Erosion Control Calculations

401 WEST GANNON AVENUE

Zebulon, North Carolina

July 10, 2023



Prepared By:

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INTRODUCTION AND GENERAL INFORMATION

This report presents the erosion control design for the land disturbance for the proposed proposed 401 West Gannon Avenue project located at the intersection of West Gannon Avenue and North Rotary Drive in Zebulon, North Carolina.

BACKGROUND

The proposed development is comprised of approximately 0.99 acres of land and is currently vacant. At full build out the project will consist of 11 condominium units with infrastructure to support the proposed development as well as one SCM to meet the Town's stormwater requirements.

EROSION CONTROL REQUIREMENTS

All sediment and erosion control devices were designed in accordance with requirements and specifications by Wake County and in the North Carolina Erosion and Sediment Control Planning and Design Manual.

Sediment and erosion control devices used for this project include sediment traps with a Faircloth skimmer, silt fence, silt fence outlets, diversion berms, and construction entrances. Diversion berms should be located as shown on the plans to divert surface flow into the sediment traps. Silt fence will be utilized where the drainage area is less than ¼ acre per 100 linear feet of silt fence. Silt fence outlets will be located at low points in along the silt fence to prevent high flow velocities from blowing out the silt fence while still allowing for the sediment to settle out of the surface flow. The construction entrances will be located at various locations for access to the areas of the project.

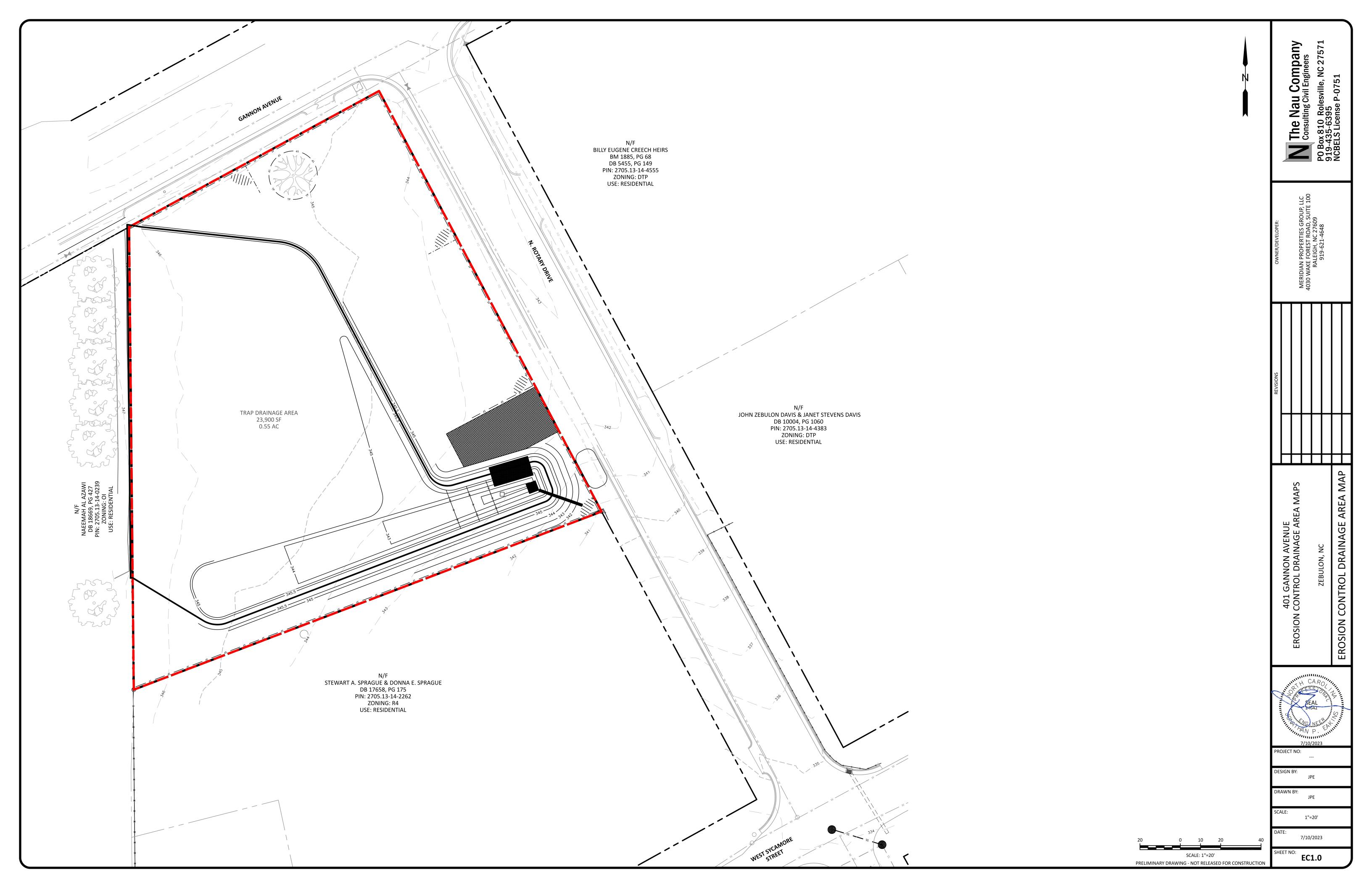
CALCULATIONS

The following equations were used in developing the calculated values in shown in the appendices to this report.

Sediment Traps and Basins

- Peak flow to the basins were determined using the rational formula
- The required volume was calculated based on 1800 cubic feet per acre draining to the basin
- The required surface area was calculated based on 435 square feet of surface area per cfs for the 10-year storm
- The sediment basin volume provided was calculated by the average end area method applied vertically
- The sediment basin surface area was taken from contour areas taken from the proposed basin grading, or interpolated between contours if necessary.
- The volume to dewater was set equal to the required volume of the sediment basin
- Skimmer orifice sizes were determined by the orifice equation based on the head available for the selected skimmer size
- The depth of flow over the spillway was calculated with the weir equation using the selected spillway length and peak flow from the 10-year storm

SEDIMENT TRAP DRAINAGE AREA MAP



SEDIMENT TRAP CALCULATIONS



SKIMMER BASIN DESIGN

Location ID SCM/Trap

Drainage Area Data

| Disturbed area | 0.6 acres |
|-------------------------------------|-------------------------|
| Design storm for surface area calcs | 10-year |
| Runoff for surface area calcs | 2.0 cfs |
| Design storm for spillway calcs | 10-year |
| Runoff for spillway calcs | 4.0 cfs |
| Calculation methodology | Software/rational calcs |

Skimmer Basin Design Requirements

| Sediment storage per disturbed acre | 1800 cubic feet/acre |
|-------------------------------------|----------------------|
| Surface area per inflow CFS | 325 square feet/cfs |
| Minimum trap dewater time | 2 days |
| Maximum trap dewater time | 5 days |
| Maximum sediment storage depth | 3.5 feet |
| Minimum freeboard from sediment | 1.5 feet |
| Minimum spillway length | 10.0 feet |
| Maximum spillway flow depth | 0.50 feet |
| Minimum freeboard for spillway flow | 1.00 feet |
| Volume to dewater | Required |
| | |

Skimmer Basin Design Data

| Sediment storage depth | 2.0 feet |
|------------------------|-----------|
| Height to spillway | 2.0 feet |
| Spillway length | 20.0 feet |
| Skimmer size | 1.50 in. |
| Orifice size | 0.50 in. |

Sediment Trap Contour and Volume Data

| Elevation | Stage | Contour area | Incremental Volume | Cumulative volume |
|-----------|-------|--------------|--------------------|-------------------|
| 342.0 | 0.0 | 78 sf | 0 | 0 |
| 343.0 | 1.0 | 691 sf | 385 | 385 |
| 344.0 | 2.0 | 1,914 sf | 1,303 | 1,687 |
| 345.0 | 3.0 | 3,746 sf | 2,830 | 4,517 |
| 345.5 | 3.5 | 4,123 sf | 1,967 | 6,484 |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |



SKIMMER BASIN DESIGN

Location ID SCM/Trap

Sediment Trap Dewatering

| Sediment Trap Dewatering Calculations | | | |
|---------------------------------------|-----------|--|--|
| Volume to dewater 990 CF | | | |
| Skimmer size | 1.5 in. | | |
| Orifice diameter | 0.50 in. | | |
| Coefficient of discharge | 0.60 | | |
| Orifice area | 0.0014 SF | | |
| Head on orifice | 0.125 ft | | |
| Flowrate | 0.002 cfs | | |
| Dewatering time 4.9 days | | | |

| Head on skimmer orifice | | | |
|-------------------------|-----------|--|--|
| Skimmer size Head | | | |
| 1.5 in. | 0.125 ft. | | |
| 2.0 in. | 0.167 ft. | | |
| 2.5 in. | 0.208 ft. | | |
| 3.0 in. | 0.250 ft. | | |
| 4.0 in. | 0.333 ft. | | |
| 5.0 in. | 0.333 ft. | | |
| 6.0 in. | 0.417 ft. | | |
| 8.0 in. | 0.500 ft. | | |

CHECK DESIGN REQUIREMENTS

| Design criteria | Required | Provided |
|-------------------------------|----------------|----------|
| Sediment storage volume | 990 CF | 1,687 CF |
| Sediment surface area | 650 CF | 1,914 CF |
| Sediment storage depth | 3.5 ft. (max) | 2.0 feet |
| Sediment freeboard | 1.5 ft. (min) | 1.5 ft |
| Spillway length | 10.0 ft. (min) | 20.0 ft |
| Flow depth over spillway | 0.50 ft. (max) | 0.15 ft |
| Freeboard at design discharge | 1.0 ft. (min) | 1.35 ft |
| Trap dewatering time | 2 to 5 days | 4.9 days |

Note: data and methodology taken from NC Erosion Control Manual

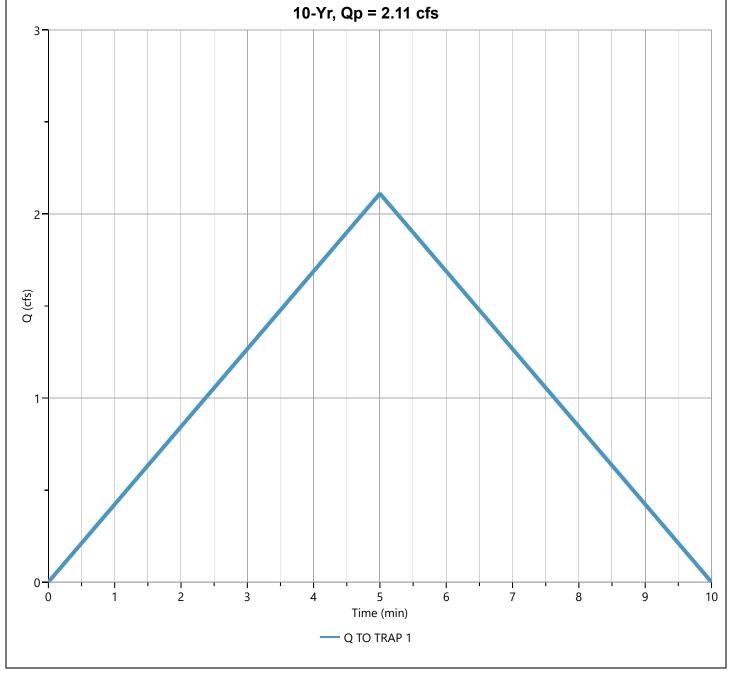
RATIONAL RUNOFF CALCULATIONS TO TRAPS

Studio Express by Hydrology Studio v 1.0.0.14

07-10-2023

Q TO TRAP 1 Hyd. No. 1

| Hydrograph Type | = Rational | Peak Flow | = 2.111 cfs |
|--------------------|------------------------------|----------------------|---------------|
| Storm Frequency | = 10-yr | Time to Peak | = 5 min |
| Time Interval | = 1 min | Runoff Volume | = 633 cft |
| Drainage Area | = 0.6 ac | Runoff Coeff. | = 0.5 |
| Tc Method | = User-Defined | Time of Conc. (Tc) | = 5.0 min |
| IDF Curve | = RDU-Rainfall Intensity.idf | Intensity | = 7.036 in/hr |
| Freq. Corr. Factor | = 1.00 | Asc/Rec Limb Factors | s = 1/1 |



OUTLET PROTECTION/RIPRAP APRON CALCULATIONS

DESIGN OF RIPRAP OUTLET PROTECTION

SCM July 10, 2023

| OUTLET FLOWRATE | 1.0_cfs |
|-------------------|----------------------|
| PIPE DIAMETER | 12 inches |
| OUTLET PIPE SLOPE | 1.00 % |
| NUMBER OF PIPES | 1 |
| PIPE SEPARATION | 0 feet |
| ZONE FROM GRAPH | 1 |
| PIPE AREA | 0.79 sq. ft. |
| FLOW VELOCITY | 1.3 ft/sec |
| MATERIAL | NCDOT Class A riprap |

| LENGTH | 4.00 | feet |
|----------------|------|--------|
| WIDTH | 3.00 | feet |
| STONE DIAMETER | 3 | inches |
| THICKNESS | 9 | inches |
| | | |

| Zone | Material | Diameter | Thickness | Length | Width |
|------|------------------------|----------|-----------|-----------|----------|
| 1 | Class A | 3 | 9 | 4 x D(o) | 3 x D(o) |
| 2 | Class B | 6 | 22 | 6 x D(o) | 3 x D(o) |
| 3 | Class I | 13 | 22 | 8 x D(o) | 3 x D(o) |
| 4 | Class I | 13 | 22 | 8 x D(o) | 3 x D(o) |
| 5 | Class II | 23 | 27 | 10 x D(o) | 3 x D(o) |
| 6 | Class II | 23 | 27 | 10 x D(o) | 3 x D(o) |
| 7 | Special study required | | | | |

- 1. Calculations based on NY DOT method Pages 8.06.05 through 8.06.06 in NC Erosion Control Manual
- 2. Outlet velocity based on full-flow velocity

