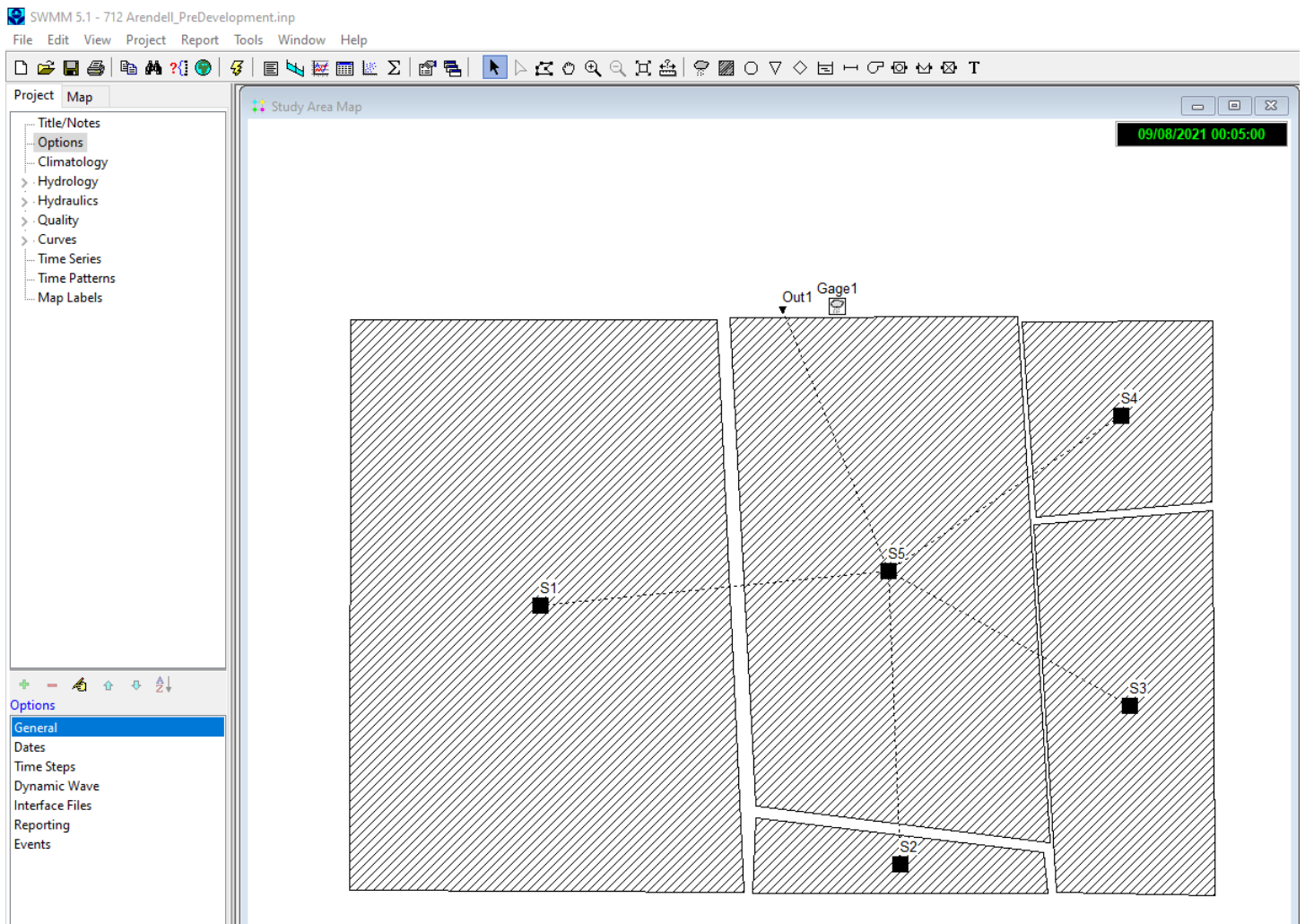
Stormwater peak flow

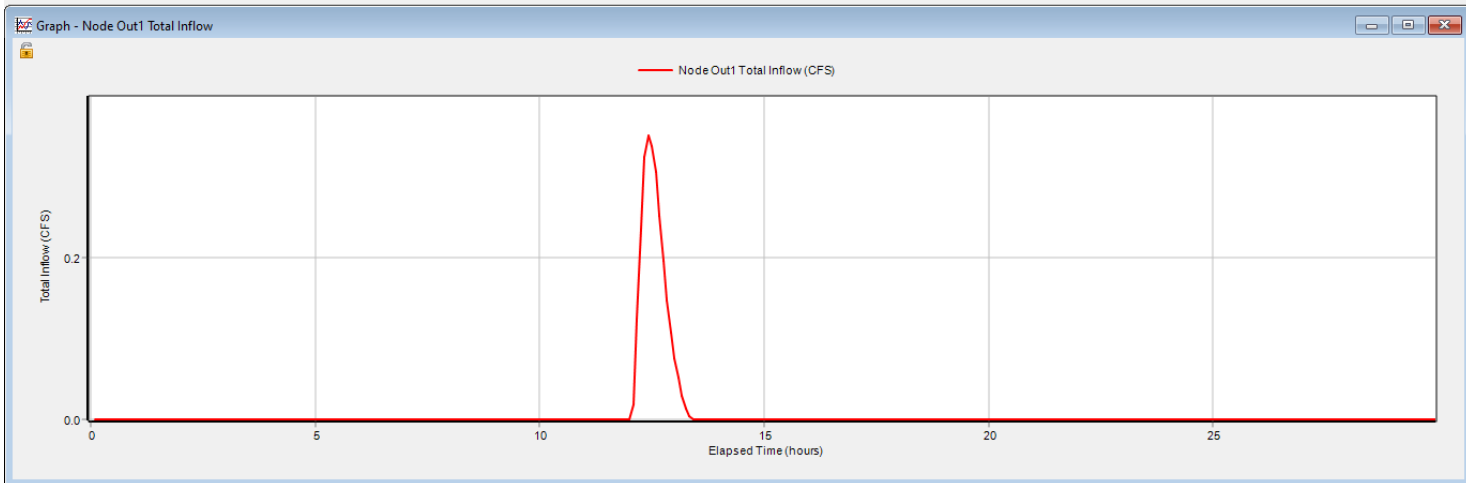
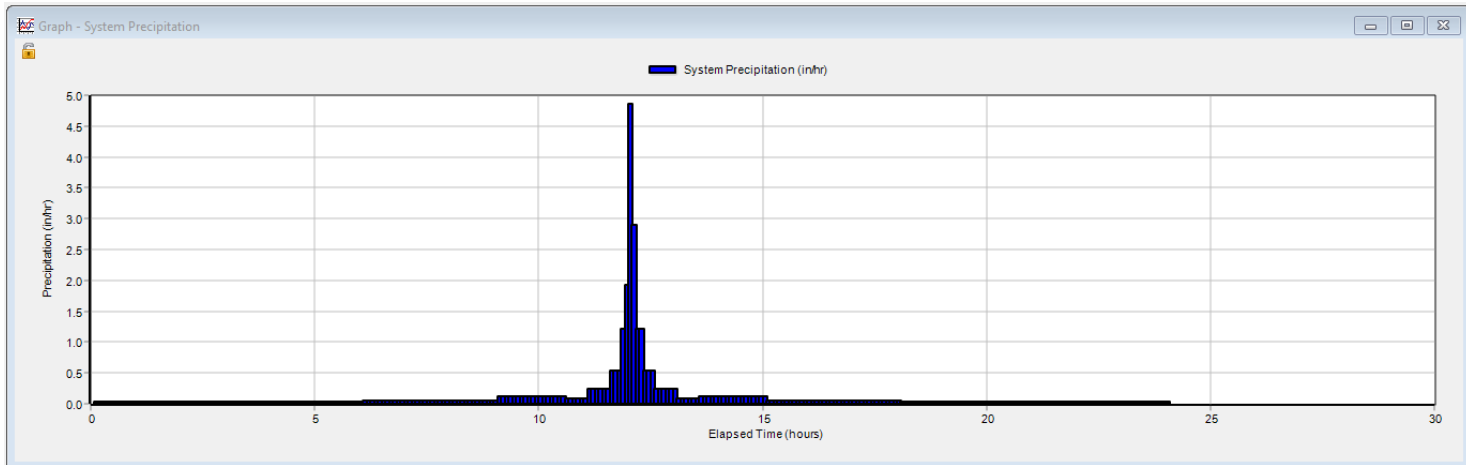
Storm Water Management Model (SWMM) developed by EPA is used to simulate the rainfall-runoff process in this project.

SWMM uses a nonlinear reservoir model to estimate surface runoff produced by rainfall over a subcatchment. The model was first published by Chen and Shubinski (1971) and included in the original release of SWMM (Metcalf and Eddy et al., 1971a). This method is closer to the reality than rational or SCM curve number methods. Although SWMM can be manipulated to use rational or SCM methods, its better to use more realistic methods such as the kinematic wave approach for routing rainfall-runoff process.

➤ Pre-development:

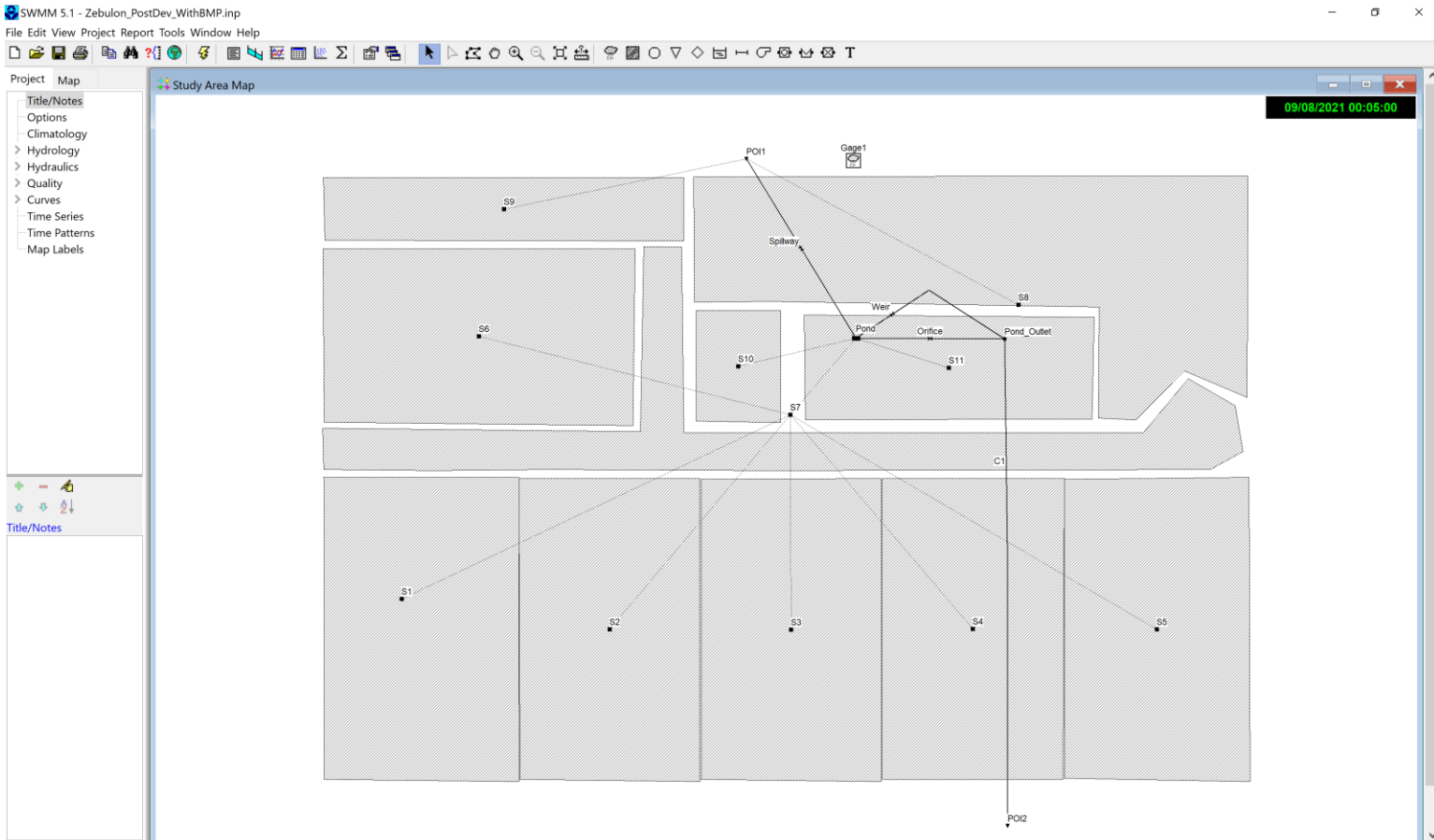
The pre-development situation is simulated using SWMM model (**Error! Reference source not found.**), the peak flow before development is 0.35 cfs.





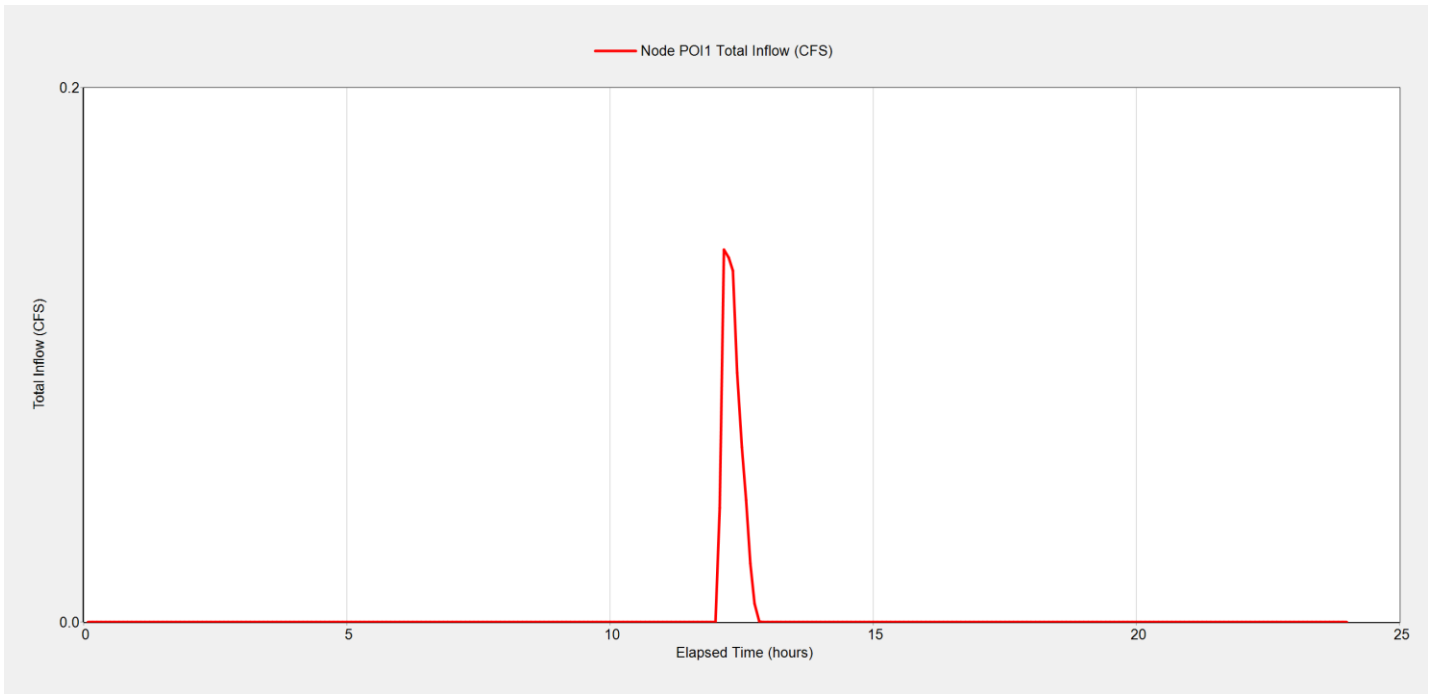
➤ **Post-development:**

Another SWMM model is built to simulate rainfall-runoff process for post-development situation in which 12 private lots, a road, and a wet pond were located. There are 11 subcatchments in the watershed in which runoff from 9 of them would flow to the pond, while runoff from 2 of them (S8 and S9) would be released to the “POI1”. The area of these two subcatchments that are not flowing to the pond is about 14,000 sf.

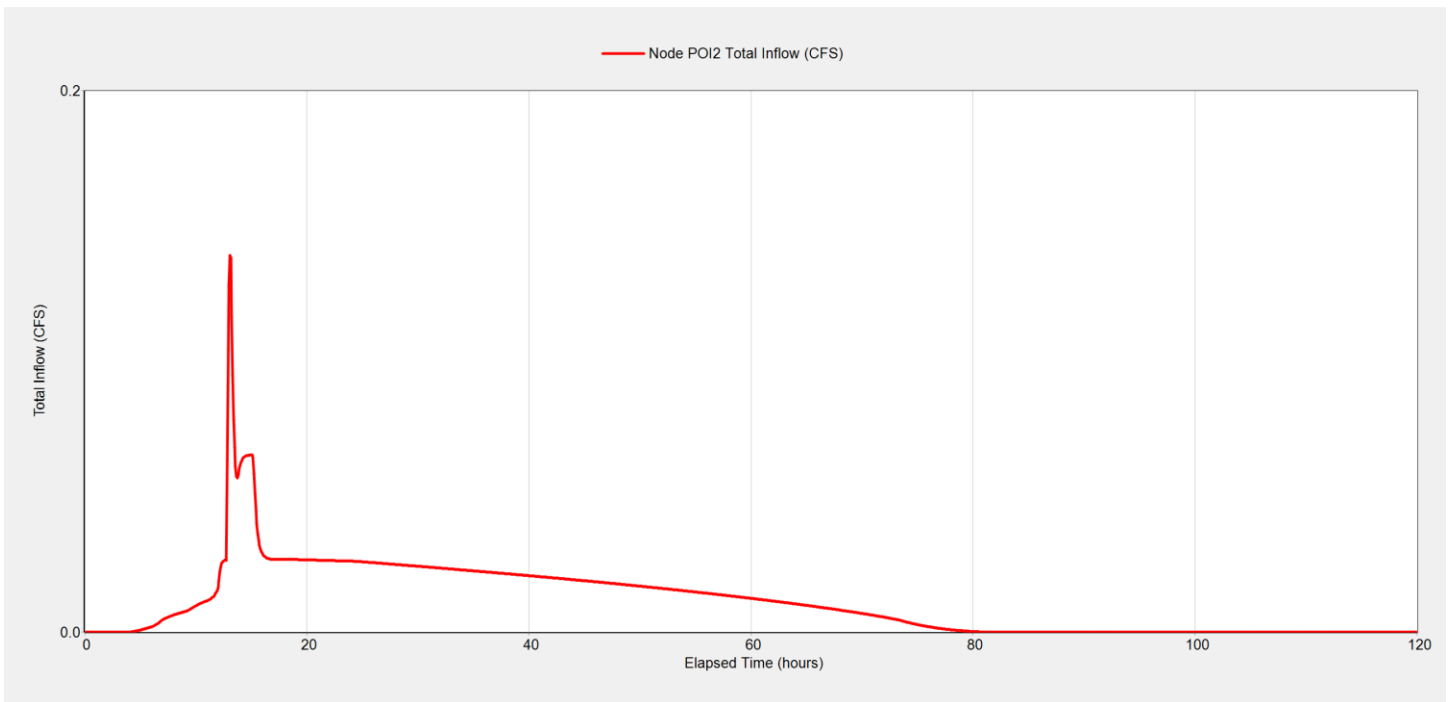


After running the model, the runoff analyses in the outlets, links, and nodes are as follow.

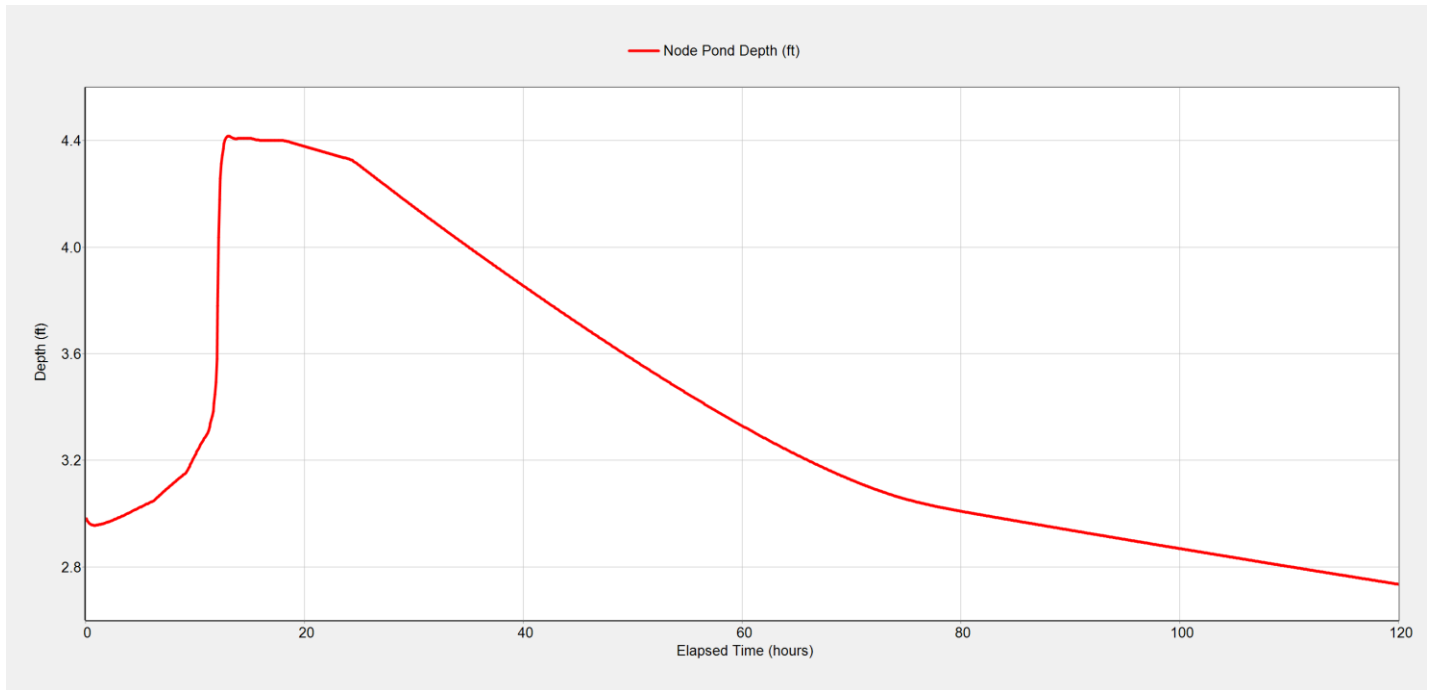
POI#1 Inflow (Stormwater runoff at the first POI)



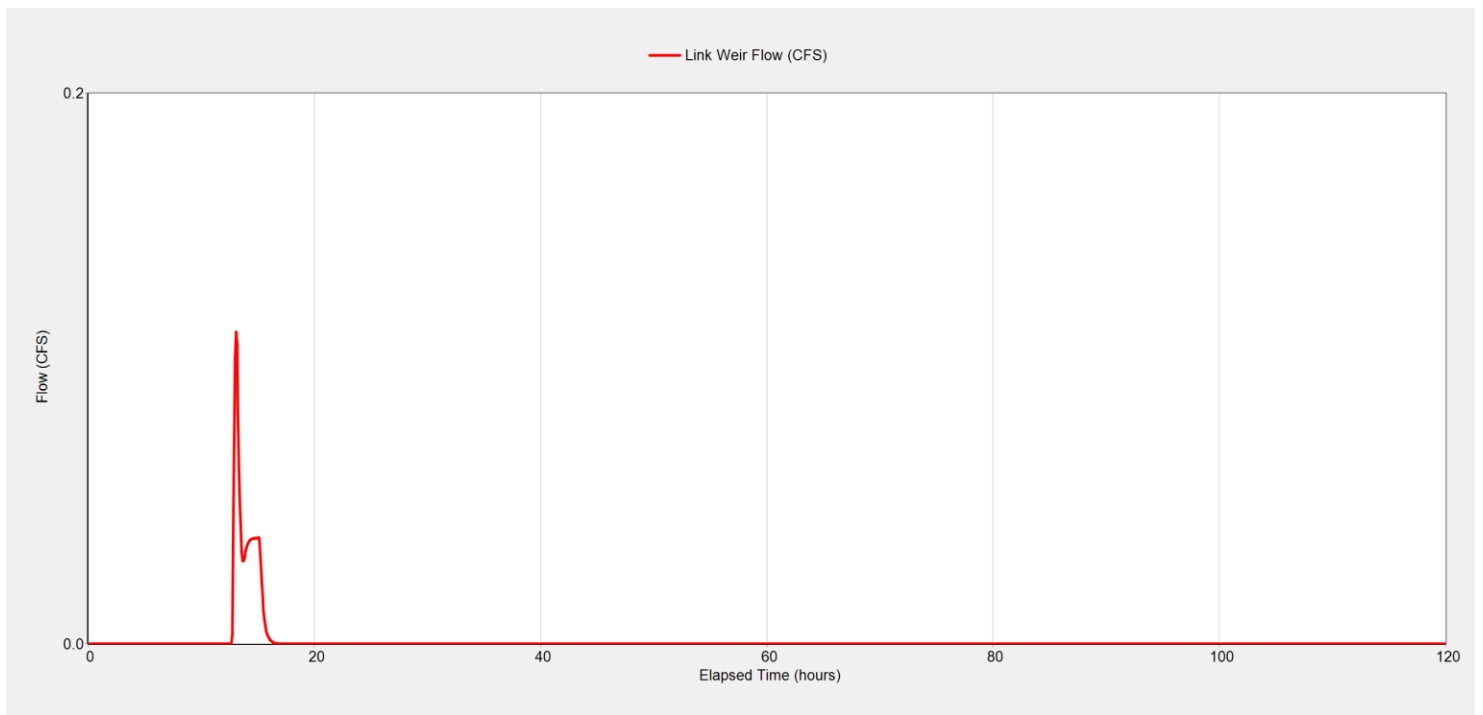
POI#2 Inflow (Stormwater runoff at the second POI)



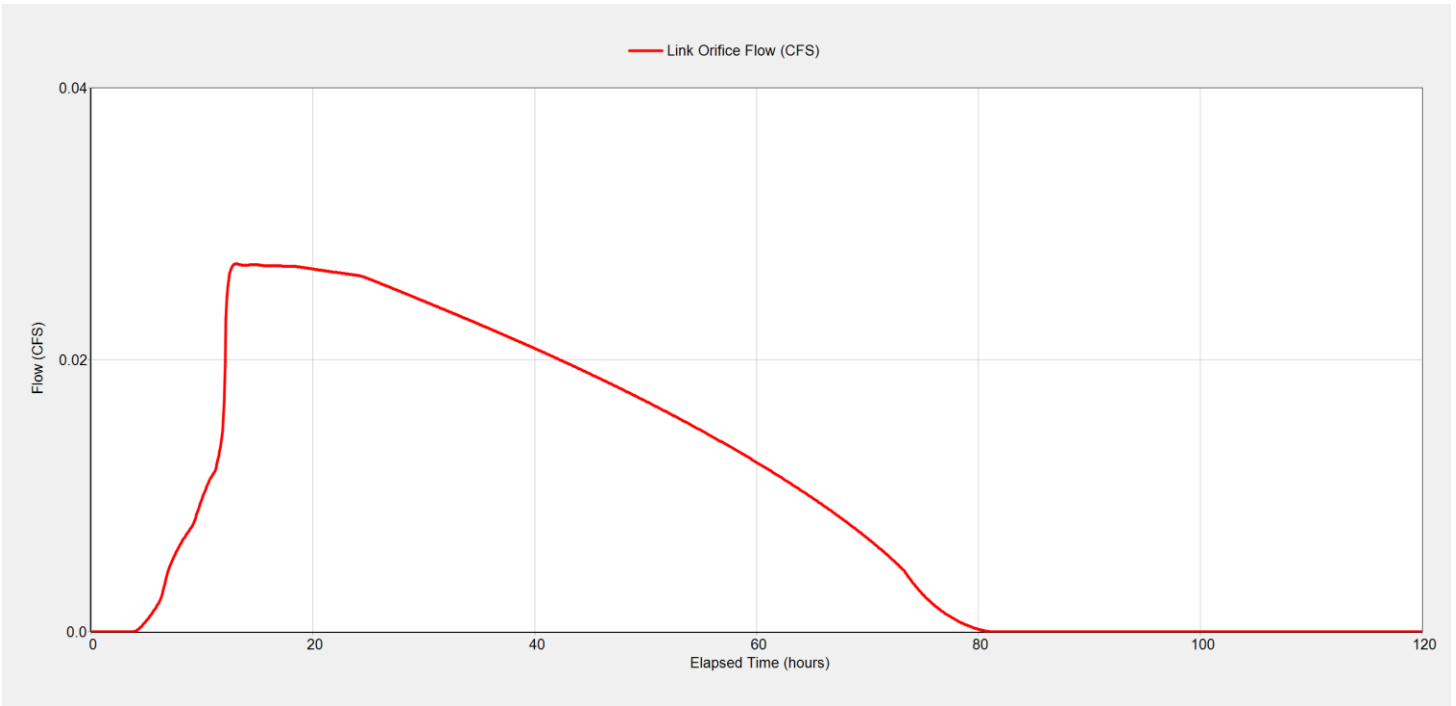
Pond depth



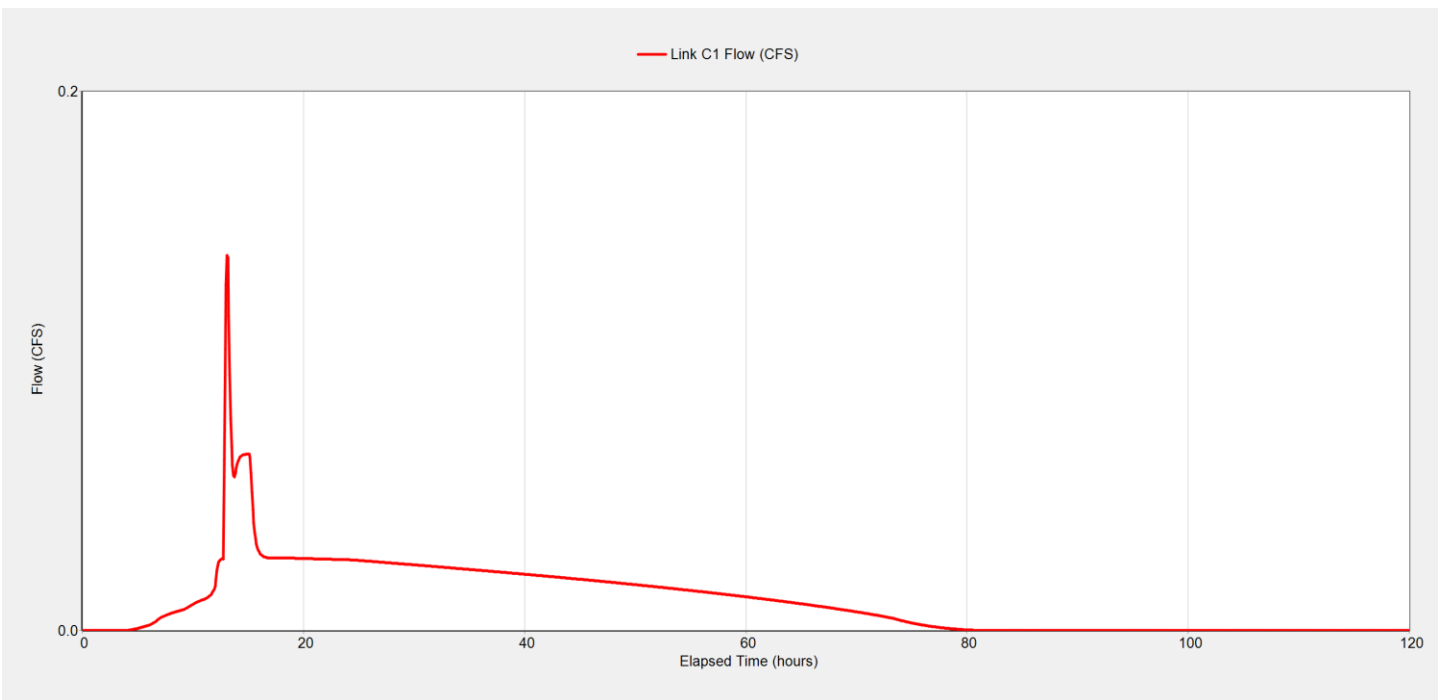
Flow at the weir.



Flow at the orifice



Flow at the pipe that connects pond outlet to the stormwater manhole.



“POI#1” total inflow: Peak flow at POI#1 is 0.14 cfs while before development, it was 0.35 cfs.

“POI#2” total inflow: The peak flow goes to the stormwater manhole is 0.15 cfs. This is the section Point of Interest (POI2).

“Pond” depth: Initial depth of pond is 3 ft (main pool). Depth increase in pond up to 4.4 feet which is the weir elevation. There are some overflow through the weir in the pond outlet structure. After about 70 hours, depth of pond would goes down to the main pool level, means the temporary pool flow out to the outlet. Therefore, runoff volume drawdown time is less than 3 days.

It should be mentioned that there is no flow at the emerfenxy “spillway”, means that the spillway is not activated based on the storm event. So, all runoffs are capture in the wet pond and released by the outlet structure to the stormwater pipeline manhole. So, the inflow at “POI#1” comes from two subcatchments that are not connected to the pond (S8 and S9), means the design SCM cannot influence on this overflow at this POI.

Considering both watershed POIs, the total peak flow in both outlets (post-development) is 0.29 cfs, which is 17% less than pre-development peak flow (0.35 cfs).

Nutrient Offset

Project Summary

| | | | |
|---|--------------------------------|---|--------------------|
| Project Name: | 712 Arendell-True home-Zebulon | | |
| Project Area (ft ²): | 60,200 ft ² | 1.3820 acres | Submission Date: |
| Disturbed Area (ft ²): | 45,981 ft ² | 1.0556 acres | |
| County: | Wake | Local Jurisdiction: | Zebulon |
| Development Land Use Type: | Single Family Residential | Owner Type: | Private |
| Development Activity Type: | Development - New | Designated Downtown Area? | no |
| Nutrient Management Watershed: | Neuse | Subwatershed: | Neuse - 03020203 |
| Phosphorus Delivery Zone: | Neuse - Contentnea | Nitrogen Delivery Zone: | Neuse - Contentnea |
| Phosphorus Delivery Factor (%): | 100% | Nitrogen Delivery Factor (%): | 100% |
| <i>Phosphorus Loading Rate Target (lb/ac/yr):</i> | <i>0.97</i> | <i>Nitrogen Loading Rate Target (lb/ac/yr):</i> | <i>7.05</i> |
| <i>Phosphorus Load Target at Site (lb/yr):</i> | <i>1.35</i> | <i>Nitrogen Load Target at Site (lb/yr):</i> | <i>9.74</i> |
| <i>Phosphorus Load Leaving Site w/SCMs (lb/yr):</i> | <i>1.17</i> | <i>Nitrogen Load Leaving Site w/SCMs (lb/yr):</i> | <i>8.59</i> |
| P Offsite Buy-Down Threshold Loading Rate (lb/ac/yr): | N/A | N Offsite Buy-Down Threshold Loading Rate | N/A |
| Total P Load Reduction Needed (lb/yr): | 0.00 | Total N Load Reduction Needed (lb/yr): | 0.00 |
| P Load Treatment Balance at Site (lb/yr): | -0.18 | N Load Treatment Balance at Site (lb/yr): | -1.16 |
| P Load Treatment Balance at Lake (lb/yr): | -0.18 | N Load Treatment Balance at Lake (lb/yr): | -1.16 |

Nutrient Export Summary

| | Pre-Project Whole Site Conditions | Post-Project Whole Site without SCMs | Post-Project Whole Site with SCMs | Post-Project SCM-Treated Area | Post-Project Untreated Area |
|---|-----------------------------------|--------------------------------------|-----------------------------------|-------------------------------|-----------------------------|
| Percent Impervious (for runoff calculation) (%) | 0.0% | 58.9% | 58.9% | 75.3% | 3.6% |
| Percent Built-Upon Area (BUA) (%) | 0.0% | 50.7% | 50.7% | 65.7% | 0.0% |
| Annual Runoff Volume (ft ³ /yr) | 10,231 | 119,477 | 109,725 | 105,767 | 3,958 |
| Annual Runoff % Change (relative to pre-D) | 0% | 1068% | 972% | | |
| Total Nitrogen EMC (mg/L) | 2.48 | 1.31 | 1.26 | 1.23 | 1.92 |
| <i>Total Nitrogen Load Leaving Site (lb/yr)</i> | <i>1.58</i> | <i>9.74</i> | <i>8.59</i> | <i>8.11</i> | <i>0.47</i> |
| <i>Total Nitrogen Loading Rate (lb/ac/yr)</i> | <i>1.15</i> | <i>7.05</i> | <i>6.21</i> | <i>7.61</i> | <i>1.50</i> |
| Total Nitrogen % Change (relative to pre-D) | 0% | 515% | 442% | | |
| Total Phosphorus EMC (mg/L) | 1.07 | 0.18 | 0.17 | 0.15 | 0.66 |
| <i>Total Phosphorus Load Leaving Site (lb/yr)</i> | <i>0.68</i> | <i>1.35</i> | <i>1.17</i> | <i>1.00</i> | <i>0.16</i> |
| <i>Total Phosphorus Loading Rate (lb/ac/yr)</i> | <i>0.49</i> | <i>0.97</i> | <i>0.84</i> | <i>0.94</i> | <i>0.51</i> |
| Total Phosphorus % Change (relative to pre-D) | 0% | 97% | 71% | | |

SCM/Catchment Summary

| SCM ID and Type | Volume Reduction (%) | TN Out (mg/L) | TP Out (mg/L) | TN Out (lbs/ac/yr) | TP Out (lbs/ac/yr) | TN Reduction (%) | TP Reduction (%) |
|-----------------------|----------------------|---------------|---------------|--------------------|--------------------|------------------|------------------|
| Catchment 1 | 8.44% | 1.23 | 0.15 | 7.61 | 0.94 | 12.44% | 15.09% |
| 101: Wet Pond per MDC | 8.44% | 1.23 | 0.15 | 7.61 | 0.94 | 12.44% | 15.09% |
| 102: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 103: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| Catchment 2 | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 201: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 202: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 203: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| Catchment 3 | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 301: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 302: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 303: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| Catchment 4 | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 401: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 402: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 403: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| Catchment 5 | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 501: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 502: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 503: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| Catchment 6 | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 601: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 602: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |
| 603: NA | 0.00% | 0.00 | 0.00 | 0.00 | 0.00 | 0.00% | 0.00% |

SCM rows in red have a data entry error for the SCM that makes an error in the calculation.

Nutrient Management Strategy Watershed - Nutrient Offset Credit Reporting Form

Please complete and submit the following information to the local government permitting your development project to characterize it and assess the need to purchase nutrient offset credits. Contact and rule implementation information can be found online at:

<http://deq.nc.gov/about/divisions/water-resources/planning/nonpoint-source-management/nutrient-offset-information>

PROJECT INFORMATION

| | | | |
|--|--------|-----------------------------------|---------------------------|
| Applicant Name: | | | |
| Project Name: | | 712 Arendell-True home-Zebulon | |
| Project Address: | | 712 North Arendell Ave | |
| Date: (mm/dd/yyyy) | | Development Land Use Type: | Single Family Residential |
| County: | Wake | Development Activity Type: | Development - New |
| Pre-Project Built-Upon Area %: | 0.00% | Project Latitude: | 0 |
| Post-Project Built-Upon Area %: | 50.68% | Project Longitude: | 0 |

WATERSHED INFORMATION

| | | | |
|---------------------------------------|--------------------|---|------|
| Nutrient Management Watershed: | Neuse | N Offsite Threshold Rate (lb/ac/yr): | N/A |
| Subwatershed: | Neuse - 03020203 | P Offsite Threshold Rate (lb/ac/yr): | N/A |
| Nitrogen Delivery Zone: | Neuse - Contentnea | Nitrogen Delivery Factor: | 100% |
| Phosphorus Delivery Zone: | Neuse - Contentnea | Phosphorus Delivery Factor: | 100% |

NUTRIENT OFFSET REQUEST

Nitrogen Load Offset Needs

| (A) | (B) | (C) | (D) | (E) | (F) | (G) | (H) | (L) (Where Applicable) |
|------------------------------------|----------------------------------|---------------------------------|-------------------------------------|-------------------|-----------------------|---------------------|--|-----------------------------------|
| Untreated Loading Rate (lbs/ac/yr) | Treated Loading Rate (lbs/ac/yr) | Loading Rate Target (lbs/ac/yr) | Reduction Need (lbs/ac/yr) B - C | Project Size (ac) | Offset Duration (yrs) | Delivery Factor (%) | State Buy Down Amount (lbs) D * E * F * G | Local Gov't Buy Down Amount (lbs) |
| 7.05 | 6.21 | 7.05 | -0.84 | 1.3820 | 30 | 100% | 0.00 | |

Phosphorus Load Offset Needs

| (A) | (B) | (C) | (D) | (E) | (F) | (G) | (H) | (L) (Where Applicable) |
|---------------------------------|-------------------------------|---------------------------------|-------------------------------------|-------------------|-----------------------|---------------------|--|-----------------------------------|
| Untreated Load Rate (lbs/ac/yr) | Treated Load Rate (lbs/ac/yr) | Loading Rate Target (lbs/ac/yr) | Reduction Need (lbs/ac/yr) B - C | Project Size (ac) | Offset Duration (yrs) | Delivery Factor (%) | State Buy Down Amount (lbs) D * E * F * G | Local Gov't Buy Down Amount (lbs) |
| 0.97 | 0.84 | 0.97 | -0.13 | 1.3820 | 30 | 100% | 0.00 | |

LOCAL GOVERNMENT AUTHORIZATION

| | |
|---|---------------|
| Local Government Name: Zebulon | |
| Staff Name: | Phone: |
| Staff Email: | Date: |
| <p style="text-align: center;">Local Government Authorizing Signature:</p> | |