

# CHAMBLEE LAKE

ZEBULON, NORTH CAROLINA

## EROSION CONTROL CALCULATION REPORT

**PROJECT NUMBER:** DRH22004  
**DESIGNED BY:** BENJAMIN KINTNER  
**CHECKED BY:** KATIE ANDERSON, PE

**DATE:** 27 FEBRUARY, 2024



MCADAMS

621 HILLSBOROUGH ST, SUITE 500  
RALEIGH, NORTH CAROLINA 27603  
NC Lic. # C-0293



## TABLE OF CONTENTS

SECTION I .....	3
1.0 PROJECT DESCRIPTION.....	3
2.0 EROSION AND SEDIMENT CONTROL MEASURES .....	3
2.1 Temporary Gravel Construction Entrance/Exit .....	3
2.2 Silt Fence .....	3
2.3 Storm Drain Inlet Protection.....	3
2.4 Temporary Diversions .....	4
2.5 Skimmer Sediment Basin.....	4
3.0 VEGETATIVE STABILIZATION .....	4
4.0 MAINTENANCE .....	4
5.0 EROSION CONTROL CALCULATION METHODOLOGY .....	4
LIST OF APPENDICES.....	5
Skimmer Sediment Basin Calculations.....	6
Drainage Area Maps.....	7
Slope Drain and Diversion Ditch Calculations .....	8



# CHAMBLEE LAKE

## ZEBULON, NORTH CAROLINA

### EROSION CONTROL CALCULATION REPORT

#### SECTION I

##### 1.0 PROJECT DESCRIPTION

The site of the proposed single-family and multi-family residential development is located on Chamblee Road in Zebulon, North Carolina. The proposed residential development includes approximately 355 dwelling units on approximately 136 acres. It is a mix of single-family and townhomes built on the site. The development also includes offsite road improvements and utility improvements along Chamblee Road.

##### 2.0 EROSION AND SEDIMENT CONTROL MEASURES

All vegetative practices and erosion and sediment control features shall be designed, constructed and maintained to control runoff resulting from the 2 and 10 year frequency storms. Supporting design calculations are attached. Plan-view drawings with details are also provided.

###### 2.1 Temporary Gravel Construction Entrance/Exit

A construction entrance will be needed to clean the tires of vehicles and equipment during wet conditions. There is a high potential for tracking mud and sediment onto adjacent local streets.

###### 2.2 Silt Fence

Sediment fences will be provided down gradient of the proposed site grading at the locations shown on the drawings. The sediment fences will act as a filter for sediment laden runoff sheet flowing from slopes or small areas that cannot be directed to a temporary sediment trap. Silt Fence Outlets will be placed at any low points along the Silt Fence to allow collected runoff to pass through without overwhelming the fencing.

###### 2.3 Storm Drain Inlet Protection

Storm sewer inlets will need to be protected to prevent sediment-laden runoff from clogging the storm pipes during construction. Inlet protection should be used on each inlet until upland areas are stabilized.



## 2.4 Temporary Diversions

Diversion ditches/berms will be constructed at the locations shown on the drawings. The ditches/berms are to divert sediment-laden water to appropriate traps or suitable outlet.

## 2.5 Skimmer Sediment Basin

Temporary sediment traps will be constructed at the locations shown on the drawings. Temporary diversion ditches will direct sediment-laden water to the traps. The sediment traps will be maintained during the course of construction until stabilization is reached. The Sediment Traps for this project are designed to be drained within 3 days after a rain event with the use of a Skimmer device.

## **3.0 VEGETATIVE STABILIZATION**

Vegetative cover shall be re-established within 14 calendar days after completion of the activity. Temporary and permanent seeding specifications are attached.

## **4.0 MAINTENANCE**

Sediment fences shall be inspected at least once a week and after each rainfall. Repairs shall be made immediately. Sediment deposits shall be removed as needed to provide adequate storage volume for the next rainfall event, and to reduce pressure on the fence. Fencing materials and sediment deposits shall be removed, and the area brought to grade following stabilization of up-gradient disturbed areas.

## **5.0 EROSION CONTROL CALCULATION METHODOLOGY**

- For the Skimmer Sediment Basins, a drainage area was measured as well as assigning disturbed surface percentage. From this disturbed percentage, a rational c factor was calculated. This data was used for basin sizing.
- The basins were sized using a series of electronic spreadsheets developed to size the basin according to Wake County criteria and to also route the 2 and 10 storm events through the proposed basin to check the spillway device for integrity during these storm events.
- The “Skimmer” water drawdown devices were sized by the method described in “A Manual for Designing, Installing and Maintaining Skimmer Sediment Basins” by J. W. Faircloth & Son.
- The various inlet types are shown on the stormwater detail sheet, within the plan set. Headwalls are used at discharge points. Structures are used at inlet points. Velocity dissipators are provided at discharge points to prevent erosion and scour in these areas. The dissipators have been sized using the NYDOT method.

## **LIST OF APPENDICES**

- SKIMMER SEDIMENT BASIN CALCULATIONS
- DRAINAGE AREA MAPS
- SLOPE DRAIN AND DIVERSION DITCH CALCULATIONS

---

## Skimmer Sediment Basin Calculations

---



**SCM-A  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-A

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	4.59 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	4.59 ac.	0.25
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>4.59 ac.</b>	<b>4.59 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.25</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	493 feet		
Height of watershed =	21 feet		
Calculated t(c) =	3.1 minutes		
Minimum t(c) =	5.0 minutes		
Time of concentration =	5.0 minutes		

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	5.74 in/hr
10	195	22	7.22 in/hr
25	232	23	8.29 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.74 in/hr	11.9 cfs	6.59 cfs
10-year storm	7.22 in/hr	14.9 cfs	8.29 cfs
25-year storm	8.29 in/hr	17.1 cfs	9.51 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	287
Bottom width =	74
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	16
Height of berm =	6
Top of trap length =	323
Top of trap width =	110
Bottom elevation =	314

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	16524 cu. ft.	102880 cu. ft.	
Sediment surface area =	6490 sq. ft.	30480 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	3.9L:1W	
Spillway length =	10.0 ft. (min)	16	
10-Year flow depth over spillway =	0.50 ft. (max)	-1.94 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	3.94 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 16,524 cubic feet

24 HOURS	Best Option =	1 - 4 inch Skimmers with a 3.6 inch orifice
2 DAYS	Best Option =	1 - 3 inch Skimmers with a 2.8 inch orifice
3 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.3 inch orifice
4 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

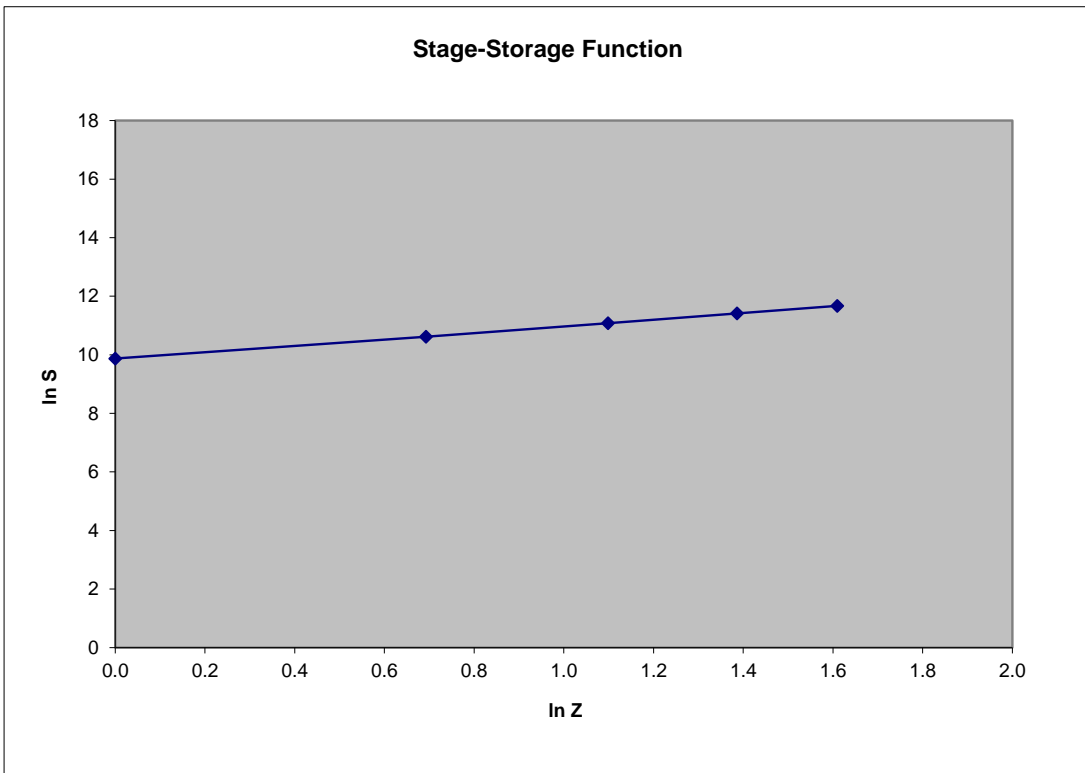
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
314	18271		0	0			
315	20539	19405	19405	1	9.8733	0.0000	1.01
316	22562	21550.5	40955.5	2	10.6202	0.6931	1.97
317	24641	23601.5	64557	3	11.0753	1.0986	2.97
318	26776	25708.5	90265.5	4	11.4105	1.3863	4.01
319	26968	26872	117137.5	5	11.6711	1.6094	5.06

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.12$  and  $K_s = 19172$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	4.59 ac	<b>Use Kirpich Equation:</b> Tc = 5.0 min
Disturbed Area =	4.59 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.74 in/hr i <sub>10</sub> = 7.22 in/hr i <sub>25</sub> = 8.29 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	493 ft	
Vertical Fall =	21 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 11.9 cfs Q <sub>10</sub> = 14.9 cfs Q <sub>25</sub> = 17.1 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 29.4 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 39.1 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 42.0 minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	<b>Peak Stage =</b>	<b>315.43</b>	<b>ft</b>
	<b>Rise =</b>	<b>1.43</b>	<b>ft</b>
<b>Freeboard =</b>	<b>4.57</b>	<b>Peak Outflow =</b>	<b>0.07</b> cfs

INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> <b>320.00</b> ft
Qp = <b>11.85</b> cfs		N = <b>1</b>	
Tp = <b>29.4</b> min		L = <b>16</b> ft	
dT = <b>1.0</b> min		Cw = <b>3.00</b>	<b>Skimmer Orifice:</b>
<b>Stage-Storage Results:</b>		Zcr = <b>318.00</b> ft	Number = <b>1.00</b> Ea
Ks = <b>19172</b>			Diameter = <b>2.50</b> Inches
b = <b>1.12</b>			Head = <b>2.35</b> inches
Z <sub>0</sub> = <b>314.0</b> ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = <b>314.00</b> ft			

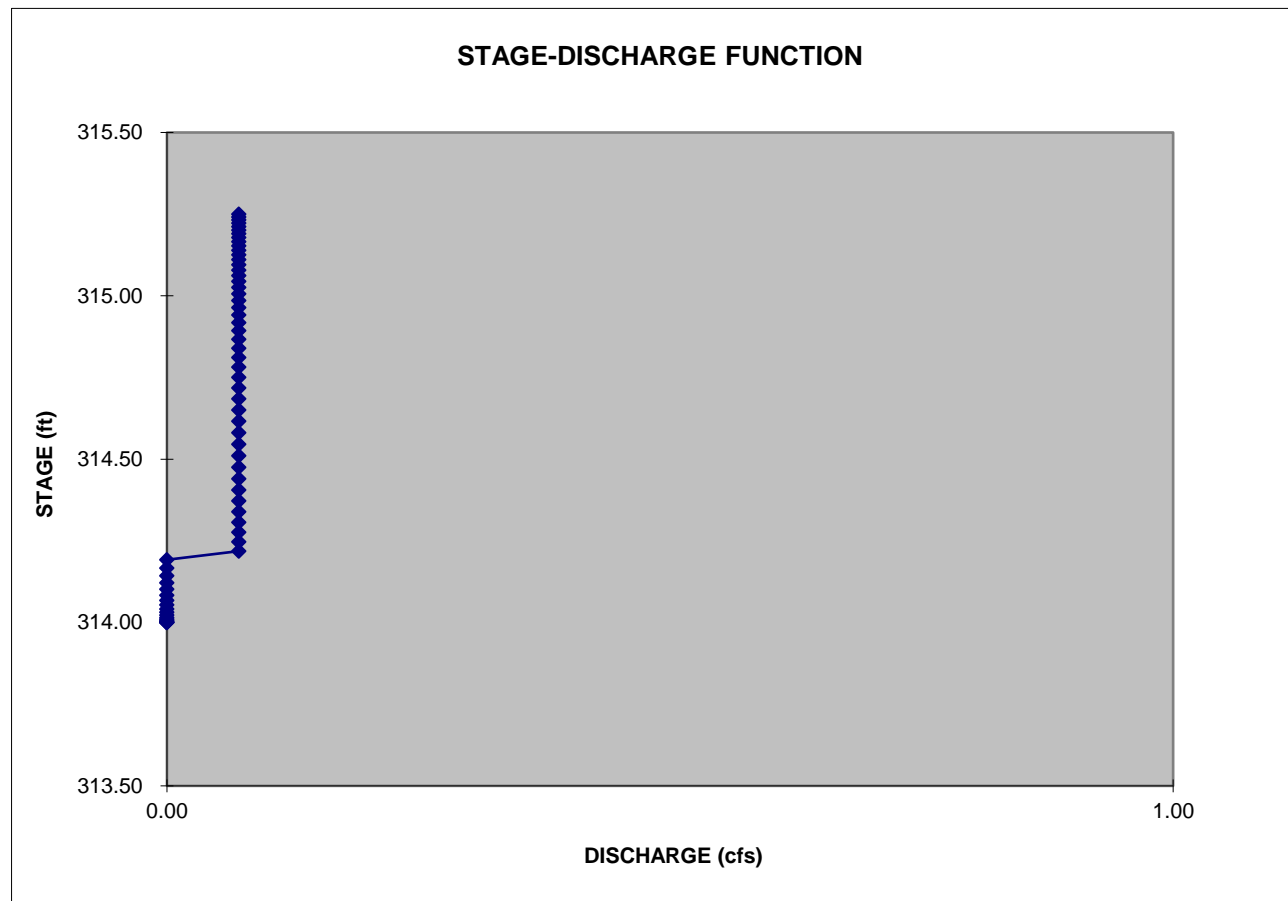
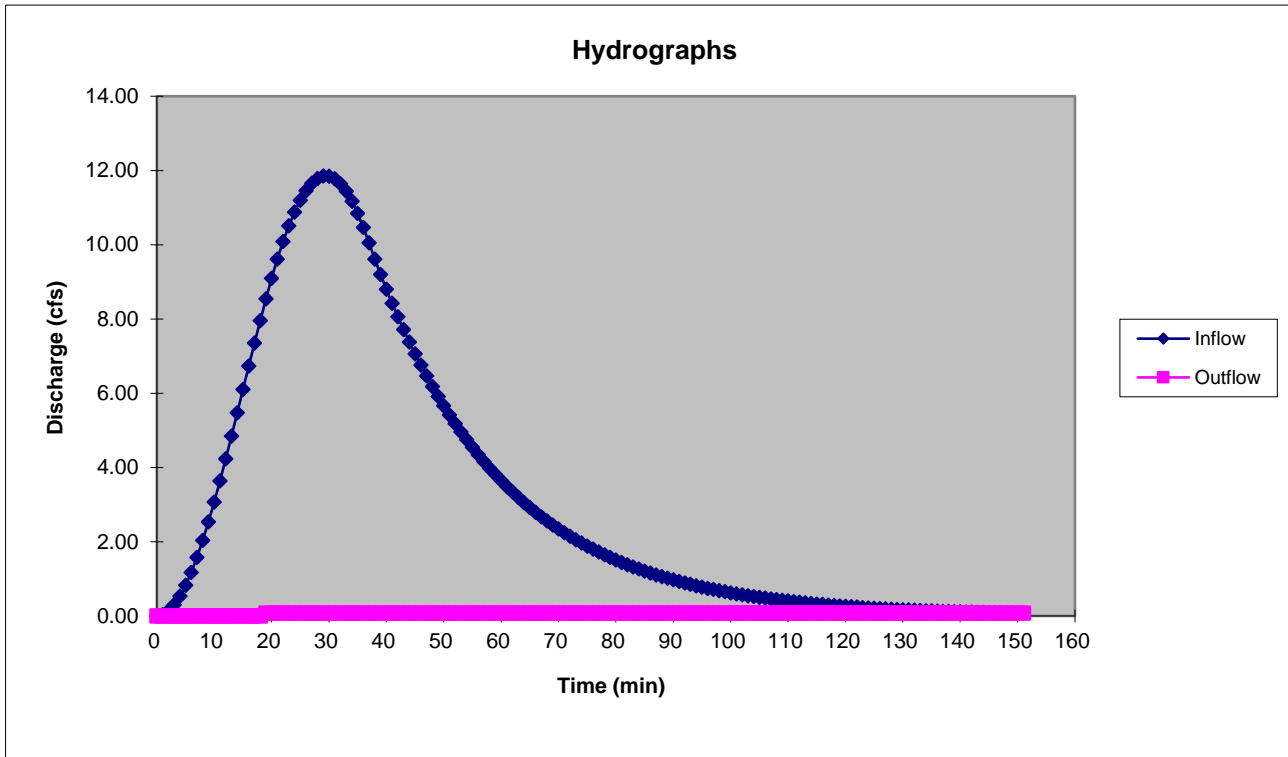
OUTPUT								
1 Time T min	2 Inflow I cts	3 Storage S cu-ft	4 Stage Z ft	5 Outflow O cts	6 Weir Flow cts	7 Skimmer Flow cts	8	9
0	0	0.00	314.00	0.0	0.0	0.0		
1	0	0.00	314.00	0.0	0.0	0.00		
2	0	2.02	314.00	0.0	0.0	0.00		
3	0	10.08	314.00	0.0	0.0	0.00		
4	1	28.14	314.00	0.0	0.0	0.00		
5	1	60.03	314.01	0.0	0.0	0.00		
6	1	109.43	314.01	0.0	0.0	0.00		
7	2	179.82	314.02	0.0	0.0	0.00		
8	2	274.45	314.02	0.0	0.0	0.00		
9	3	396.28	314.03	0.0	0.0	0.00		
10	3	547.97	314.04	0.0	0.0	0.00		
11	4	731.84	314.05	0.0	0.0	0.00		
12	4	949.85	314.07	0.0	0.0	0.00		
13	5	1203.55	314.08	0.0	0.0	0.00		
14	5	1494.11	314.10	0.0	0.0	0.00		
15	6	1822.26	314.12	0.0	0.0	0.00		
16	7	2188.32	314.14	0.0	0.0	0.00		
17	7	2592.17	314.17	0.0	0.0	0.00		
18	8	3033.26	314.19	0.0	0.0	0.00		
19	9	3510.62	314.22	0.1	0.0	0.07		
20	9	4018.58	314.25	0.1	0.0	0.07		
21	10	4559.65	314.28	0.1	0.0	0.07		
22	10	5131.66	314.31	0.1	0.0	0.07		
23	11	5732.11	314.34	0.1	0.0	0.07		
24	11	6358.17	314.37	0.1	0.0	0.07		
25	11	7006.71	314.41	0.1	0.0	0.07		
26	11	7674.35	314.44	0.1	0.0	0.07		
27	12	8357.51	314.48	0.1	0.0	0.07		
28	12	9052.40	314.51	0.1	0.0	0.07		
29	12	9755.12	314.55	0.1	0.0	0.07		
30	12	10461.68	314.58	0.1	0.0	0.07		
31	12	11168.03	314.62	0.1	0.0	0.07		
32	12	11870.14	314.65	0.1	0.0	0.07		
33	11	12564.03	314.68	0.1	0.0	0.07		
34	11	13245.79	314.72	0.1	0.0	0.07		
35	11	13911.68	314.75	0.1	0.0	0.07		
36	10	14558.11	314.78	0.1	0.0	0.07		
37	10	15181.73	314.81	0.1	0.0	0.07		
38	10	15780.26	314.84	0.1	0.0	0.07		
39	9	16352.76	314.87	0.1	0.0	0.07		
40	9	16900.35	314.89	0.1	0.0	0.07		





# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.94	ft	
Peak Stage =	316.06	ft	
Rise =	2.06	ft	
Peak Outflow =	0.07	cfs	

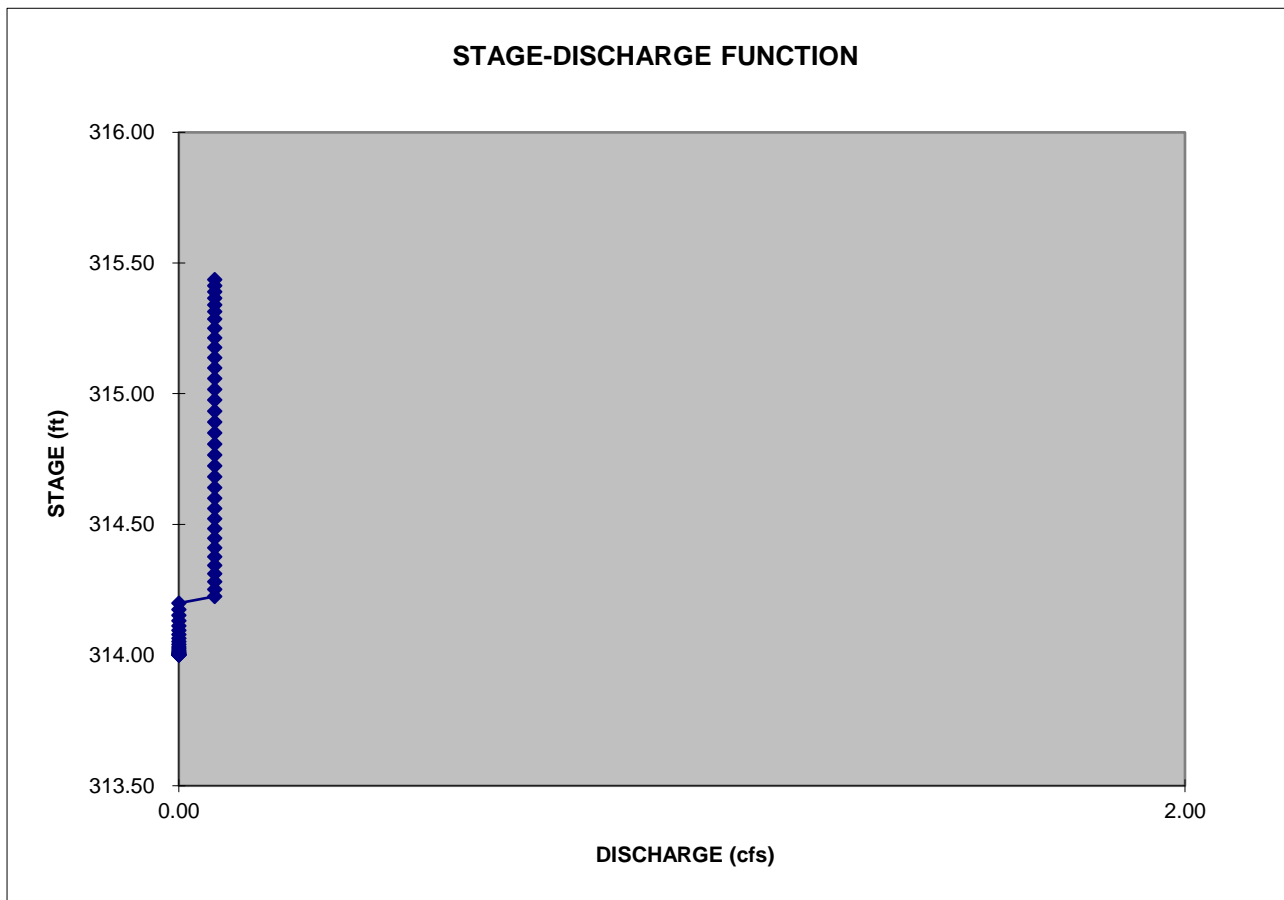
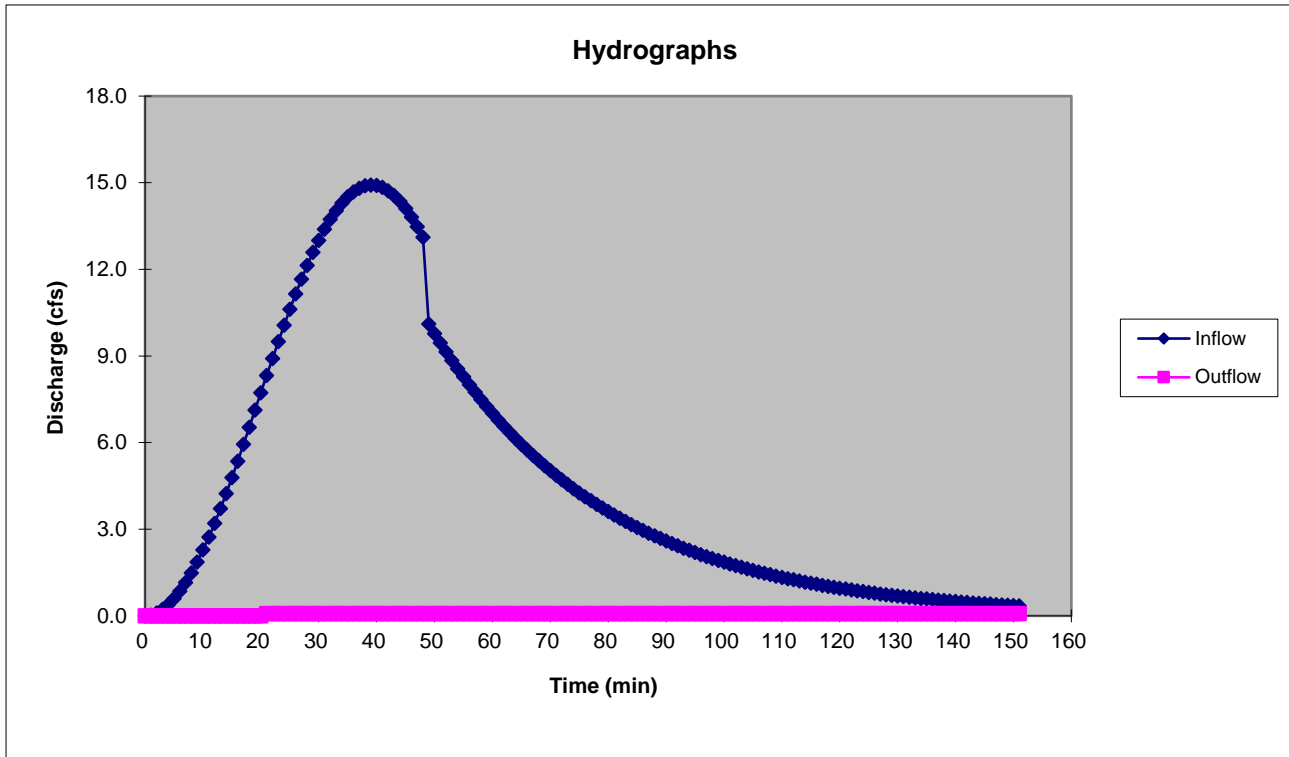
INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 320.00 ft	
Qp = 14.92 cfs	N = 1		
Tp = 39.1 min	L = 16 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 318.00 ft	<b>Skimmer Orifice:</b>	
Ks = 19172		Number = 1	Ea
b = 1.12		Diameter = 2.50	Inches
Z <sub>0</sub> = 314.0 ft (inv)		Head = 2.35	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 314.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	314.00	0.0	0.0	0.0		
1	0	0.00	314.00	0.0	0.0	0.00		
2	0	1.44	314.00	0.0	0.0	0.00		
3	0	7.20	314.00	0.0	0.0	0.00		
4	0	20.12	314.00	0.0	0.0	0.00		
5	1	43.00	314.00	0.0	0.0	0.00		
6	1	78.58	314.01	0.0	0.0	0.00		
7	1	129.51	314.01	0.0	0.0	0.00		
8	1	198.35	314.02	0.0	0.0	0.00		
9	2	287.54	314.02	0.0	0.0	0.00		
10	2	399.38	314.03	0.0	0.0	0.00		
11	3	536.04	314.04	0.0	0.0	0.00		
12	3	699.53	314.05	0.0	0.0	0.00		
13	4	891.66	314.06	0.0	0.0	0.00		
14	4	1114.09	314.08	0.0	0.0	0.00		
15	5	1368.27	314.09	0.0	0.0	0.00		
16	5	1655.45	314.11	0.0	0.0	0.00		
17	6	1976.65	314.13	0.0	0.0	0.00		
18	7	2332.69	314.15	0.0	0.0	0.00		
19	7	2724.15	314.17	0.0	0.0	0.00		
20	8	3151.41	314.20	0.0	0.0	0.00		
21	8	3614.60	314.22	0.1	0.0	0.07		
22	9	4109.31	314.25	0.1	0.0	0.07		
23	9	4639.52	314.28	0.1	0.0	0.07		
24	10	5204.65	314.31	0.1	0.0	0.07		
25	11	5803.93	314.34	0.1	0.0	0.07		
26	11	6436.34	314.38	0.1	0.0	0.07		
27	12	7100.68	314.41	0.1	0.0	0.07		
28	12	7795.51	314.45	0.1	0.0	0.07		
29	13	8519.21	314.48	0.1	0.0	0.07		
30	13	9269.98	314.52	0.1	0.0	0.07		
31	13	10045.83	314.56	0.1	0.0	0.07		
32	14	10844.63	314.60	0.1	0.0	0.07		
33	14	11664.07	314.64	0.1	0.0	0.07		
34	14	12501.75	314.68	0.1	0.0	0.07		
35	15	13355.11	314.72	0.1	0.0	0.07		
36	15	14221.51	314.77	0.1	0.0	0.07		
37	15	15098.24	314.81	0.1	0.0	0.07		
38	15	15982.48	314.85	0.1	0.0	0.07		
39	15	16871.41	314.89	0.1	0.0	0.07		
40	15	17762.15	314.93	0.1	0.0	0.07		
41	15	18651.82	314.98	0.1	0.0	0.07		
42	15	19537.54	315.02	0.1	0.0	0.07		
43	15	20416.46	315.06	0.1	0.0	0.07		
44	14	21285.78	315.10	0.1	0.0	0.07		
45	14	22142.75	315.14	0.1	0.0	0.07		
46	14	22984.69	315.18	0.1	0.0	0.07		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	13	23809.06	315.21	0.1	0.0	0.07		
48	13	24613.38	315.25	0.1	0.0	0.07		
49	10	25395.33	315.29	0.1	0.0	0.07		
50	10	25997.07	315.31	0.1	0.0	0.07		
51	9	26579.01	315.34	0.1	0.0	0.07		
52	9	27141.79	315.37	0.1	0.0	0.07		
53	9	27686.04	315.39	0.1	0.0	0.07		
54	9	28212.37	315.41	0.1	0.0	0.07		
55	8	28721.35	315.44	0.1	0.0	0.07		
56	8	29213.57	315.46	0.1	0.0	0.07		
57	8	29689.55	315.48	0.1	0.0	0.07		
58	7	30149.85	315.50	0.1	0.0	0.07		
59	7	30594.96	315.52	0.1	0.0	0.07		
60	7	31025.39	315.54	0.1	0.0	0.07		
61	7	31441.61	315.56	0.1	0.0	0.07		
62	7	31844.09	315.58	0.1	0.0	0.07		
63	6	32233.28	315.59	0.1	0.0	0.07		
64	6	32609.61	315.61	0.1	0.0	0.07		
65	6	32973.50	315.63	0.1	0.0	0.07		
66	6	33325.36	315.64	0.1	0.0	0.07		
67	6	33665.58	315.66	0.1	0.0	0.07		
68	5	33994.54	315.67	0.1	0.0	0.07		
69	5	34312.62	315.68	0.1	0.0	0.07		
70	5	34620.16	315.70	0.1	0.0	0.07		
71	5	34917.51	315.71	0.1	0.0	0.07		
72	5	35205.00	315.72	0.1	0.0	0.07		
73	5	35482.96	315.74	0.1	0.0	0.07		
74	4	35751.70	315.75	0.1	0.0	0.07		
75	4	36011.51	315.76	0.1	0.0	0.07		
76	4	36262.69	315.77	0.1	0.0	0.07		
77	4	36505.53	315.78	0.1	0.0	0.07		
78	4	36740.29	315.79	0.1	0.0	0.07		
79	4	36967.24	315.80	0.1	0.0	0.07		
80	4	37186.63	315.81	0.1	0.0	0.07		
81	3	37398.71	315.82	0.1	0.0	0.07		
82	3	37603.72	315.83	0.1	0.0	0.07		
83	3	37801.90	315.84	0.1	0.0	0.07		
84	3	37993.45	315.85	0.1	0.0	0.07		
85	3	38178.61	315.85	0.1	0.0	0.07		
86	3	38357.58	315.86	0.1	0.0	0.07		
87	3	38530.55	315.87	0.1	0.0	0.07		
88	3	38697.74	315.88	0.1	0.0	0.07		
89	3	38859.32	315.88	0.1	0.0	0.07		
90	3	39015.48	315.89	0.1	0.0	0.07		
91	3	39166.40	315.90	0.1	0.0	0.07		
92	2	39312.25	315.90	0.1	0.0	0.07		
93	2	39453.19	315.91	0.1	0.0	0.07		
94	2	39589.39	315.91	0.1	0.0	0.07		
95	2	39720.99	315.92	0.1	0.0	0.07		
96	2	39848.15	315.93	0.1	0.0	0.07		
97	2	39971.02	315.93	0.1	0.0	0.07		
98	2	40089.74	315.94	0.1	0.0	0.07		
99	2	40204.43	315.94	0.1	0.0	0.07		
100	2	40315.23	315.95	0.1	0.0	0.07		
101	2	40422.28	315.95	0.1	0.0	0.07		
102	2	40525.69	315.96	0.1	0.0	0.07		
103	2	40625.57	315.96	0.1	0.0	0.07		
104	2	40722.06	315.96	0.1	0.0	0.07		
105	2	40815.25	315.97	0.1	0.0	0.07		
106	2	40905.25	315.97	0.1	0.0	0.07		
107	1	40992.18	315.98	0.1	0.0	0.07		
108	1	41076.12	315.98	0.1	0.0	0.07		
109	1	41157.18	315.98	0.1	0.0	0.07		
110	1	41235.45	315.99	0.1	0.0	0.07		
111	1	41311.03	315.99	0.1	0.0	0.07		
112	1	41383.99	315.99	0.1	0.0	0.07		
113	1	41454.43	316.00	0.1	0.0	0.07		
114	1	41522.43	316.00	0.1	0.0	0.07		
115	1	41588.06	316.00	0.1	0.0	0.07		
116	1	41651.41	316.00	0.1	0.0	0.07		
117	1	41712.55	316.01	0.1	0.0	0.07		
118	1	41771.56	316.01	0.1	0.0	0.07		
119	1	41828.49	316.01	0.1	0.0	0.07		
120	1	41883.42	316.01	0.1	0.0	0.07		
121	1	41936.42	316.02	0.1	0.0	0.07		
122	1	41987.55	316.02	0.1	0.0	0.07		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	42036.87	316.02	0.1	0.0	0.07		
124	1	42084.43	316.02	0.1	0.0	0.07		
125	1	42130.30	316.02	0.1	0.0	0.07		
126	1	42174.53	316.03	0.1	0.0	0.07		
127	1	42217.18	316.03	0.1	0.0	0.07		
128	1	42258.29	316.03	0.1	0.0	0.07		
129	1	42297.92	316.03	0.1	0.0	0.07		
130	1	42336.11	316.03	0.1	0.0	0.07		
131	1	42372.92	316.03	0.1	0.0	0.07		
132	1	42408.38	316.04	0.1	0.0	0.07		
133	1	42442.54	316.04	0.1	0.0	0.07		
134	1	42475.45	316.04	0.1	0.0	0.07		
135	1	42507.14	316.04	0.1	0.0	0.07		
136	1	42537.66	316.04	0.1	0.0	0.07		
137	1	42567.04	316.04	0.1	0.0	0.07		
138	1	42595.31	316.04	0.1	0.0	0.07		
139	1	42622.53	316.05	0.1	0.0	0.07		
140	0	42648.71	316.05	0.1	0.0	0.07		
141	0	42673.90	316.05	0.1	0.0	0.07		
142	0	42698.13	316.05	0.1	0.0	0.07		
143	0	42721.43	316.05	0.1	0.0	0.07		
144	0	42743.82	316.05	0.1	0.0	0.07		
145	0	42765.34	316.05	0.1	0.0	0.07		
146	0	42786.02	316.05	0.1	0.0	0.07		
147	0	42805.88	316.05	0.1	0.0	0.07		
148	0	42824.95	316.05	0.1	0.0	0.07		
149	0	42843.26	316.06	0.1	0.0	0.07		
150	0	42860.84	316.06	0.1	0.0	0.07		
151	0	42877.69	<b>316.06</b>	0.1	0.0	0.07		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.78	ft	
Peak Stage =	316.22	ft	
Rise =	2.22	ft	
Peak Outflow =	0.07	cfs	

INPUT					
<b>Hydrograph Results:</b>		<b>Weir:</b>		<b>Top of Dam:</b> 320.00 ft	
Qp =	17.11 cfs	N =	1		
Tp =	39.1 min	L =	16 ft		
dT =	1.0 min	Cw =	3.00		
<b>Stage-Storage Results:</b>		Zcr =	318.00 ft	<b>Skimmer Orifice:</b>	
Ks =	19172			Number =	1.00 Ea
b =	1.12			Diameter =	2.50 Inches
Z <sub>0</sub> =	314.0 ft (inv)			Head =	2.35 inches
<b>Initial Water Level:</b>					
Z <sub>i</sub> =	314.00 ft				

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	314.00	0.0	0.0	0.0		
1	0	0.00	314.00	0.0	0.0	0.00		
2	0	1.65	314.00	0.0	0.0	0.00		
3	0	8.26	314.00	0.0	0.0	0.00		
4	0	23.08	314.00	0.0	0.0	0.00		
5	1	49.33	314.00	0.0	0.0	0.00		
6	1	90.15	314.01	0.0	0.0	0.00		
7	1	148.58	314.01	0.0	0.0	0.00		
8	2	227.56	314.02	0.0	0.0	0.00		
9	2	329.88	314.03	0.0	0.0	0.00		
10	3	458.19	314.04	0.0	0.0	0.00		
11	3	614.98	314.05	0.0	0.0	0.00		
12	4	802.53	314.06	0.0	0.0	0.00		
13	4	1022.96	314.07	0.0	0.0	0.00		
14	5	1278.15	314.09	0.0	0.0	0.00		
15	5	1569.76	314.11	0.0	0.0	0.00		
16	6	1899.22	314.13	0.0	0.0	0.00		
17	7	2267.71	314.15	0.0	0.0	0.00		
18	7	2676.18	314.17	0.0	0.0	0.00		
19	8	3125.29	314.20	0.0	0.0	0.00		
20	9	3615.47	314.22	0.1	0.0	0.07		
21	10	4142.57	314.25	0.1	0.0	0.07		
22	10	4710.77	314.28	0.1	0.0	0.07		
23	11	5319.68	314.32	0.1	0.0	0.07		
24	12	5968.66	314.35	0.1	0.0	0.07		
25	12	6656.81	314.39	0.1	0.0	0.07		
26	13	7382.98	314.43	0.1	0.0	0.07		
27	13	8145.77	314.46	0.1	0.0	0.07		
28	14	8943.55	314.51	0.1	0.0	0.07		
29	14	9774.45	314.55	0.1	0.0	0.07		
30	15	10636.40	314.59	0.1	0.0	0.07		
31	15	11527.13	314.63	0.1	0.0	0.07		
32	16	12444.18	314.68	0.1	0.0	0.07		
33	16	13384.93	314.72	0.1	0.0	0.07		
34	16	14346.58	314.77	0.1	0.0	0.07		
35	17	15326.23	314.82	0.1	0.0	0.07		
36	17	16320.85	314.87	0.1	0.0	0.07		
37	17	17327.30	314.91	0.1	0.0	0.07		
38	17	18342.39	314.96	0.1	0.0	0.07		
39	17	19362.85	315.01	0.1	0.0	0.07		
40	17	20385.38	315.06	0.1	0.0	0.07		
41	17	21406.69	315.10	0.1	0.0	0.07		
42	17	22423.47	315.15	0.1	0.0	0.07		
43	17	23432.44	315.20	0.1	0.0	0.07		
44	16	24430.40	315.24	0.1	0.0	0.07		
45	16	25414.19	315.29	0.1	0.0	0.07		
46	16	26380.74	315.33	0.1	0.0	0.07		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	15	27327.13	315.37	0.1	0.0	0.07		
48	15	28250.52	315.42	0.1	0.0	0.07		
49	10	29148.25	315.46	0.1	0.0	0.07		
50	10	29749.99	315.48	0.1	0.0	0.07		
51	9	30331.93	315.51	0.1	0.0	0.07		
52	9	30894.71	315.53	0.1	0.0	0.07		
53	9	31438.96	315.56	0.1	0.0	0.07		
54	9	31965.28	315.58	0.1	0.0	0.07		
55	8	32474.27	315.60	0.1	0.0	0.07		
56	8	32966.48	315.63	0.1	0.0	0.07		
57	8	33442.47	315.65	0.1	0.0	0.07		
58	7	33902.76	315.67	0.1	0.0	0.07		
59	7	34347.88	315.69	0.1	0.0	0.07		
60	7	34778.30	315.70	0.1	0.0	0.07		
61	7	35194.52	315.72	0.1	0.0	0.07		
62	7	35597.00	315.74	0.1	0.0	0.07		
63	6	35986.19	315.76	0.1	0.0	0.07		
64	6	36362.52	315.77	0.1	0.0	0.07		
65	6	36726.41	315.79	0.1	0.0	0.07		
66	6	37078.27	315.81	0.1	0.0	0.07		
67	6	37418.50	315.82	0.1	0.0	0.07		
68	5	37747.46	315.83	0.1	0.0	0.07		
69	5	38065.53	315.85	0.1	0.0	0.07		
70	5	38373.08	315.86	0.1	0.0	0.07		
71	5	38670.43	315.87	0.1	0.0	0.07		
72	5	38957.92	315.89	0.1	0.0	0.07		
73	5	39235.88	315.90	0.1	0.0	0.07		
74	4	39504.61	315.91	0.1	0.0	0.07		
75	4	39764.43	315.92	0.1	0.0	0.07		
76	4	40015.61	315.93	0.1	0.0	0.07		
77	4	40258.45	315.94	0.1	0.0	0.07		
78	4	40493.21	315.95	0.1	0.0	0.07		
79	4	40720.15	315.96	0.1	0.0	0.07		
80	4	40939.55	315.97	0.1	0.0	0.07		
81	3	41151.63	315.98	0.1	0.0	0.07		
82	3	41356.64	315.99	0.1	0.0	0.07		
83	3	41554.81	316.00	0.1	0.0	0.07		
84	3	41746.37	316.01	0.1	0.0	0.07		
85	3	41931.53	316.02	0.1	0.0	0.07		
86	3	42110.49	316.02	0.1	0.0	0.07		
87	3	42283.47	316.03	0.1	0.0	0.07		
88	3	42450.66	316.04	0.1	0.0	0.07		
89	3	42612.24	316.05	0.1	0.0	0.07		
90	3	42768.40	316.05	0.1	0.0	0.07		
91	3	42919.32	316.06	0.1	0.0	0.07		
92	2	43065.17	316.06	0.1	0.0	0.07		
93	2	43206.11	316.07	0.1	0.0	0.07		
94	2	43342.30	316.08	0.1	0.0	0.07		
95	2	43473.91	316.08	0.1	0.0	0.07		
96	2	43601.07	316.09	0.1	0.0	0.07		
97	2	43723.94	316.09	0.1	0.0	0.07		
98	2	43842.65	316.10	0.1	0.0	0.07		
99	2	43957.35	316.10	0.1	0.0	0.07		
100	2	44068.15	316.11	0.1	0.0	0.07		
101	2	44175.20	316.11	0.1	0.0	0.07		
102	2	44278.60	316.12	0.1	0.0	0.07		
103	2	44378.49	316.12	0.1	0.0	0.07		
104	2	44474.97	316.13	0.1	0.0	0.07		
105	2	44568.16	316.13	0.1	0.0	0.07		
106	2	44658.17	316.13	0.1	0.0	0.07		
107	1	44745.09	316.14	0.1	0.0	0.07		
108	1	44829.04	316.14	0.1	0.0	0.07		
109	1	44910.10	316.14	0.1	0.0	0.07		
110	1	44988.37	316.15	0.1	0.0	0.07		
111	1	45063.94	316.15	0.1	0.0	0.07		
112	1	45136.91	316.15	0.1	0.0	0.07		
113	1	45207.35	316.16	0.1	0.0	0.07		
114	1	45275.34	316.16	0.1	0.0	0.07		
115	1	45340.98	316.16	0.1	0.0	0.07		
116	1	45404.33	316.16	0.1	0.0	0.07		
117	1	45465.47	316.17	0.1	0.0	0.07		
118	1	45524.47	316.17	0.1	0.0	0.07		
119	1	45581.41	316.17	0.1	0.0	0.07		
120	1	45636.34	316.17	0.1	0.0	0.07		
121	1	45689.34	316.18	0.1	0.0	0.07		
122	1	45740.47	316.18	0.1	0.0	0.07		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	45789.78	316.18	0.1	0.0	0.07		
124	1	45837.35	316.18	0.1	0.0	0.07		
125	1	45883.22	316.19	0.1	0.0	0.07		
126	1	45927.45	316.19	0.1	0.0	0.07		
127	1	45970.09	316.19	0.1	0.0	0.07		
128	1	46011.20	316.19	0.1	0.0	0.07		
129	1	46050.83	316.19	0.1	0.0	0.07		
130	1	46089.03	316.19	0.1	0.0	0.07		
131	1	46125.83	316.20	0.1	0.0	0.07		
132	1	46161.29	316.20	0.1	0.0	0.07		
133	1	46195.46	316.20	0.1	0.0	0.07		
134	1	46228.36	316.20	0.1	0.0	0.07		
135	1	46260.06	316.20	0.1	0.0	0.07		
136	1	46290.57	316.20	0.1	0.0	0.07		
137	1	46319.95	316.20	0.1	0.0	0.07		
138	1	46348.23	316.21	0.1	0.0	0.07		
139	1	46375.45	316.21	0.1	0.0	0.07		
140	0	46401.63	316.21	0.1	0.0	0.07		
141	0	46426.82	316.21	0.1	0.0	0.07		
142	0	46451.05	316.21	0.1	0.0	0.07		
143	0	46474.34	316.21	0.1	0.0	0.07		
144	0	46496.73	316.21	0.1	0.0	0.07		
145	0	46518.26	316.21	0.1	0.0	0.07		
146	0	46538.93	316.21	0.1	0.0	0.07		
147	0	46558.80	316.21	0.1	0.0	0.07		
148	0	46577.87	316.21	0.1	0.0	0.07		
149	0	46596.18	316.22	0.1	0.0	0.07		
150	0	46613.75	316.22	0.1	0.0	0.07		
151	0	46630.61	316.22	0.1	0.0	0.07		

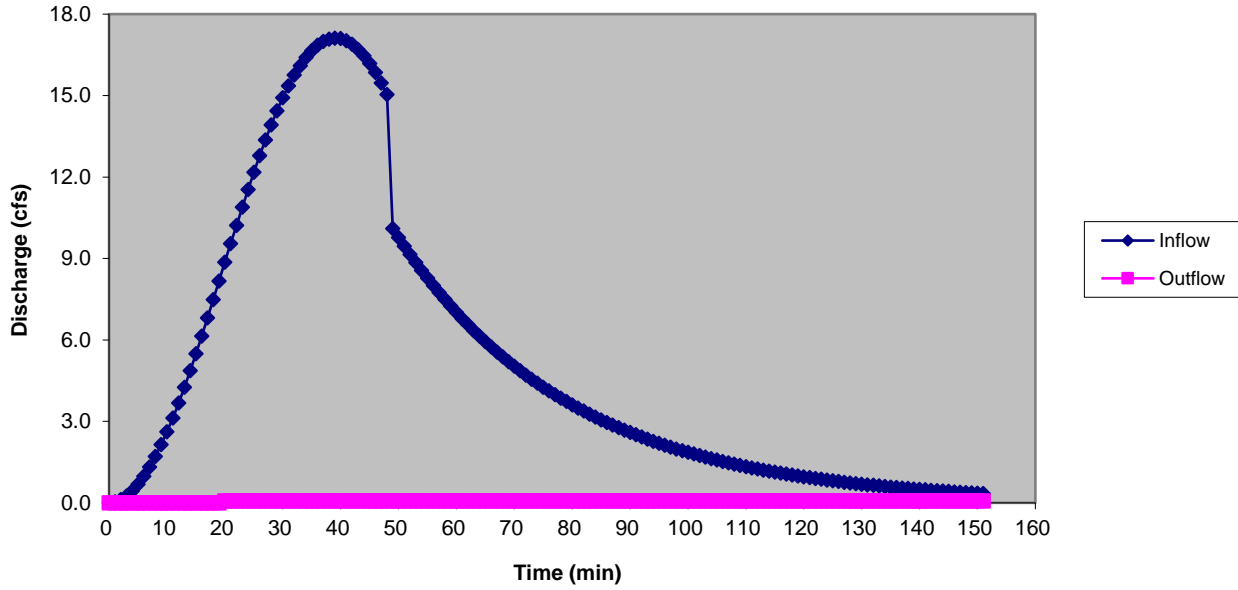




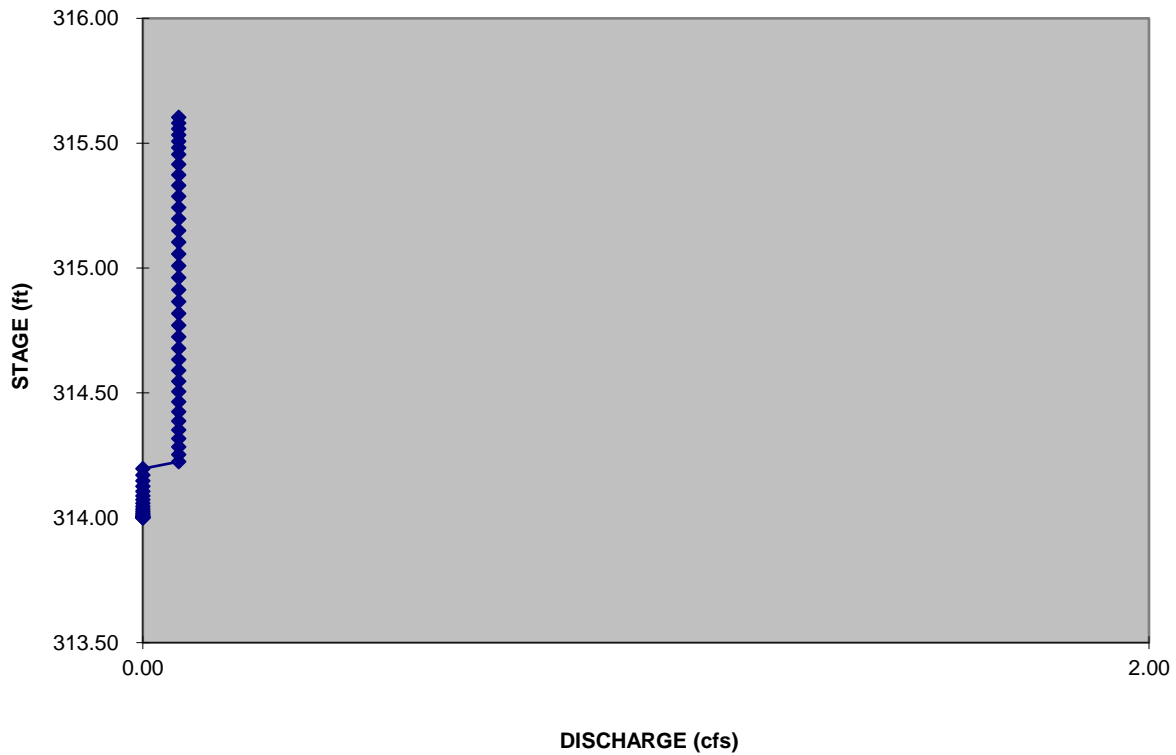
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

Hydrographs



STAGE-DISCHARGE FUNCTION





**SCM-B  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID SCM-B

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	12.24 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	0.50 ac.	0.25
Undisturbed grassy area	0.00 ac.	11.74 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>12.24 ac.</b>	<b>12.24 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	1476 feet		
Height of watershed =	29 feet		
Calculated t(c) =	9.8 minutes		
Minimum t(c) =	5.0 minutes		
Time of concentration =	9.8 minutes		

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	4.75 in/hr
10	195	22	6.14 in/hr
25	232	23	7.08 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	4.75 in/hr	26.2 cfs	20.12 cfs
10-year storm	6.14 in/hr	33.8 cfs	25.98 cfs
25-year storm	7.08 in/hr	39.0 cfs	29.97 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	245
Bottom width =	90
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	24
Height of berm =	6
Top of trap length =	281
Top of trap width =	126
Bottom elevation =	310

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	44064 cu. ft.	104970 cu. ft.	
Sediment surface area =	14700 sq. ft.	30670 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	2.7L:1W	
Spillway length =	10.0 ft. (min)	24	
10-Year flow depth over spillway =	0.50 ft. (max)	0.02 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.98 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 44,064 cubic feet

24 HOURS	Best Option =	1 - 6 inch Skimmers with a 5.6 inch orifice
2 DAYS	Best Option =	1 - 5 inch Skimmers with a 4.1 inch orifice
3 DAYS	Best Option =	1 - 4 inch Skimmers with a 3.4 inch orifice
4 DAYS	Best Option =	1 - 4 inch Skimmers with a 3 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
310	22976		0	0			
311	25208	24092	24092	1	10.0896	0.0000	1.01
312	27506	26357	50449	2	10.8287	0.6931	1.97
313	29870	28688	79137	3	11.2789	1.0986	2.96
314	32301	31085.5	110222.5	4	11.6103	1.3863	4.00
315	34797	33549	143771.5	5	11.8760	1.6094	5.08

**STAGE-STORAGE FUNCTION:**

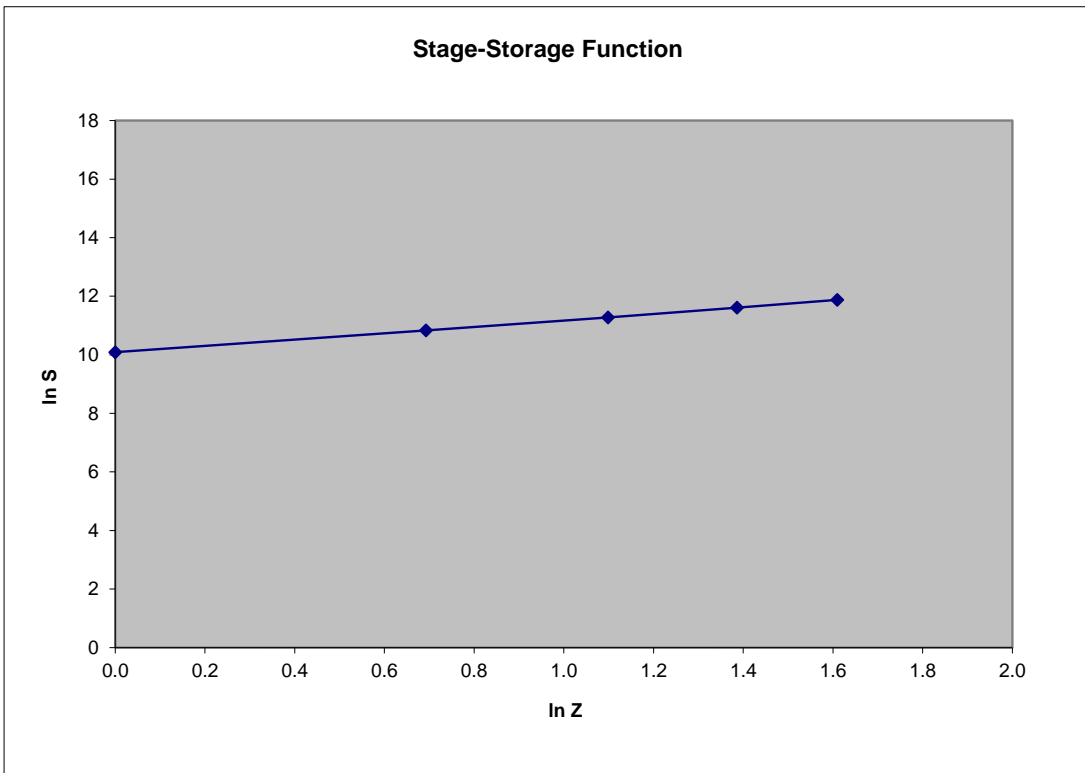
$$S = K_s * Z^b$$

where:

$b = 1.11$

and

$K_s = 23773$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	12.24 ac	<b>Use Kirpich Equation:</b> Tc = 9.8 min
Disturbed Area =	12.24 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 4.75 in/hr i <sub>10</sub> = 6.14 in/hr i <sub>25</sub> = 7.08 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	1476 ft	
Vertical Fall =	29 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 26.2 cfs Q <sub>10</sub> = 33.8 cfs Q <sub>25</sub> = 39.0 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 35.6 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 46.0 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 49.1 minutes



# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

### COMPUTED RESULTS

Freeboard =	3.16	ft		
	Peak Stage =	312.84	ft	
	Rise =	2.84	ft	
		Peak Outflow =	0.22	cfs

### INPUT

#### Hydrograph Results:

Qp = 26.18 cfs  
 Tp = 35.6 min  
 dT = 1.0 min

#### Weir:

N = 1  
 L = 24 ft  
 Cw = 3.00  
 Zcr = 314.00 ft

Top of Dam: 316.00 ft

#### Stage-Storage Results:

Ks = 23773  
 b = 1.11  
 Z<sub>0</sub> = 310.0 ft (inv)

#### Skimmer Orifice:

Number = 1.00 Ea  
 Diameter = 4.00 Inches  
 Head = 3.42 inches

#### Initial Water Level:

Z<sub>i</sub> = 310.00 ft

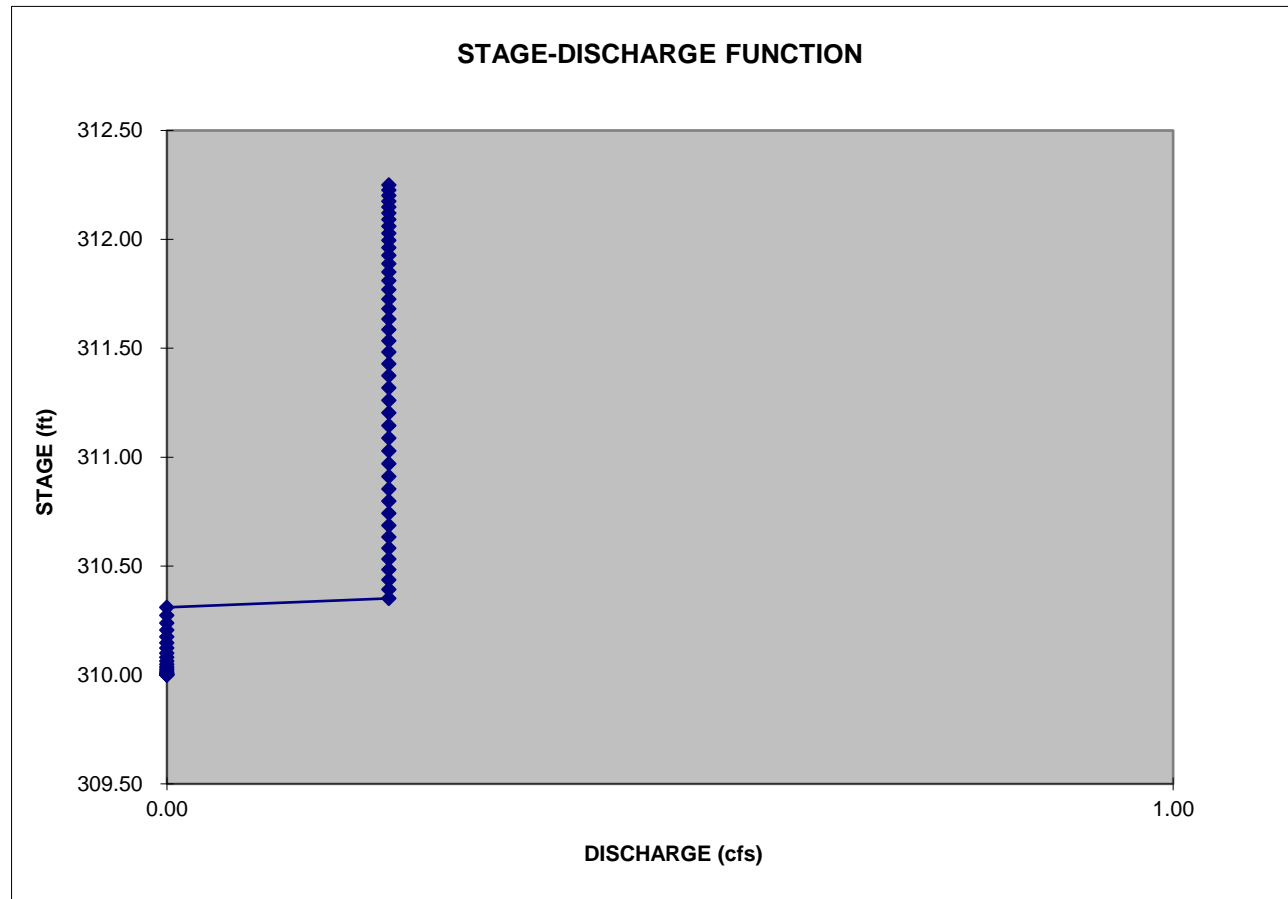
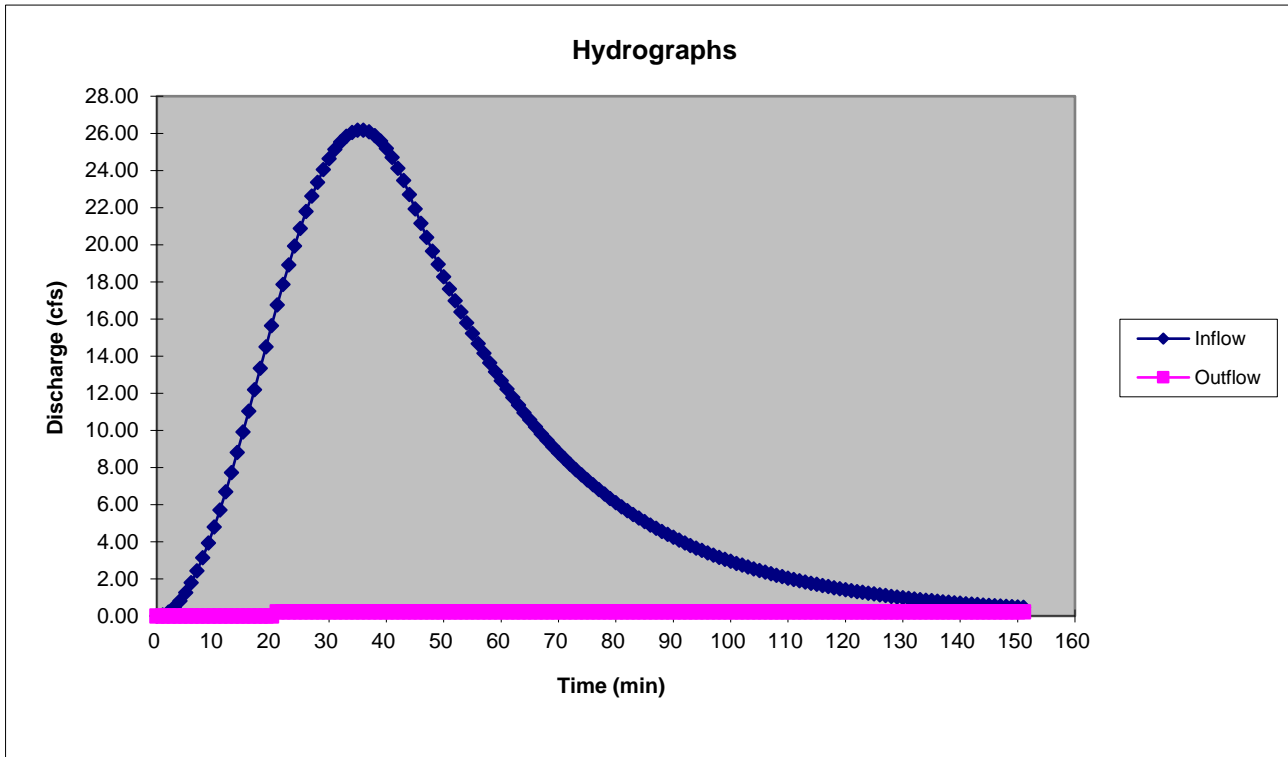
### OUTPUT

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	3.06	310.00	0.0	0.0	0.00		
3	0	15.29	310.00	0.0	0.0	0.00		
4	1	42.71	310.00	0.0	0.0	0.00		
5	1	91.23	310.01	0.0	0.0	0.00		
6	2	166.60	310.01	0.0	0.0	0.00		
7	2	274.36	310.02	0.0	0.0	0.00		
8	3	419.79	310.03	0.0	0.0	0.00		
9	4	607.89	310.04	0.0	0.0	0.00		
10	5	843.30	310.05	0.0	0.0	0.00		
11	6	1130.32	310.06	0.0	0.0	0.00		
12	7	1472.84	310.08	0.0	0.0	0.00		
13	8	1874.31	310.10	0.0	0.0	0.00		
14	9	2337.72	310.12	0.0	0.0	0.00		
15	10	2865.58	310.15	0.0	0.0	0.00		
16	11	3459.91	310.18	0.0	0.0	0.00		
17	12	4122.18	310.21	0.0	0.0	0.00		
18	13	4853.37	310.24	0.0	0.0	0.00		
19	14	5653.89	310.27	0.0	0.0	0.00		
20	16	6523.62	310.31	0.0	0.0	0.00		
21	17	7461.91	310.35	0.2	0.0	0.22		
22	18	8454.34	310.39	0.2	0.0	0.22		
23	19	9512.41	310.44	0.2	0.0	0.22		
24	20	10633.89	310.48	0.2	0.0	0.22		
25	21	11816.06	310.53	0.2	0.0	0.22		
26	22	13055.73	310.58	0.2	0.0	0.22		
27	23	14349.24	310.63	0.2	0.0	0.22		
28	23	15692.53	310.69	0.2	0.0	0.22		
29	24	17081.15	310.74	0.2	0.0	0.22		
30	25	18510.28	310.80	0.2	0.0	0.22		
31	25	19974.80	310.85	0.2	0.0	0.22		
32	26	21469.32	310.91	0.2	0.0	0.22		
33	26	22988.19	310.97	0.2	0.0	0.22		
34	26	24525.59	311.03	0.2	0.0	0.22		
35	26	26075.57	311.09	0.2	0.0	0.22		
36	26	27632.03	311.15	0.2	0.0	0.22		
37	26	29188.89	311.20	0.2	0.0	0.22		
38	26	30740.00	311.26	0.2	0.0	0.22		
39	26	32279.29	311.32	0.2	0.0	0.22		
40	25	33800.79	311.37	0.2	0.0	0.22		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.98	ft	
Peak Stage =	314.02	ft	
Rise =	4.02	ft	
Peak Outflow =	0.50	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft	
Qp = 33.80 cfs	N = 1		
Tp = 46.0 min	L = 24 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	<b>Skimmer Orifice:</b>	
Ks = 23773		Number = 1	Ea
b = 1.11		Diameter = 4.00	Inches
Z <sub>0</sub> = 310.0 ft (inv)		Head = 3.42	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

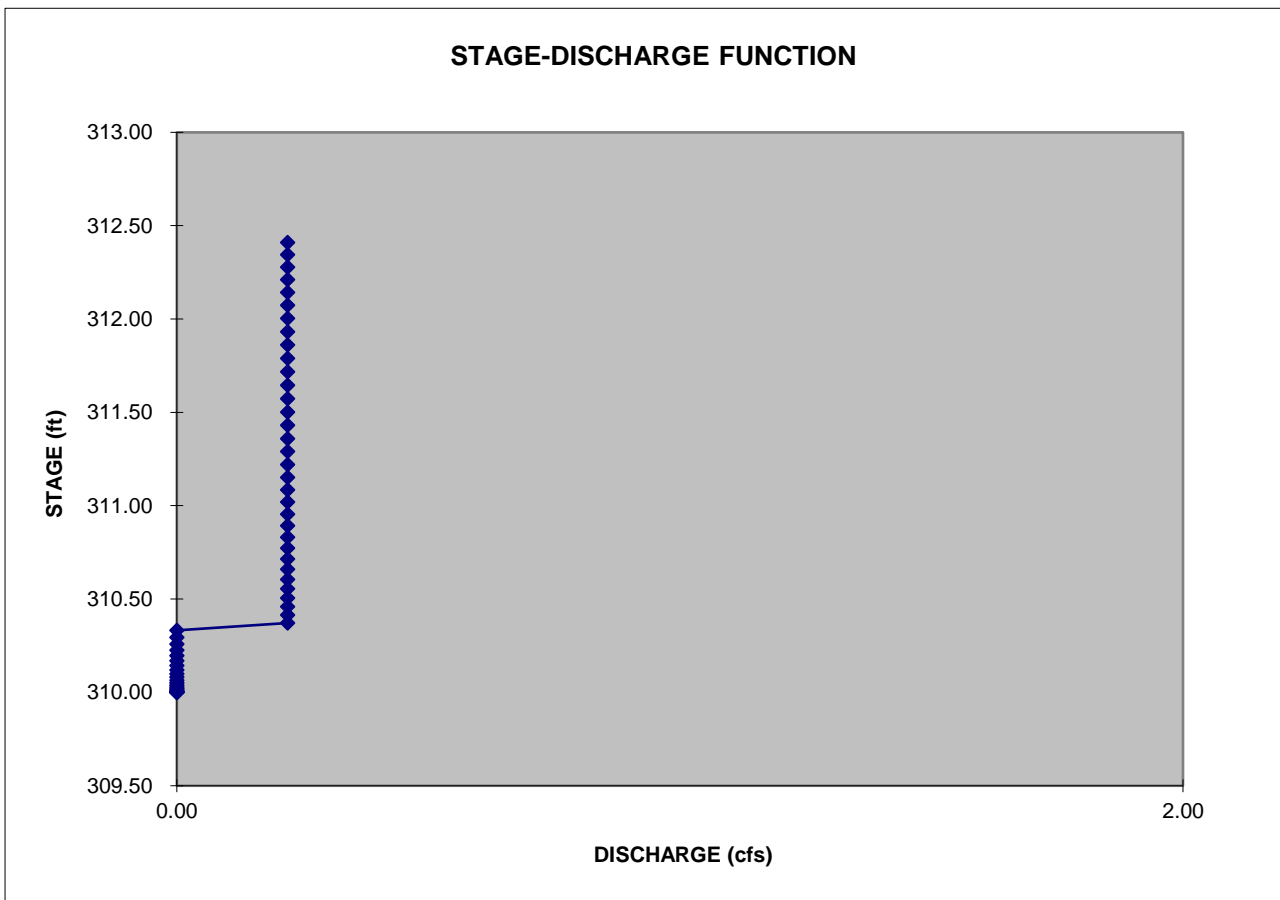
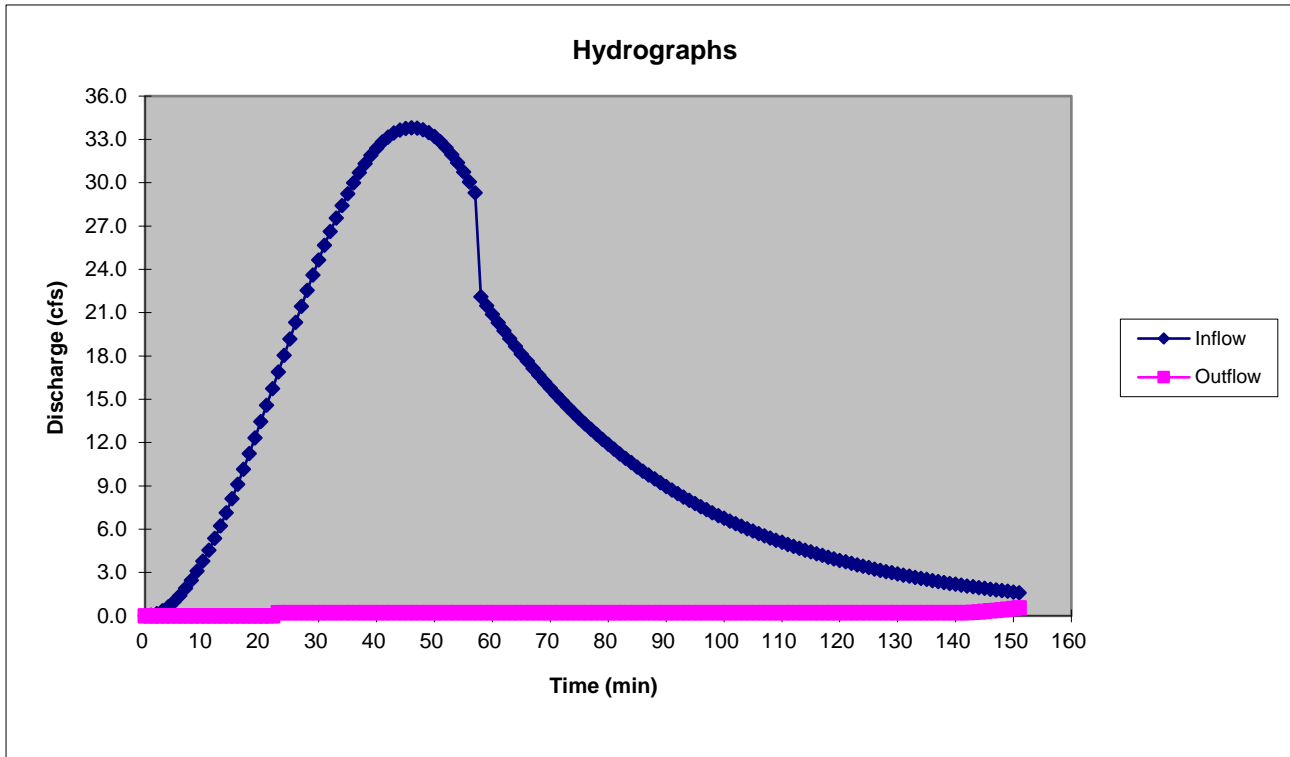
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T	Inflow I	Storage S	Stage Z	Outflow O	Weir Flow	Skimmer Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	2.36	310.00	0.0	0.0	0.00		
3	0	11.79	310.00	0.0	0.0	0.00		
4	1	32.95	310.00	0.0	0.0	0.00		
5	1	70.48	310.01	0.0	0.0	0.00		
6	1	128.92	310.01	0.0	0.0	0.00		
7	2	212.71	310.01	0.0	0.0	0.00		
8	2	326.18	310.02	0.0	0.0	0.00		
9	3	473.52	310.03	0.0	0.0	0.00		
10	4	658.77	310.04	0.0	0.0	0.00		
11	5	885.78	310.05	0.0	0.0	0.00		
12	5	1158.22	310.07	0.0	0.0	0.00		
13	6	1479.53	310.08	0.0	0.0	0.00		
14	7	1852.94	310.10	0.0	0.0	0.00		
15	8	2281.43	310.12	0.0	0.0	0.00		
16	9	2767.72	310.14	0.0	0.0	0.00		
17	10	3314.28	310.17	0.0	0.0	0.00		
18	11	3923.27	310.20	0.0	0.0	0.00		
19	12	4596.58	310.23	0.0	0.0	0.00		
20	13	5335.80	310.26	0.0	0.0	0.00		
21	15	6142.21	310.29	0.0	0.0	0.00		
22	16	7016.77	310.33	0.0	0.0	0.00		
23	17	7960.13	310.37	0.2	0.0	0.22		
24	18	8959.39	310.41	0.2	0.0	0.22		
25	19	10027.79	310.46	0.2	0.0	0.22		
26	20	11165.02	310.51	0.2	0.0	0.22		
27	21	12370.43	310.55	0.2	0.0	0.22		
28	23	13643.09	310.61	0.2	0.0	0.22		
29	24	14981.72	310.66	0.2	0.0	0.22		
30	25	16384.74	310.71	0.2	0.0	0.22		
31	26	17850.30	310.77	0.2	0.0	0.22		
32	27	19376.23	310.83	0.2	0.0	0.22		
33	28	20960.09	310.89	0.2	0.0	0.22		
34	28	22599.15	310.96	0.2	0.0	0.22		
35	29	24290.46	311.02	0.2	0.0	0.22		
36	30	26030.80	311.09	0.2	0.0	0.22		
37	31	27816.73	311.15	0.2	0.0	0.22		
38	31	29644.60	311.22	0.2	0.0	0.22		
39	32	31510.55	311.29	0.2	0.0	0.22		
40	32	33410.57	311.36	0.2	0.0	0.22		
41	33	35340.47	311.43	0.2	0.0	0.22		
42	33	37295.92	311.50	0.2	0.0	0.22		
43	33	39272.49	311.57	0.2	0.0	0.22		
44	34	41265.63	311.65	0.2	0.0	0.22		
45	34	43270.74	311.72	0.2	0.0	0.22		
46	34	45283.13	311.79	0.2	0.0	0.22		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	34	47298.09	311.86	0.2	0.0	0.22		
48	34	49310.92	311.93	0.2	0.0	0.22		
49	33	51316.90	312.00	0.2	0.0	0.22		
50	33	53311.35	312.07	0.2	0.0	0.22		
51	33	55289.65	312.14	0.2	0.0	0.22		
52	32	57247.26	312.21	0.2	0.0	0.22		
53	32	59179.72	312.28	0.2	0.0	0.22		
54	31	61082.69	312.34	0.2	0.0	0.22		
55	31	62951.99	312.41	0.2	0.0	0.22		
56	30	64783.56	312.47	0.2	0.0	0.22		
57	29	66573.54	312.53	0.2	0.0	0.22		
58	22	68318.27	312.59	0.2	0.0	0.22		
59	21	69630.69	312.64	0.2	0.0	0.22		
60	21	70906.20	312.68	0.2	0.0	0.22		
61	20	72145.84	312.72	0.2	0.0	0.22		
62	20	73350.60	312.77	0.2	0.0	0.22		
63	19	74521.45	312.81	0.2	0.0	0.22		
64	19	75659.34	312.84	0.2	0.0	0.22		
65	18	76765.19	312.88	0.2	0.0	0.22		
66	18	77839.89	312.92	0.2	0.0	0.22		
67	17	78884.30	312.95	0.2	0.0	0.22		
68	17	79899.27	312.99	0.2	0.0	0.22		
69	16	80885.61	313.02	0.2	0.0	0.22		
70	16	81844.13	313.05	0.2	0.0	0.22		
71	15	82775.60	313.08	0.2	0.0	0.22		
72	15	83680.77	313.12	0.2	0.0	0.22		
73	14	84560.38	313.14	0.2	0.0	0.22		
74	14	85415.13	313.17	0.2	0.0	0.22		
75	14	86245.72	313.20	0.2	0.0	0.22		
76	13	87052.82	313.23	0.2	0.0	0.22		
77	13	87837.08	313.25	0.2	0.0	0.22		
78	13	88599.14	313.28	0.2	0.0	0.22		
79	12	89339.62	313.30	0.2	0.0	0.22		
80	12	90059.12	313.33	0.2	0.0	0.22		
81	12	90758.22	313.35	0.2	0.0	0.22		
82	11	91437.49	313.37	0.2	0.0	0.22		
83	11	92097.49	313.40	0.2	0.0	0.22		
84	11	92738.74	313.42	0.2	0.0	0.22		
85	10	93361.78	313.44	0.2	0.0	0.22		
86	10	93967.10	313.46	0.2	0.0	0.22		
87	10	94555.20	313.48	0.2	0.0	0.22		
88	9	95126.56	313.50	0.2	0.0	0.22		
89	9	95681.65	313.52	0.2	0.0	0.22		
90	9	96220.92	313.53	0.2	0.0	0.22		
91	9	96744.81	313.55	0.2	0.0	0.22		
92	8	97253.75	313.57	0.2	0.0	0.22		
93	8	97748.15	313.58	0.2	0.0	0.22		
94	8	98228.42	313.60	0.2	0.0	0.22		
95	8	98694.95	313.62	0.2	0.0	0.22		
96	8	99148.13	313.63	0.2	0.0	0.22		
97	7	99588.32	313.65	0.2	0.0	0.22		
98	7	100015.89	313.66	0.2	0.0	0.22		
99	7	100431.19	313.67	0.2	0.0	0.22		
100	7	100834.56	313.69	0.2	0.0	0.22		
101	7	101226.34	313.70	0.2	0.0	0.22		
102	6	101606.84	313.71	0.2	0.0	0.22		
103	6	101976.37	313.72	0.2	0.0	0.22		
104	6	102335.26	313.74	0.2	0.0	0.22		
105	6	102683.78	313.75	0.2	0.0	0.22		
106	6	103022.24	313.76	0.2	0.0	0.22		
107	6	103350.90	313.77	0.2	0.0	0.22		
108	5	103670.05	313.78	0.2	0.0	0.22		
109	5	103979.94	313.79	0.2	0.0	0.22		
110	5	104280.84	313.80	0.2	0.0	0.22		
111	5	104573.00	313.81	0.2	0.0	0.22		
112	5	104856.65	313.82	0.2	0.0	0.22		
113	5	105132.04	313.83	0.2	0.0	0.22		
114	5	105399.39	313.84	0.2	0.0	0.22		
115	4	105658.94	313.85	0.2	0.0	0.22		
116	4	105910.89	313.85	0.2	0.0	0.22		
117	4	106155.46	313.86	0.2	0.0	0.22		
118	4	106392.85	313.87	0.2	0.0	0.22		
119	4	106623.26	313.88	0.2	0.0	0.22		
120	4	106846.90	313.88	0.2	0.0	0.22		
121	4	107063.93	313.89	0.2	0.0	0.22		
122	4	107274.56	313.90	0.2	0.0	0.22		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	4	107478.96	313.91	0.2	0.0	0.22		
124	3	107677.30	313.91	0.2	0.0	0.22		
125	3	107869.75	313.92	0.2	0.0	0.22		
126	3	108056.47	313.92	0.2	0.0	0.22		
127	3	108237.63	313.93	0.2	0.0	0.22		
128	3	108413.38	313.94	0.2	0.0	0.22		
129	3	108583.86	313.94	0.2	0.0	0.22		
130	3	108749.23	313.95	0.2	0.0	0.22		
131	3	108909.63	313.95	0.2	0.0	0.22		
132	3	109065.20	313.96	0.2	0.0	0.22		
133	3	109216.07	313.96	0.2	0.0	0.22		
134	3	109362.37	313.97	0.2	0.0	0.22		
135	3	109504.22	313.97	0.2	0.0	0.22		
136	2	109641.76	313.98	0.2	0.0	0.22		
137	2	109775.11	313.98	0.2	0.0	0.22		
138	2	109904.37	313.98	0.2	0.0	0.22		
139	2	110029.67	313.99	0.2	0.0	0.22		
140	2	110151.11	313.99	0.2	0.0	0.22		
141	2	110268.80	314.00	0.2	0.0	0.22		
142	2	110382.85	314.00	0.2	0.0	0.22		
143	2	110493.31	314.00	0.2	0.0	0.22		
144	2	110599.23	314.01	0.3	0.0	0.22		
145	2	110700.10	314.01	0.3	0.1	0.22		
146	2	110795.66	314.01	0.3	0.1	0.22		
147	2	110885.80	314.02	0.4	0.2	0.22		
148	2	110970.49	314.02	0.4	0.2	0.22		
149	2	111049.76	314.02	0.5	0.2	0.22		
150	2	111123.69	314.02	0.5	0.3	0.22		
151	2	111192.39	<b>314.03</b>	0.5	0.3	0.22		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.87	ft	
Peak Stage =	314.13	ft	
Rise =	4.13	ft	
Peak Outflow =	3.77	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft	
Qp = 38.99 cfs	N = 1		
Tp = 46.0 min	L = 24 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	<b>Skimmer Orifice:</b>	
Ks = 23773		Number = 1.00	Ea
b = 1.11		Diameter = 4.00	Inches
Z <sub>0</sub> = 310.0 ft (inv)		Head = 3.42	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	2.72	310.00	0.0	0.0	0.00		
3	0	13.59	310.00	0.0	0.0	0.00		
4	1	38.01	310.00	0.0	0.0	0.00		
5	1	81.30	310.01	0.0	0.0	0.00		
6	2	148.70	310.01	0.0	0.0	0.00		
7	2	245.35	310.02	0.0	0.0	0.00		
8	3	376.23	310.02	0.0	0.0	0.00		
9	4	546.18	310.03	0.0	0.0	0.00		
10	4	759.85	310.04	0.0	0.0	0.00		
11	5	1021.70	310.06	0.0	0.0	0.00		
12	6	1335.94	310.07	0.0	0.0	0.00		
13	7	1706.55	310.09	0.0	0.0	0.00		
14	8	2137.25	310.11	0.0	0.0	0.00		
15	9	2631.49	310.14	0.0	0.0	0.00		
16	11	3192.40	310.16	0.0	0.0	0.00		
17	12	3822.82	310.19	0.0	0.0	0.00		
18	13	4525.26	310.22	0.0	0.0	0.00		
19	14	5301.89	310.26	0.0	0.0	0.00		
20	16	6154.54	310.30	0.0	0.0	0.00		
21	17	7084.68	310.34	0.2	0.0	0.22		
22	18	8080.20	310.38	0.2	0.0	0.22		
23	19	9155.09	310.42	0.2	0.0	0.22		
24	21	10309.71	310.47	0.2	0.0	0.22		
25	22	11544.08	310.52	0.2	0.0	0.22		
26	23	12857.83	310.57	0.2	0.0	0.22		
27	25	14250.24	310.63	0.2	0.0	0.22		
28	26	15720.20	310.69	0.2	0.0	0.22		
29	27	17266.26	310.75	0.2	0.0	0.22		
30	28	18886.60	310.81	0.2	0.0	0.22		
31	30	20579.07	310.88	0.2	0.0	0.22		
32	31	22341.17	310.95	0.2	0.0	0.22		
33	32	24170.08	311.02	0.2	0.0	0.22		
34	33	26062.68	311.09	0.2	0.0	0.22		
35	34	28015.53	311.16	0.2	0.0	0.22		
36	35	30024.95	311.23	0.2	0.0	0.22		
37	35	32086.94	311.31	0.2	0.0	0.22		
38	36	34197.31	311.39	0.2	0.0	0.22		
39	37	36351.61	311.47	0.2	0.0	0.22		
40	37	38545.20	311.55	0.2	0.0	0.22		
41	38	40773.25	311.63	0.2	0.0	0.22		
42	38	43030.79	311.71	0.2	0.0	0.22		
43	39	45312.68	311.79	0.2	0.0	0.22		
44	39	47613.68	311.87	0.2	0.0	0.22		
45	39	49928.48	311.95	0.2	0.0	0.22		
46	39	52251.68	312.04	0.2	0.0	0.22		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	39	54577.86	312.12	0.2	0.0	0.22		
48	39	56901.57	312.20	0.2	0.0	0.22		
49	39	59217.38	312.28	0.2	0.0	0.22		
50	38	61519.89	312.36	0.2	0.0	0.22		
51	38	63803.78	312.44	0.2	0.0	0.22		
52	37	66063.80	312.52	0.2	0.0	0.22		
53	37	68294.81	312.59	0.2	0.0	0.22		
54	36	70491.81	312.67	0.2	0.0	0.22		
55	35	72649.96	312.74	0.2	0.0	0.22		
56	35	74764.60	312.81	0.2	0.0	0.22		
57	34	76831.27	312.88	0.2	0.0	0.22		
58	22	78845.75	312.95	0.2	0.0	0.22		
59	21	80158.16	313.00	0.2	0.0	0.22		
60	21	81433.67	313.04	0.2	0.0	0.22		
61	20	82673.31	313.08	0.2	0.0	0.22		
62	20	83878.07	313.12	0.2	0.0	0.22		
63	19	85048.93	313.16	0.2	0.0	0.22		
64	19	86186.82	313.20	0.2	0.0	0.22		
65	18	87292.67	313.24	0.2	0.0	0.22		
66	18	88367.36	313.27	0.2	0.0	0.22		
67	17	89411.78	313.31	0.2	0.0	0.22		
68	17	90426.74	313.34	0.2	0.0	0.22		
69	16	91413.09	313.37	0.2	0.0	0.22		
70	16	92371.61	313.41	0.2	0.0	0.22		
71	15	93303.08	313.44	0.2	0.0	0.22		
72	15	94208.25	313.47	0.2	0.0	0.22		
73	14	95087.86	313.50	0.2	0.0	0.22		
74	14	95942.61	313.52	0.2	0.0	0.22		
75	14	96773.20	313.55	0.2	0.0	0.22		
76	13	97580.29	313.58	0.2	0.0	0.22		
77	13	98364.56	313.61	0.2	0.0	0.22		
78	13	99126.62	313.63	0.2	0.0	0.22		
79	12	99867.10	313.65	0.2	0.0	0.22		
80	12	100586.60	313.68	0.2	0.0	0.22		
81	12	101285.70	313.70	0.2	0.0	0.22		
82	11	101964.97	313.72	0.2	0.0	0.22		
83	11	102624.96	313.75	0.2	0.0	0.22		
84	11	103266.22	313.77	0.2	0.0	0.22		
85	10	103889.25	313.79	0.2	0.0	0.22		
86	10	104494.57	313.81	0.2	0.0	0.22		
87	10	105082.68	313.83	0.2	0.0	0.22		
88	9	105654.04	313.85	0.2	0.0	0.22		
89	9	106209.13	313.86	0.2	0.0	0.22		
90	9	106748.40	313.88	0.2	0.0	0.22		
91	9	107272.29	313.90	0.2	0.0	0.22		
92	8	107781.23	313.92	0.2	0.0	0.22		
93	8	108275.63	313.93	0.2	0.0	0.22		
94	8	108755.90	313.95	0.2	0.0	0.22		
95	8	109222.43	313.96	0.2	0.0	0.22		
96	8	109675.60	313.98	0.2	0.0	0.22		
97	7	110115.80	313.99	0.2	0.0	0.22		
98	7	110543.37	314.01	0.3	0.0	0.22		
99	7	110956.79	314.02	0.4	0.2	0.22		
100	7	111348.61	314.03	0.6	0.4	0.22		
101	7	111715.57	314.04	0.9	0.7	0.22		
102	6	112056.10	314.06	1.2	0.9	0.22		
103	6	112369.60	314.07	1.4	1.2	0.22		
104	6	112656.16	314.07	1.7	1.5	0.22		
105	6	112916.30	314.08	2.0	1.7	0.22		
106	6	113150.89	314.09	2.2	2.0	0.22		
107	6	113361.04	314.10	2.4	2.2	0.22		
108	5	113548.01	314.10	2.6	2.4	0.22		
109	5	113713.18	314.11	2.8	2.6	0.22		
110	5	113857.94	314.11	3.0	2.8	0.22		
111	5	113983.74	314.12	3.1	2.9	0.22		
112	5	114091.97	314.12	3.3	3.1	0.22		
113	5	114184.01	314.12	3.4	3.2	0.22		
114	5	114261.19	314.13	3.5	3.3	0.22		
115	4	114324.77	314.13	3.6	3.3	0.22		
116	4	114375.95	314.13	3.6	3.4	0.22		
117	4	114415.84	314.13	3.7	3.5	0.22		
118	4	114445.50	314.13	3.7	3.5	0.22		
119	4	114465.89	314.13	3.7	3.5	0.22		
120	4	114477.94	314.13	3.8	3.5	0.22		
121	4	114482.45	314.13	3.8	3.5	0.22		
122	4	114480.21	314.13	3.8	3.5	0.22		

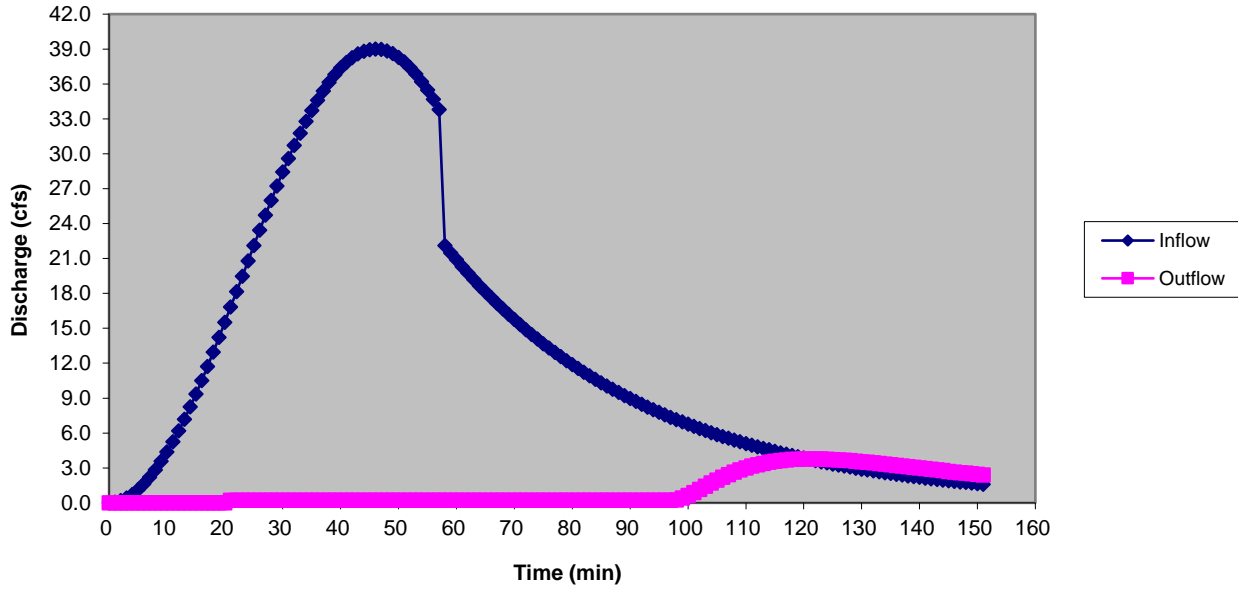
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	4	114471.90	314.13	3.8	3.5	0.22		
124	3	114458.19	314.13	3.7	3.5	0.22		
125	3	114439.64	314.13	3.7	3.5	0.22		
126	3	114416.80	314.13	3.7	3.5	0.22		
127	3	114390.15	314.13	3.6	3.4	0.22		
128	3	114360.13	314.13	3.6	3.4	0.22		
129	3	114327.15	314.13	3.6	3.3	0.22		
130	3	114291.57	314.13	3.5	3.3	0.22		
131	3	114253.71	314.13	3.5	3.3	0.22		
132	3	114213.87	314.13	3.4	3.2	0.22		
133	3	114172.34	314.12	3.4	3.2	0.22		
134	3	114129.33	314.12	3.3	3.1	0.22		
135	3	114085.08	314.12	3.3	3.0	0.22		
136	2	114039.78	314.12	3.2	3.0	0.22		
137	2	113993.62	314.12	3.2	2.9	0.22		
138	2	113946.74	314.12	3.1	2.9	0.22		
139	2	113899.29	314.12	3.0	2.8	0.22		
140	2	113851.41	314.11	3.0	2.8	0.22		
141	2	113803.21	314.11	2.9	2.7	0.22		
142	2	113754.78	314.11	2.9	2.7	0.22		
143	2	113706.24	314.11	2.8	2.6	0.22		
144	2	113657.65	314.11	2.8	2.5	0.22		
145	2	113609.09	314.11	2.7	2.5	0.22		
146	2	113560.63	314.10	2.6	2.4	0.22		
147	2	113512.33	314.10	2.6	2.4	0.22		
148	2	113464.24	314.10	2.5	2.3	0.22		
149	2	113416.41	314.10	2.5	2.3	0.22		
150	2	113368.88	314.10	2.4	2.2	0.22		
151	2	113321.68	314.10	2.4	2.2	0.22		



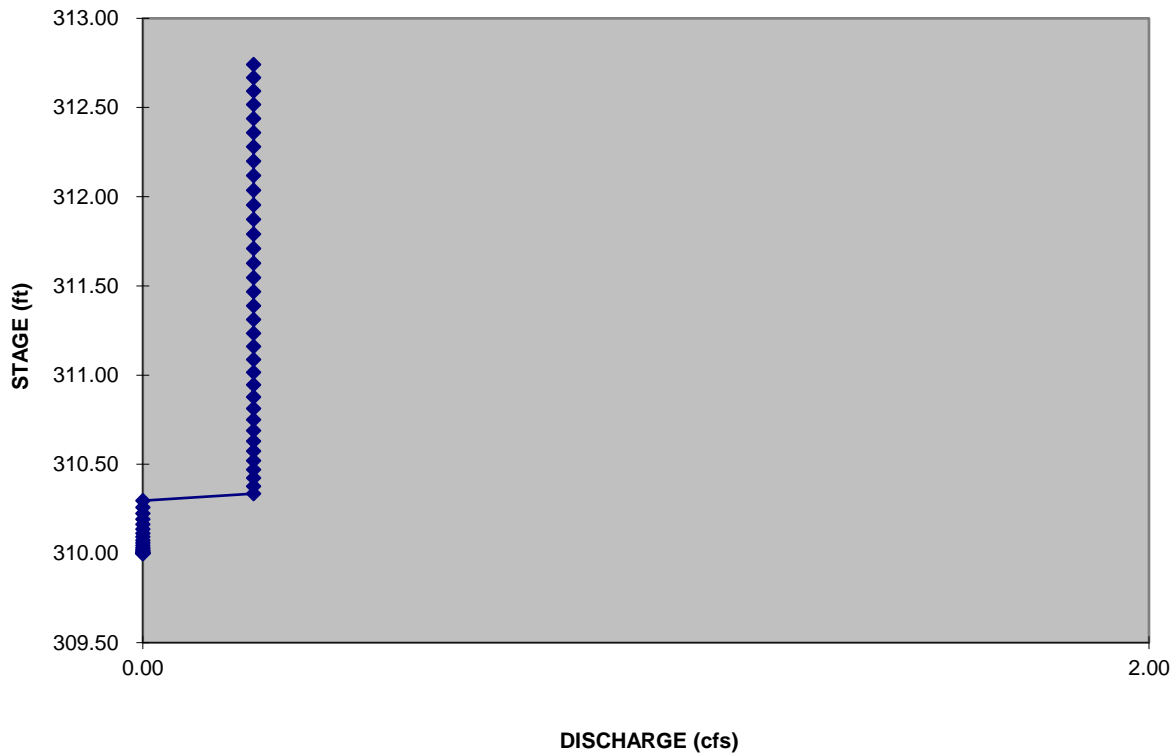
# McADAMS

## GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM C  
SKIMMER BASIN WITH TEMP RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Riser-Barrel Design  
Stage-Storage Function  
Hydrograph Formulation  
Anti-Bouyancy Calculations  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm



# McADAMS

## RISER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Basin ID: SCM C

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	7.93 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	7.93 ac.	0.25
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>7.93 ac.</b>	<b>7.93 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.25</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		<i>K Values</i>	
Length of flow path =	847	feet	Overland on grassed surfaces:	2.0
Height of watershed =	8	feet	Overland on paved surfaces:	0.4
Calculated t(c) =	8.4	minutes	Channel in natural channels:	1.0
Minimum t(c) =	5.0	minutes	Channel in mixed urban setting:	1.1
Time of concentration =	8.4	minutes	Channel in paved pipes or channels:	0.2

### Time of Concentration

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	4.99 in/hr
10	195	22	6.40 in/hr
25	232	23	7.38 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	4.99 in/hr	17.8 cfs	9.89 cfs
10-year storm	6.40 in/hr	22.9 cfs	12.70 cfs
25-year storm	7.38 in/hr	26.3 cfs	14.62 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	

### Sediment Trap Dimensions

Bottom length =	410
Bottom width =	85
Sediment depth =	1.0
Freeboard (from sediment depth) =	3.0
Depth to crest of emergency spillway =	4.0
Side slopes =	3.0 H:1V
Riser Diameter (in) =	36
Emergency Spillway length =	20
Height of berm =	4.0
Top of trap length =	434
Top of trap width =	109
Bottom Elevation =	312

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	14274 cu. ft.	36340 cu. ft.
Sediment surface area =	7430 sq. ft.	37860 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	1
Trap bottom length to width ratio =	2.0L:1W (min)	4.8L:1W
Emergency Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	-2.61 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	2.61 ft
10-Year flow depth over riser =	1.50 ft. (max)	0.39 ft

1. Reference for Rational Coefficients:

Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 14,274 cubic feet

24 HOURS	Best Option =	1 - 4 inch Skimmers with a 3.4 inch orifice
2 DAYS	Best Option =	1 - 3 inch Skimmers with a 2.6 inch orifice
3 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.2 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.8 inch orifice

<<== USE



# McADAMS

## RISER - BARREL DESIGN

### DESIGN PRINCIPAL AND EMERGENCY SPILLWAY

Determine minimum principal spillway outflow  
 Minimum outflow = 0.2 cfs per acre

Drainage area = **7.93** acres  
 Minimum outflow = **1.6** cfs

Principal spillway elevation = **313.00**  
 Emergency spillway elevation = **316.00**

### SIZE RISER/BARREL

Constant TW elevation = **311.00** feet  
 Riser diameter = **36** inches  
 Barrel diameter = **24** inches  
 Barrel Manning's n = **0.013**  
 Riser crest elevation = **313.00** feet  
 Barrel invert in elevation = **311.50** feet  
 Barrel invert out = **311.00** feet  
 Barrel length = **50** feet  
 Barrel slope = **0.0100** ft/ft

\*Typically in

\*CMP=0.02

Basin WSEL (feet)	Riser as orifice* (cfs)	Riser as weir** (cfs)	Barrel TW condition s (cfs)	Barrel HW condition (cfs)	Primary spillway outflow (cfs)	Action ?
<b>316.00</b>	<b>59.0</b>	<b>161.6</b>	<b>38.6</b>	<b>28.3</b>	<b>28.3</b>	<b>OK</b>

\* 
$$= 0.6 * \pi * \frac{RiserDiameter^2}{12} * \sqrt{2 * g * h}$$

\*\* 
$$= 3.3 * \pi * \frac{RiserDiameter}{12} * (h)^{\frac{3}{2}}$$

nvert out of barrel, unless tailwater can be calculated



# McADAMS

## STAGE-STORAGE FUNCTION

CHAMBLEE LAKE

### AVERAGE - END CALCULATIONS

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
312	34632		0	0			
313	38069	36351	36350.5	1	10.5010	0.0000	0.99
314	41563	39816	76166.5	2	11.2407	0.6931	1.99
315	45114	43338.5	119505	3	11.6911	1.0986	3.06
316	48721	46917.5	166422.5	4	12.0223	1.3863	4.18
317	0	24360.5	190783	5	12.1589	1.6094	4.76

### STAGE-STORAGE FUNCTION:

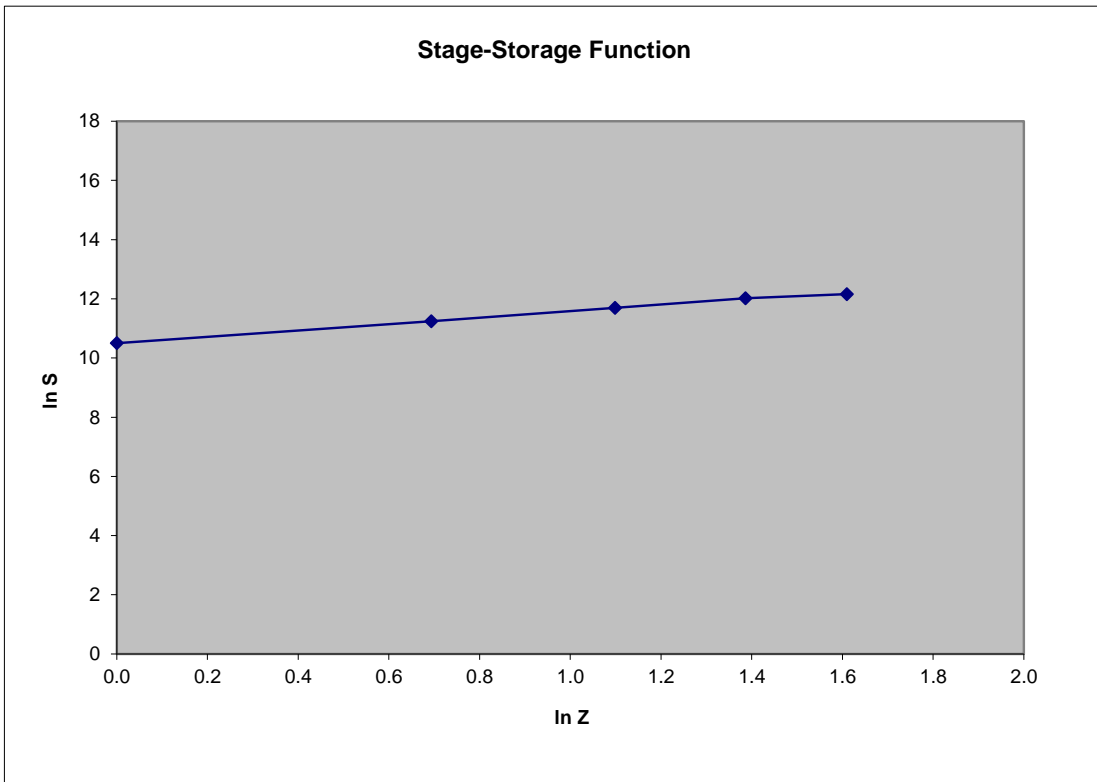
$$S = K_s * Z^b$$

where:

b = 1.06

and

K<sub>s</sub> = 36749





# ANTI - FLOTATION CALCULATION DESIGN SHEET

CHAMBLEE LAKE

## CMP RISER

Riser Diameter =  inches  
Riser Height =  feet  
Length of barrel exposed from berm section =  feet  
Diameter of barrel =  inches

Volume displaced by riser =  CF  
Volume displaced by barrel =  CF

Total Volume Displaced by System =  CF  
Total Water Wt. Displaced by System =  LBS.

## Design of concrete anti-flotation block

Safety factor for use =   
Unit weight for concrete =  PCF  
Unit Weight for water =  PCF

Buoyant wt. concrete (w/ safety factor applied) =  PCF  
With Safety Factor, conc. must weigh (buoyant wt.) =  LBS.  
Volume of Concrete Required =  CF

Enter size wanted for concrete block ==>

Width =  feet  
Length =  feet  
Thickness =  inches

Volume of concrete provided =  CF

## Check validity of anti-flotation design

Total volume of water displaced =  CF  
Total volume of concrete present =  CF

Total weight of water displaced =  LBS.  
Total weight of concrete present =  LBS.

Safety Factor w/ design as above =

**Therefore, for anti-flotation, use a concrete block:**

Width =  feet  
Length =  feet  
Thickness =  inches

**This block shall be integrally attached to the riser unit. The barrel shall attach to the riser with a watertight joint.**



# **McADAMS** **HYDROGRAPH FORMULATION**

## CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT	OUTPUT
Drainage Area, DA = <span style="border: 1px solid black; padding: 2px;">7.93</span> ac Disturbed Area = <span style="border: 1px solid black; padding: 2px;">7.93</span> ac Undisturbed Woods = <span style="border: 1px solid black; padding: 2px;">0.00</span> ac Undisturbed Grass = <span style="border: 1px solid black; padding: 2px;">0.00</span> ac Hydraulic Length = <span style="border: 1px solid black; padding: 2px;">847</span> ft Vertical Fall = <span style="border: 1px solid black; padding: 2px;">8</span> ft For 1 yr Storm, g = <span style="border: 1px solid black; padding: 2px;">132</span> h = <span style="border: 1px solid black; padding: 2px;">18</span> For 10 yr Storm, g = <span style="border: 1px solid black; padding: 2px;">195</span> h = <span style="border: 1px solid black; padding: 2px;">22</span> For 25 yr Storm, g = <span style="border: 1px solid black; padding: 2px;">232</span> h = <span style="border: 1px solid black; padding: 2px;">23</span>	<b>Use Kirpich Equation:</b> Tc = <span style="border: 1px solid black; padding: 2px;">8.4 min</span>  <b>Use Malcom Method:</b> i <sub>1</sub> = <span style="border: 1px solid black; padding: 2px;">4.99 in/hr</span> i <sub>10</sub> = <span style="border: 1px solid black; padding: 2px;">6.40 in/hr</span> i <sub>25</sub> = <span style="border: 1px solid black; padding: 2px;">7.38 in/hr</span>  <b>Use Rational Method:</b> Q <sub>2</sub> = <span style="border: 1px solid black; padding: 2px;">17.8 cfs</span> Q <sub>10</sub> = <span style="border: 1px solid black; padding: 2px;">22.9 cfs</span> Q <sub>25</sub> = <span style="border: 1px solid black; padding: 2px;">26.3 cfs</span>
"C" CALCULATION - See Sediment Trap Sizing  "C" = <span style="border-bottom: 3px double black; padding: 0 20px;">0.45</span>	

**SOLUTION - Tp:**

INPUT	OUTPUT
P <sub>n6</sub> = <span style="border: 1px solid black; padding: 2px;">2.7</span> in (2 Year Storm) CN = <span style="border: 1px solid black; padding: 2px;">91</span> (newly graded areas)	S = <span style="border: 1px solid black; padding: 2px;">0.99</span> in Q* = <span style="border: 1px solid black; padding: 2px;">1.75</span> in  T <sub>p2</sub> = <span style="border: 1px solid black; padding: 2px;">33.9</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid black; padding: 2px;">3.9</span> in (10 Year Storm)	Q* = <span style="border: 1px solid black; padding: 2px;">2.92</span> in  T <sub>p10</sub> = <span style="border: 1px solid black; padding: 2px;">44.1</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid black; padding: 2px;">4.6</span> in (25 Year Storm)	Q* = <span style="border: 1px solid black; padding: 2px;">3.59</span> in  T <sub>p25</sub> = <span style="border: 1px solid black; padding: 2px;">47.1</span> minutes

# McADAMS ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

## COMPUTED RESULTS

	Peak Stage =	313.18	ft
	Rise =	1.18	ft
Freeboard =	2.82	ft	
	Peak Outflow =	2.23	cfs

## INPUT

### Hydrograph Results:

Qp = 17.81 cfs  
 Tp = 33.9 min  
 dT = 1.0 min

### Stage-Storage Results:

Ks = 36749  
 b = 1.06  
 Z<sub>0</sub> = 312.0 ft (inv)

### Initial Water Level:

Z<sub>i</sub> = 312.00 ft

### Weir:

N = 1  
 L = 20 ft  
 Cw = 3.00  
 Zcr = 316.00 ft

### Riser:

N = 1  
 D = 36 in  
 Cw = 3.00  
 Zcr = 313.00 ft

Top of Dam: 316.00 ft

### Skimmer Orifice:

Number = 1.00 Ea  
 Diameter = 2.50 Inches  
 Head = 2.18 inches

## OUTPUT

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts	Riser Flow cts	
0	0	0.00	312.00	0.0	0.0	0.0	0.0	
1	0	0.00	312.00	0.0	0.0	0.00	0.0	
2	0	2.30	312.00	0.0	0.0	0.00	0.0	
3	0	11.47	312.00	0.0	0.0	0.00	0.0	
4	1	32.02	312.00	0.0	0.0	0.00	0.0	
5	1	68.38	312.00	0.0	0.0	0.00	0.0	
6	1	124.83	312.00	0.0	0.0	0.00	0.0	
7	2	205.48	312.01	0.0	0.0	0.00	0.0	
8	2	314.22	312.01	0.0	0.0	0.00	0.0	
9	3	454.72	312.02	0.0	0.0	0.00	0.0	
10	4	630.36	312.02	0.0	0.0	0.00	0.0	
11	4	844.23	312.03	0.0	0.0	0.00	0.0	
12	5	1099.07	312.04	0.0	0.0	0.00	0.0	
13	6	1397.31	312.05	0.0	0.0	0.00	0.0	
14	7	1740.96	312.06	0.0	0.0	0.00	0.0	
15	7	2131.66	312.07	0.0	0.0	0.00	0.0	
16	8	2570.65	312.08	0.0	0.0	0.00	0.0	
17	9	3058.75	312.09	0.0	0.0	0.00	0.0	
18	10	3596.35	312.11	0.0	0.0	0.00	0.0	
19	11	4183.44	312.13	0.0	0.0	0.00	0.0	
20	11	4819.54	312.15	0.0	0.0	0.00	0.0	
21	12	5503.79	312.17	0.0	0.0	0.00	0.0	
22	13	6234.90	312.19	0.0	0.0	0.00	0.0	
23	14	7011.17	312.21	0.0	0.0	0.00	0.0	
24	14	7830.53	312.23	0.1	0.0	0.07	0.0	
25	15	8686.39	312.26	0.1	0.0	0.07	0.0	
26	16	9580.08	312.28	0.1	0.0	0.07	0.0	
27	16	10508.48	312.31	0.1	0.0	0.07	0.0	
28	17	11468.15	312.33	0.1	0.0	0.07	0.0	
29	17	12455.42	312.36	0.1	0.0	0.07	0.0	
30	17	13466.34	312.39	0.1	0.0	0.07	0.0	
31	17	14496.78	312.41	0.1	0.0	0.07	0.0	
32	18	15542.45	312.44	0.1	0.0	0.07	0.0	
33	18	16598.90	312.47	0.1	0.0	0.07	0.0	
34	18	17661.61	312.50	0.1	0.0	0.07	0.0	
35	18	18726.00	312.53	0.1	0.0	0.07	0.0	
36	18	19787.48	312.56	0.1	0.0	0.07	0.0	
37	17	20841.48	312.58	0.1	0.0	0.07	0.0	
38	17	21883.50	312.61	0.1	0.0	0.07	0.0	
39	17	22909.12	312.64	0.1	0.0	0.07	0.0	
40	16	23914.10	312.67	0.1	0.0	0.07	0.0	
41	16	24894.35	312.69	0.1	0.0	0.07	0.0	
42	15	25845.99	312.72	0.1	0.0	0.07	0.0	
43	15	26765.41	312.74	0.1	0.0	0.07	0.0	
44	14	27651.37	312.76	0.1	0.0	0.07	0.0	
45	14	28503.81	312.79	0.1	0.0	0.07	0.0	

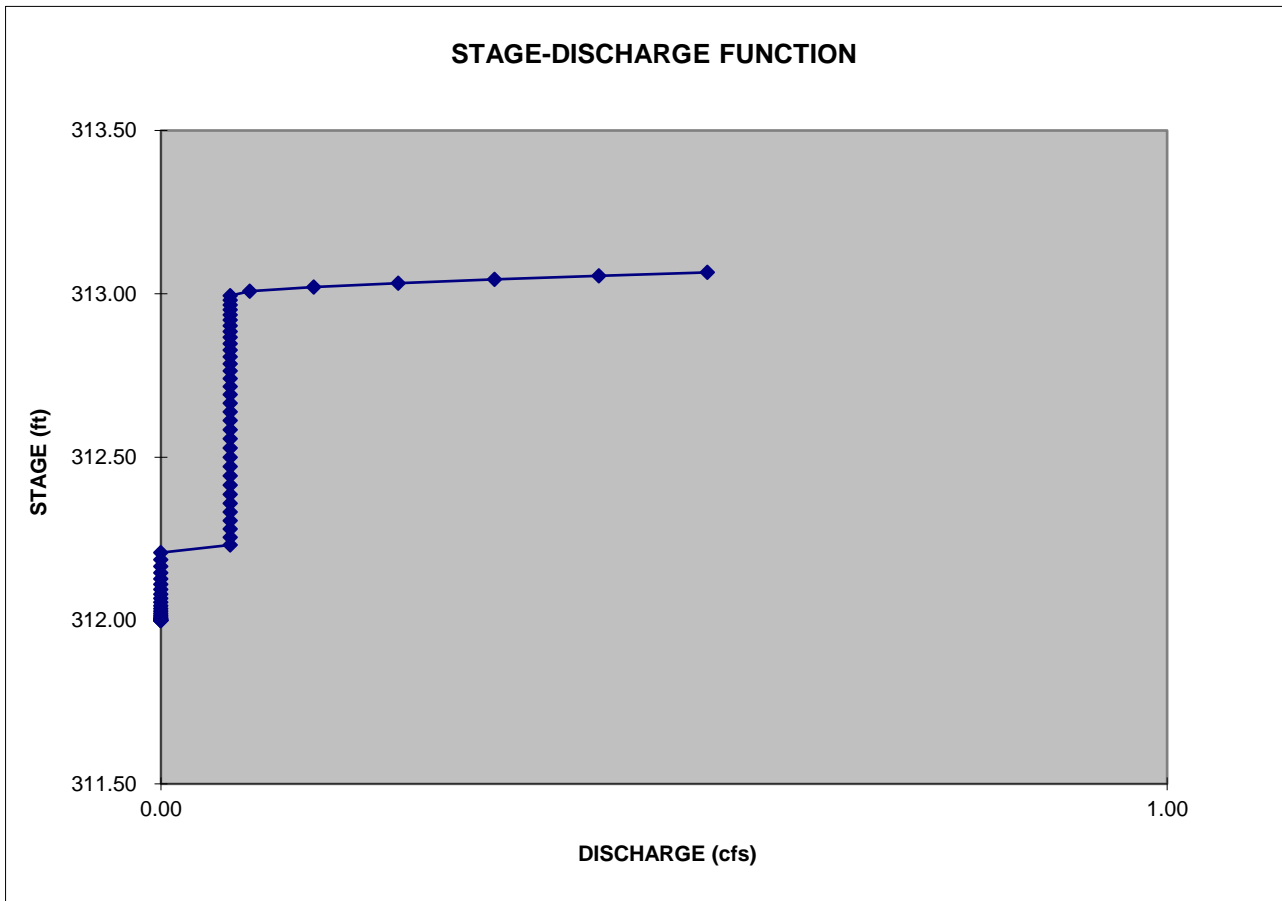
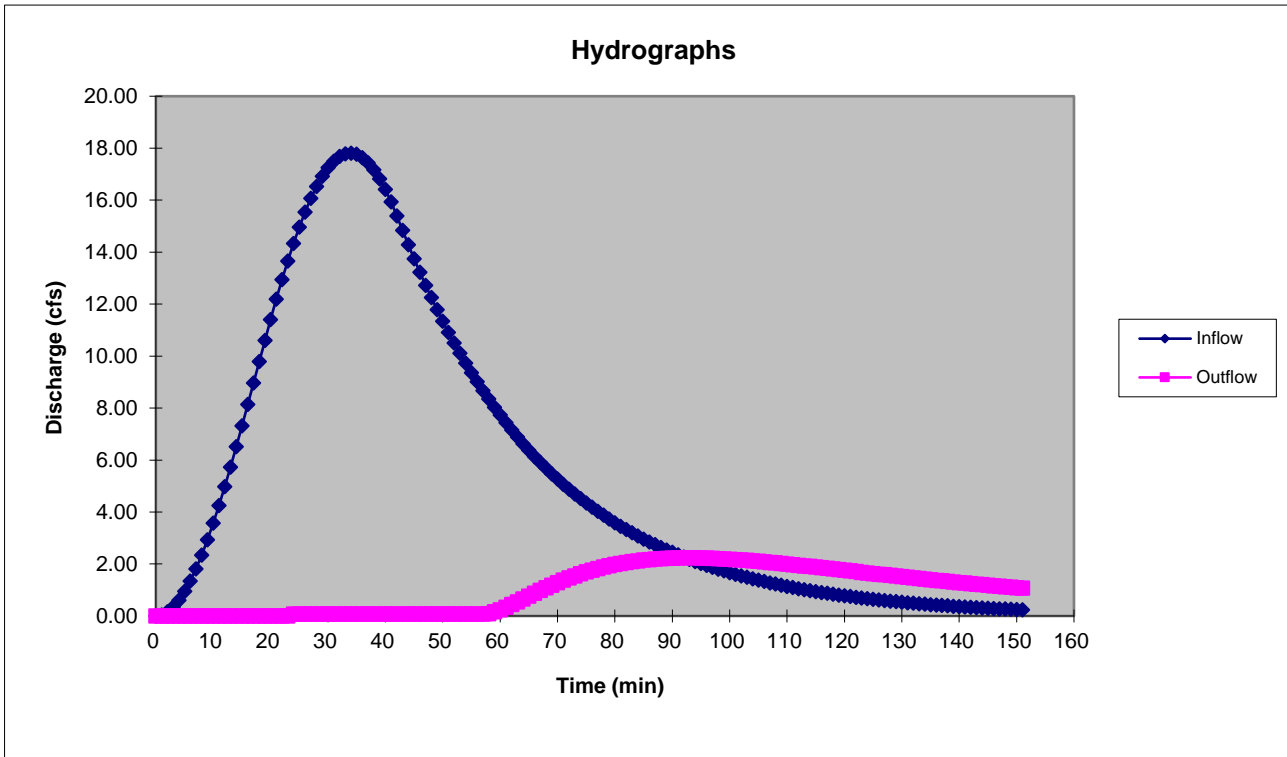


OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts	Riser Flow cts	
46	13	29323.99	312.81	0.1	0.0	0.07	0.0	
47	13	30113.12	312.83	0.1	0.0	0.07	0.0	
48	12	30872.39	312.85	0.1	0.0	0.07	0.0	
49	12	31602.90	312.87	0.1	0.0	0.07	0.0	
50	11	32305.75	312.89	0.1	0.0	0.07	0.0	
51	11	32981.98	312.90	0.1	0.0	0.07	0.0	
52	11	33632.58	312.92	0.1	0.0	0.07	0.0	
53	10	34258.53	312.94	0.1	0.0	0.07	0.0	
54	10	34860.75	312.95	0.1	0.0	0.07	0.0	
55	9	35440.13	312.97	0.1	0.0	0.07	0.0	
56	9	35997.54	312.98	0.1	0.0	0.07	0.0	
57	9	36533.80	312.99	0.1	0.0	0.07	0.0	
58	8	37049.71	313.01	0.1	0.0	0.07	0.0	
59	8	37544.88	313.02	0.2	0.0	0.07	0.1	
60	8	38017.38	313.03	0.2	0.0	0.07	0.2	
61	7	38466.72	313.04	0.3	0.0	0.07	0.3	
62	7	38892.84	313.06	0.4	0.0	0.07	0.4	
63	7	39295.95	313.07	0.5	0.0	0.07	0.5	
64	7	39676.42	313.08	0.7	0.0	0.07	0.6	
65	6	40034.75	313.08	0.8	0.0	0.07	0.7	
66	6	40371.49	313.09	0.9	0.0	0.07	0.8	
67	6	40687.28	313.10	1.0	0.0	0.07	0.9	
68	6	40982.78	313.11	1.1	0.0	0.07	1.0	
69	5	41258.70	313.12	1.2	0.0	0.07	1.1	
70	5	41515.75	313.12	1.3	0.0	0.07	1.2	
71	5	41754.66	313.13	1.4	0.0	0.07	1.3	
72	5	41976.16	313.13	1.5	0.0	0.07	1.4	
73	5	42180.99	313.14	1.5	0.0	0.07	1.5	
74	5	42369.88	313.14	1.6	0.0	0.07	1.5	
75	4	42543.54	313.15	1.7	0.0	0.07	1.6	
76	4	42702.67	313.15	1.8	0.0	0.07	1.7	
77	4	42847.97	313.16	1.8	0.0	0.07	1.8	
78	4	42980.11	313.16	1.9	0.0	0.07	1.8	
79	4	43099.74	313.16	1.9	0.0	0.07	1.9	
80	4	43207.50	313.17	2.0	0.0	0.07	1.9	
81	3	43304.00	313.17	2.0	0.0	0.07	2.0	
82	3	43389.83	313.17	2.1	0.0	0.07	2.0	
83	3	43465.58	313.17	2.1	0.0	0.07	2.0	
84	3	43531.79	313.17	2.1	0.0	0.07	2.1	
85	3	43588.98	313.18	2.1	0.0	0.07	2.1	
86	3	43637.68	313.18	2.2	0.0	0.07	2.1	
87	3	43678.36	313.18	2.2	0.0	0.07	2.1	
88	3	43711.49	313.18	2.2	0.0	0.07	2.1	
89	3	43737.53	313.18	2.2	0.0	0.07	2.1	
90	2	43756.89	313.18	2.2	0.0	0.07	2.2	
91	2	43769.98	313.18	2.2	0.0	0.07	2.2	
92	2	43777.20	313.18	2.2	0.0	0.07	2.2	
93	2	43778.91	313.18	2.2	0.0	0.07	2.2	
94	2	43775.46	313.18	2.2	0.0	0.07	2.2	
95	2	43767.18	313.18	2.2	0.0	0.07	2.2	
96	2	43754.41	313.18	2.2	0.0	0.07	2.2	
97	2	43737.43	313.18	2.2	0.0	0.07	2.1	
98	2	43716.53	313.18	2.2	0.0	0.07	2.1	
99	2	43692.00	313.18	2.2	0.0	0.07	2.1	
100	2	43664.07	313.18	2.2	0.0	0.07	2.1	
101	2	43633.01	313.18	2.2	0.0	0.07	2.1	
102	2	43599.04	313.18	2.2	0.0	0.07	2.1	
103	1	43562.37	313.17	2.1	0.0	0.07	2.1	
104	1	43523.23	313.17	2.1	0.0	0.07	2.0	
105	1	43481.79	313.17	2.1	0.0	0.07	2.0	
106	1	43438.25	313.17	2.1	0.0	0.07	2.0	
107	1	43392.79	313.17	2.1	0.0	0.07	2.0	
108	1	43345.56	313.17	2.0	0.0	0.07	2.0	
109	1	43296.72	313.17	2.0	0.0	0.07	1.9	
110	1	43246.43	313.17	2.0	0.0	0.07	1.9	
111	1	43194.81	313.17	2.0	0.0	0.07	1.9	
112	1	43142.00	313.16	1.9	0.0	0.07	1.9	
113	1	43088.11	313.16	1.9	0.0	0.07	1.9	
114	1	43033.27	313.16	1.9	0.0	0.07	1.8	



McADAMS

# GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	2.61	ft	
Peak Stage =	313.39	ft	
Rise =	1.39	ft	
Peak Outflow =	7.07	cfs	

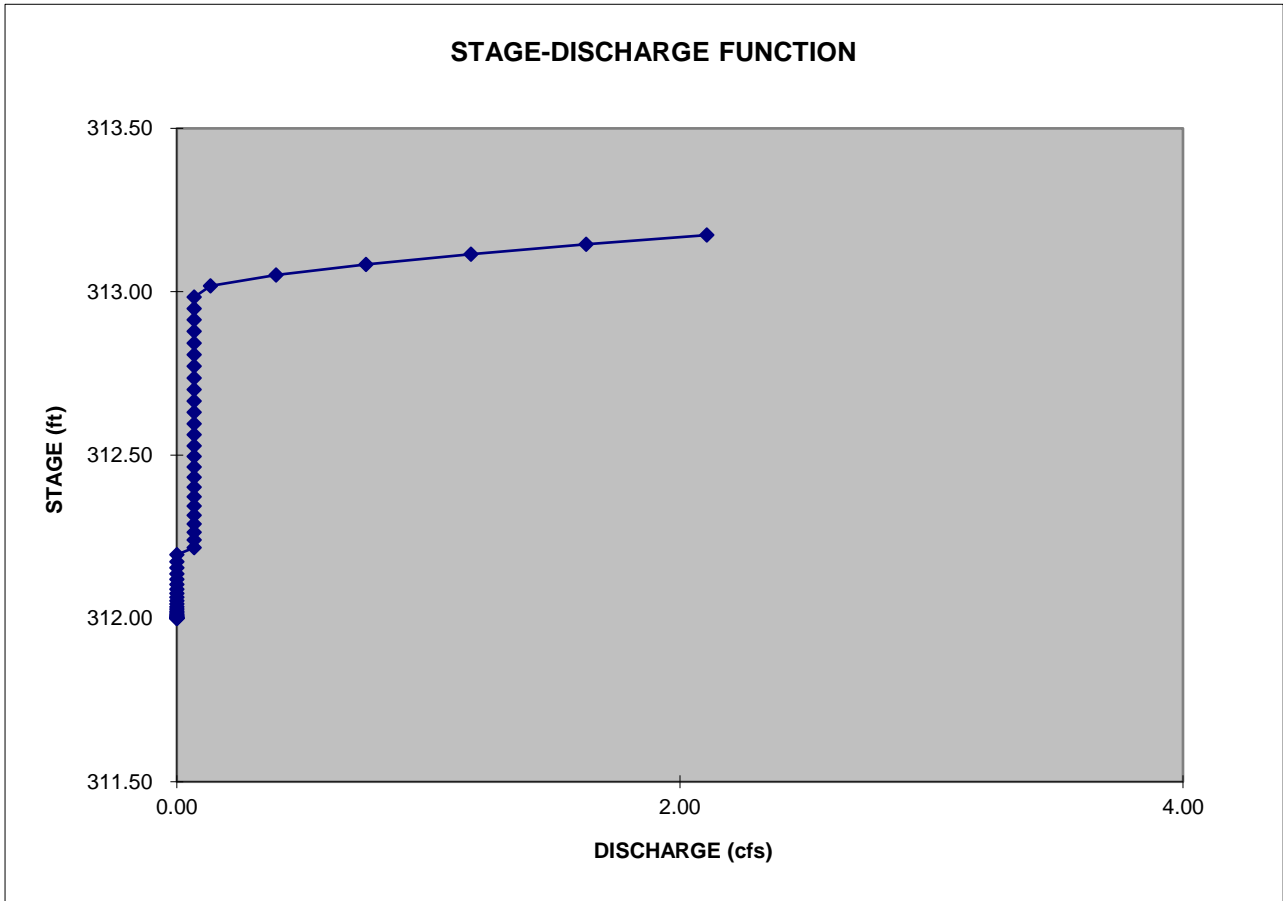
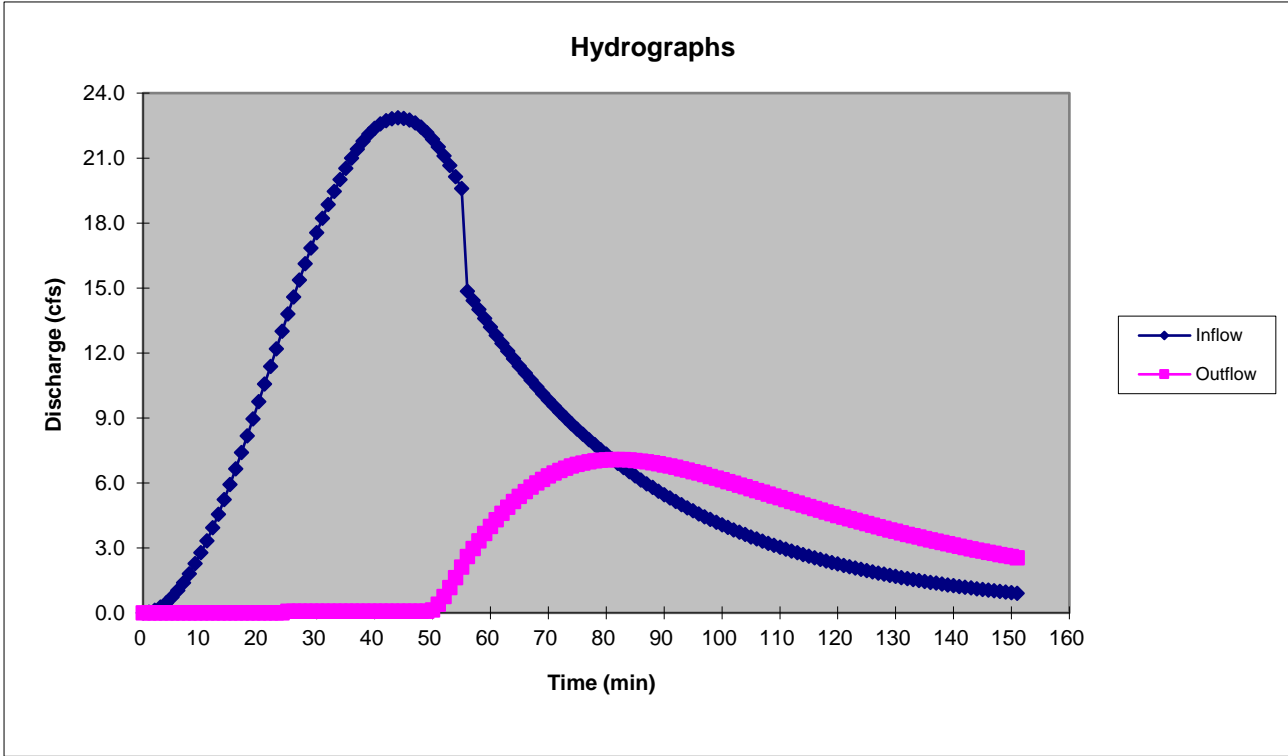
INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft	
Qp = 22.85 cfs	N = 1		
Tp = 44.1 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 316.00 ft	<b>Skimmer Orifice:</b>	
Ks = 36749	<b>Riser:</b>	Number = 1	Ea
b = 1.06	N = 1	Diameter = 2.50	Inches
Z <sub>0</sub> = 312.0 ft (inv)	D = 36 in	Head = 2.18	inches
<b>Initial Water Level:</b>	Cw = 3.00		
Z <sub>i</sub> = 312.00 ft	Zcr = 313.00 ft		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts	Riser Flow cts	
0	0	0.00	312.00	0.0	0.0	0.0	0.0	
1	0	0.00	312.00	0.0	0.0	0.00	0.0	
2	0	1.74	312.00	0.0	0.0	0.00	0.0	
3	0	8.68	312.00	0.0	0.0	0.00	0.0	
4	0	24.25	312.00	0.0	0.0	0.00	0.0	
5	1	51.87	312.00	0.0	0.0	0.00	0.0	
6	1	94.85	312.00	0.0	0.0	0.00	0.0	
7	1	156.46	312.01	0.0	0.0	0.00	0.0	
8	2	239.85	312.01	0.0	0.0	0.00	0.0	
9	2	348.08	312.01	0.0	0.0	0.00	0.0	
10	3	484.07	312.02	0.0	0.0	0.00	0.0	
11	3	650.61	312.02	0.0	0.0	0.00	0.0	
12	4	850.32	312.03	0.0	0.0	0.00	0.0	
13	5	1085.67	312.04	0.0	0.0	0.00	0.0	
14	5	1358.95	312.04	0.0	0.0	0.00	0.0	
15	6	1672.23	312.05	0.0	0.0	0.00	0.0	
16	7	2027.40	312.06	0.0	0.0	0.00	0.0	
17	7	2426.15	312.08	0.0	0.0	0.00	0.0	
18	8	2869.92	312.09	0.0	0.0	0.00	0.0	
19	9	3359.93	312.10	0.0	0.0	0.00	0.0	
20	10	3897.19	312.12	0.0	0.0	0.00	0.0	
21	11	4482.43	312.14	0.0	0.0	0.00	0.0	
22	11	5116.18	312.15	0.0	0.0	0.00	0.0	
23	12	5798.68	312.17	0.0	0.0	0.00	0.0	
24	13	6529.97	312.19	0.0	0.0	0.00	0.0	
25	14	7309.79	312.22	0.1	0.0	0.07	0.0	
26	15	8133.56	312.24	0.1	0.0	0.07	0.0	
27	15	9004.67	312.26	0.1	0.0	0.07	0.0	
28	16	9922.17	312.29	0.1	0.0	0.07	0.0	
29	17	10884.86	312.32	0.1	0.0	0.07	0.0	
30	18	11891.31	312.34	0.1	0.0	0.07	0.0	
31	18	12939.88	312.37	0.1	0.0	0.07	0.0	
32	19	14028.71	312.40	0.1	0.0	0.07	0.0	
33	19	15155.74	312.43	0.1	0.0	0.07	0.0	
34	20	16318.71	312.46	0.1	0.0	0.07	0.0	
35	21	17515.17	312.50	0.1	0.0	0.07	0.0	
36	21	18742.52	312.53	0.1	0.0	0.07	0.0	
37	21	19997.99	312.56	0.1	0.0	0.07	0.0	
38	22	21278.68	312.60	0.1	0.0	0.07	0.0	
39	22	22581.55	312.63	0.1	0.0	0.07	0.0	
40	22	23903.45	312.67	0.1	0.0	0.07	0.0	
41	23	25241.14	312.70	0.1	0.0	0.07	0.0	
42	23	26591.28	312.74	0.1	0.0	0.07	0.0	
43	23	27950.50	312.77	0.1	0.0	0.07	0.0	
44	23	29315.35	312.81	0.1	0.0	0.07	0.0	
45	23	30682.39	312.84	0.1	0.0	0.07	0.0	
46	23	32048.12	312.88	0.1	0.0	0.07	0.0	

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts	Riser Flow cts	
47	23	33409.10	312.91	0.1	0.0	0.07	0.0	
48	22	34761.86	312.95	0.1	0.0	0.07	0.0	
49	22	36103.02	312.98	0.1	0.0	0.07	0.0	
50	22	37429.23	313.02	0.1	0.0	0.07	0.1	
51	22	38733.29	313.05	0.4	0.0	0.07	0.3	
52	21	40000.31	313.08	0.8	0.0	0.07	0.7	
53	21	41221.44	313.11	1.2	0.0	0.07	1.1	
54	20	42390.11	313.14	1.6	0.0	0.07	1.6	
55	20	43501.15	313.17	2.1	0.0	0.07	2.0	
56	15	44550.53	313.20	2.6	0.0	0.07	2.5	
57	14	45285.47	313.22	3.0	0.0	0.07	2.9	
58	14	45972.73	313.24	3.3	0.0	0.07	3.2	
59	14	46613.63	313.25	3.7	0.0	0.07	3.6	
60	13	47209.61	313.27	4.0	0.0	0.07	3.9	
61	13	47762.24	313.28	4.3	0.0	0.07	4.2	
62	12	48273.14	313.29	4.6	0.0	0.07	4.5	
63	12	48743.98	313.31	4.9	0.0	0.07	4.8	
64	12	49176.46	313.32	5.1	0.0	0.07	5.1	
65	11	49572.29	313.33	5.4	0.0	0.07	5.3	
66	11	49933.17	313.34	5.6	0.0	0.07	5.5	
67	11	50260.78	313.35	5.8	0.0	0.07	5.7	
68	10	50556.78	313.35	6.0	0.0	0.07	5.9	
69	10	50822.80	313.36	6.2	0.0	0.07	6.1	
70	10	51060.43	313.37	6.3	0.0	0.07	6.2	
71	10	51271.20	313.37	6.5	0.0	0.07	6.4	
72	9	51456.61	313.38	6.6	0.0	0.07	6.5	
73	9	51618.09	313.38	6.7	0.0	0.07	6.6	
74	9	51757.04	313.38	6.8	0.0	0.07	6.7	
75	8	51874.79	313.39	6.9	0.0	0.07	6.8	
76	8	51972.62	313.39	6.9	0.0	0.07	6.8	
77	8	52051.75	313.39	7.0	0.0	0.07	6.9	
78	8	52113.36	313.39	7.0	0.0	0.07	6.9	
79	8	52158.55	313.39	7.0	0.0	0.07	7.0	
80	7	52188.40	313.39	7.1	0.0	0.07	7.0	
81	7	52203.90	313.39	7.1	0.0	0.07	7.0	
82	7	52206.03	313.39	7.1	0.0	0.07	7.0	
83	7	52195.68	313.39	7.1	0.0	0.07	7.0	
84	7	52173.74	313.39	7.1	0.0	0.07	7.0	
85	6	52141.00	313.39	7.0	0.0	0.07	7.0	
86	6	52098.26	313.39	7.0	0.0	0.07	6.9	
87	6	52046.23	313.39	7.0	0.0	0.07	6.9	
88	6	51985.61	313.39	6.9	0.0	0.07	6.9	
89	6	51917.05	313.39	6.9	0.0	0.07	6.8	
90	5	51841.17	313.39	6.8	0.0	0.07	6.8	
91	5	51758.54	313.38	6.8	0.0	0.07	6.7	
92	5	51669.71	313.38	6.7	0.0	0.07	6.6	
93	5	51575.20	313.38	6.7	0.0	0.07	6.6	
94	5	51475.48	313.38	6.6	0.0	0.07	6.5	
95	5	51371.02	313.37	6.5	0.0	0.07	6.5	
96	5	51262.23	313.37	6.4	0.0	0.07	6.4	
97	4	51149.52	313.37	6.4	0.0	0.07	6.3	
98	4	51033.27	313.36	6.3	0.0	0.07	6.2	
99	4	50913.82	313.36	6.2	0.0	0.07	6.2	
100	4	50791.51	313.36	6.1	0.0	0.07	6.1	
101	4	50666.65	313.36	6.1	0.0	0.07	6.0	
102	4	50539.52	313.35	6.0	0.0	0.07	5.9	
103	4	50410.40	313.35	5.9	0.0	0.07	5.8	
104	4	50279.54	313.35	5.8	0.0	0.07	5.7	
105	4	50147.19	313.34	5.7	0.0	0.07	5.7	
106	3	50013.55	313.34	5.6	0.0	0.07	5.6	
107	3	49878.84	313.34	5.6	0.0	0.07	5.5	
108	3	49743.24	313.33	5.5	0.0	0.07	5.4	
109	3	49606.95	313.33	5.4	0.0	0.07	5.3	
110	3	49470.13	313.33	5.3	0.0	0.07	5.2	
111	3	49332.93	313.32	5.2	0.0	0.07	5.2	
112	3	49195.50	313.32	5.1	0.0	0.07	5.1	
113	3	49057.97	313.31	5.1	0.0	0.07	5.0	
114	3	48920.47	313.31	5.0	0.0	0.07	4.9	
115	3	48783.12	313.31	4.9	0.0	0.07	4.8	
116	3	48646.02	313.30	4.8	0.0	0.07	4.7	



# GRAPHED DATA FOR 10 YEAR STORM





**SCM-D  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-D

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	1.33 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	0.53 ac.	0.25
Undisturbed grassy area	0.00 ac.	0.80 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>1.33 ac.</b>	<b>1.33 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.31</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	284	feet	
Height of watershed =	8	feet	
Calculated t(c) =	2.4	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	5.0	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for **Raleigh, NC**

Return Period	g	h	Intensity
2	132	18	5.74 in/hr
10	195	22	7.22 in/hr
25	232	23	8.29 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.74 in/hr	3.4 cfs	2.37 cfs
10-year storm	7.22 in/hr	4.3 cfs	2.98 cfs
25-year storm	8.29 in/hr	5.0 cfs	3.42 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	95
Bottom width =	75
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	16
Height of berm =	6
Top of trap length =	131
Top of trap width =	111
Bottom elevation =	310

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	4788 cu. ft.	37420 cu. ft.	
Sediment surface area =	1880 sq. ft.	11780 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	1.3L:1W	Ratio too low
Spillway length =	10.0 ft. (min)	16	
10-Year flow depth over spillway =	0.50 ft. (max)	-2.59 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	4.59 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = **4,788** cubic feet

24 HOURS	Best Option =	1 - 2.5 inch Skimmers with a 2.2 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.6 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.3 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.2 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
310	7708		0	0			
311	9026	8367	8367	1	9.0321	0.0000	1.02
312	10410	9718	18085	2	9.8028	0.6931	1.96
313	11861	11135.5	29220.5	3	10.2826	1.0986	2.94
314	13378	12619.5	41840	4	10.6416	1.3863	3.99
315	14961	14169.5	56009.5	5	10.9333	1.6094	5.12

**STAGE-STORAGE FUNCTION:**

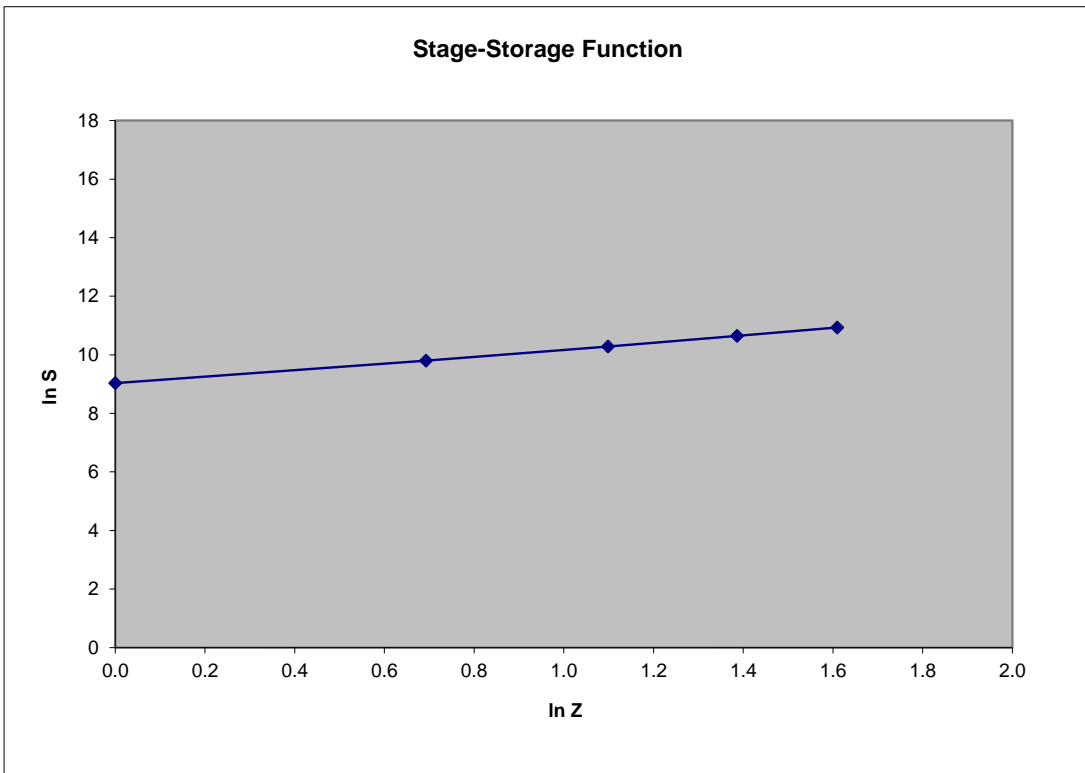
$$S = K_s * Z^b$$

where:

$b = 1.18$

and

$K_s = 8192$







# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	<b>1.33</b> ac	<b>Use Kirpich Equation:</b> Tc = <b>5.0 min</b>
Disturbed Area =	<b>1.33</b> ac	
Undisturbed Woods =	<b>0.00</b> ac	<b>Use Malcom Method:</b> i <sub>1</sub> = <b>5.74 in/hr</b> i <sub>10</sub> = <b>7.22 in/hr</b> i <sub>25</sub> = <b>8.29 in/hr</b>
Undisturbed Grass =	<b>0.00</b> ac	
Hydraulic Length =	<b>284</b> ft	
Vertical Fall =	<b>8</b> ft	<b>Use Rational Method:</b> Q <sub>2</sub> = <b>3.4 cfs</b> Q <sub>10</sub> = <b>4.3 cfs</b> Q <sub>25</sub> = <b>5.0 cfs</b>
For 1 yr Storm, g =	<b>132</b>	
h =	<b>18</b>	
For 10 yr Storm, g =	<b>195</b>	
h =	<b>22</b>	
For 25 yr Storm, g =	<b>232</b>	
h =	<b>23</b>	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u><u>0.45</u></u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	<b>2.7</b> in (2 Year Storm)	S = <b>0.99</b> in
CN =	<b>91</b> (newly graded areas)	Q* = <b>1.75</b> in
		T <sub>p2</sub> = <b>29.4</b> minutes
P <sub>n6</sub> =	<b>3.9</b> in (10 Year Storm)	Q* = <b>2.92</b> in
		T <sub>p10</sub> = <b>39.1</b> minutes
P <sub>n6</sub> =	<b>4.6</b> in (25 Year Storm)	Q* = <b>3.59</b> in
		T <sub>p25</sub> = <b>42.0</b> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	311.00	ft
	Rise =	1.00	ft
Freeboard =	5.00	ft	Peak Outflow =
			0.03
			cfs

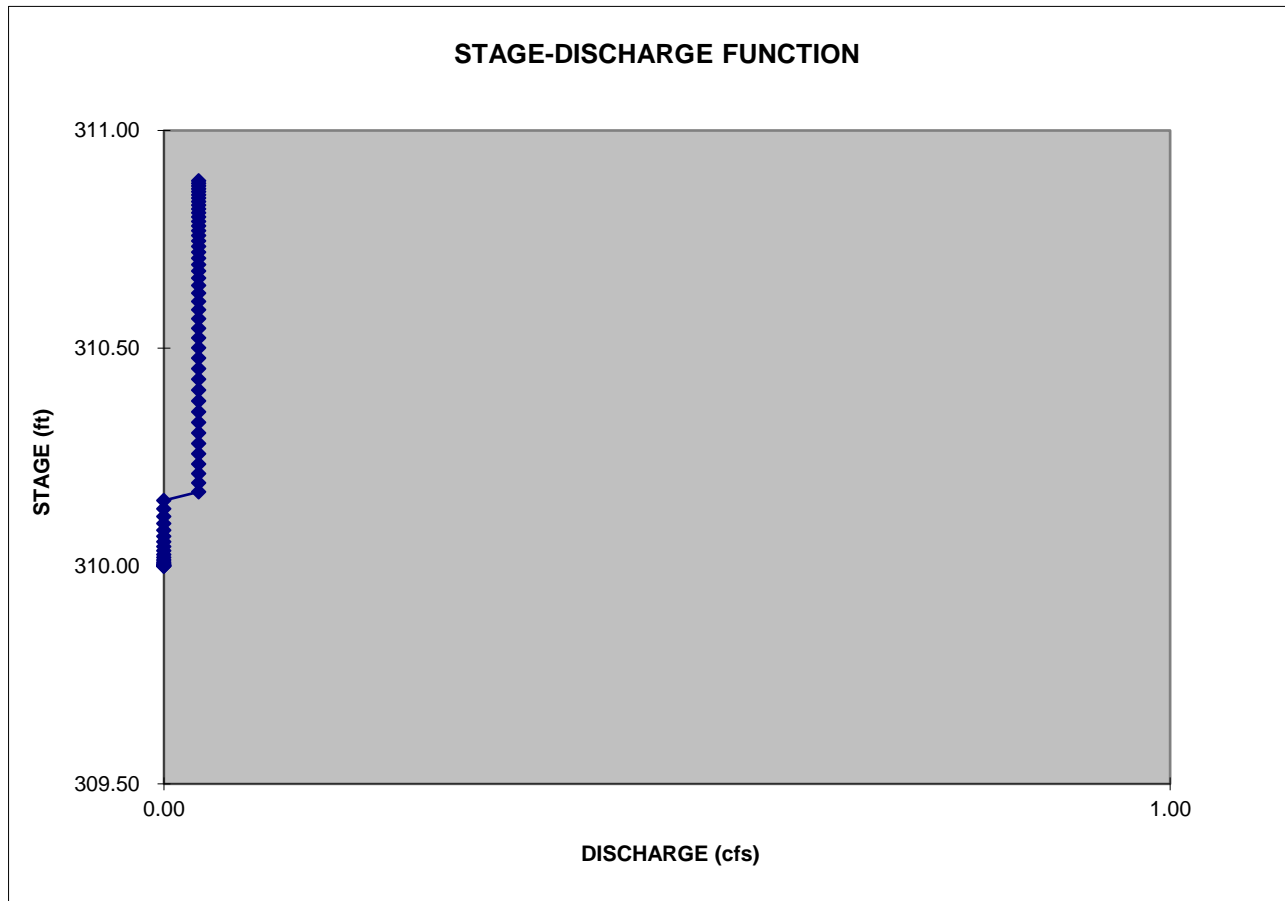
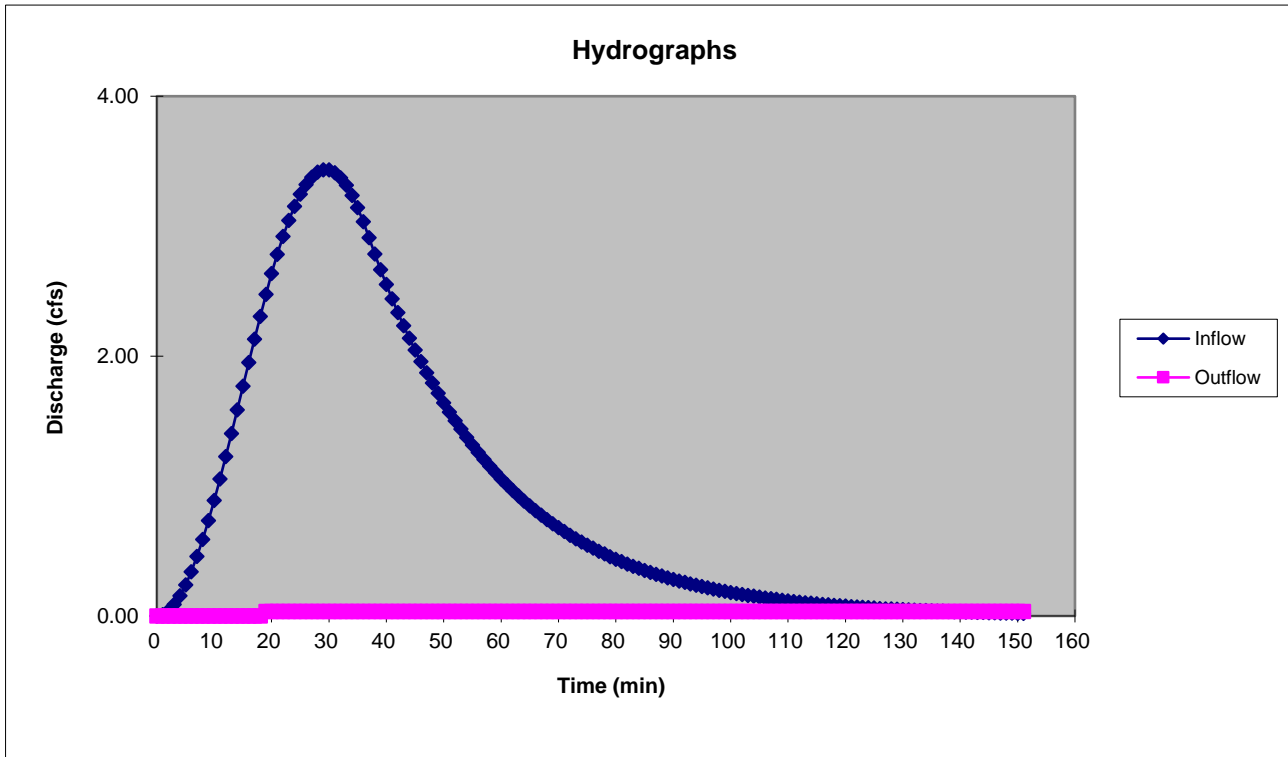
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft
Qp =	3.43	cfs	N =
			1
Tp =	29.4	min	L =
			16
dT =	1.0	min	Cw =
			3.00
<b>Stage-Storage Results:</b>			Zcr =
Ks =	8192		314.00
b =	1.18		
Z <sub>0</sub> =	310.0	ft (inv)	
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	310.00	ft	
			<b>Skimmer Orifice:</b>
			Number =
			1.00
			Diameter =
			2.00
			Head =
			1.35
			inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	0.59	310.00	0.0	0.0	0.00		
3	0	2.92	310.00	0.0	0.0	0.00		
4	0	8.15	310.00	0.0	0.0	0.00		
5	0	17.39	310.01	0.0	0.0	0.00		
6	0	31.71	310.01	0.0	0.0	0.00		
7	0	52.10	310.01	0.0	0.0	0.00		
8	1	79.52	310.02	0.0	0.0	0.00		
9	1	114.83	310.03	0.0	0.0	0.00		
10	1	158.78	310.04	0.0	0.0	0.00		
11	1	212.06	310.04	0.0	0.0	0.00		
12	1	275.23	310.06	0.0	0.0	0.00		
13	1	348.74	310.07	0.0	0.0	0.00		
14	2	432.93	310.08	0.0	0.0	0.00		
15	2	528.02	310.10	0.0	0.0	0.00		
16	2	634.09	310.11	0.0	0.0	0.00		
17	2	751.11	310.13	0.0	0.0	0.00		
18	2	878.92	310.15	0.0	0.0	0.00		
19	2	1017.24	310.17	0.0	0.0	0.03		
20	3	1163.59	310.19	0.0	0.0	0.03		
21	3	1319.54	310.21	0.0	0.0	0.03		
22	3	1484.45	310.23	0.0	0.0	0.03		
23	3	1657.60	310.26	0.0	0.0	0.03		
24	3	1838.18	310.28	0.0	0.0	0.03		
25	3	2025.27	310.31	0.0	0.0	0.03		
26	3	2217.89	310.33	0.0	0.0	0.03		
27	3	2415.01	310.35	0.0	0.0	0.03		
28	3	2615.53	310.38	0.0	0.0	0.03		
29	3	2818.31	310.40	0.0	0.0	0.03		
30	3	3022.21	310.43	0.0	0.0	0.03		
31	3	3226.05	310.45	0.0	0.0	0.03		
32	3	3428.66	310.48	0.0	0.0	0.03		
33	3	3628.89	310.50	0.0	0.0	0.03		
34	3	3825.60	310.52	0.0	0.0	0.03		
35	3	4017.72	310.55	0.0	0.0	0.03		
36	3	4204.20	310.57	0.0	0.0	0.03		
37	3	4384.06	310.59	0.0	0.0	0.03		
38	3	4556.66	310.61	0.0	0.0	0.03		
39	3	4721.71	310.63	0.0	0.0	0.03		
40	3	4879.55	310.64	0.0	0.0	0.03		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	4.59	ft	
Peak Stage =	311.41	ft	
Rise =	1.41	ft	
Peak Outflow =	0.03	cfs	

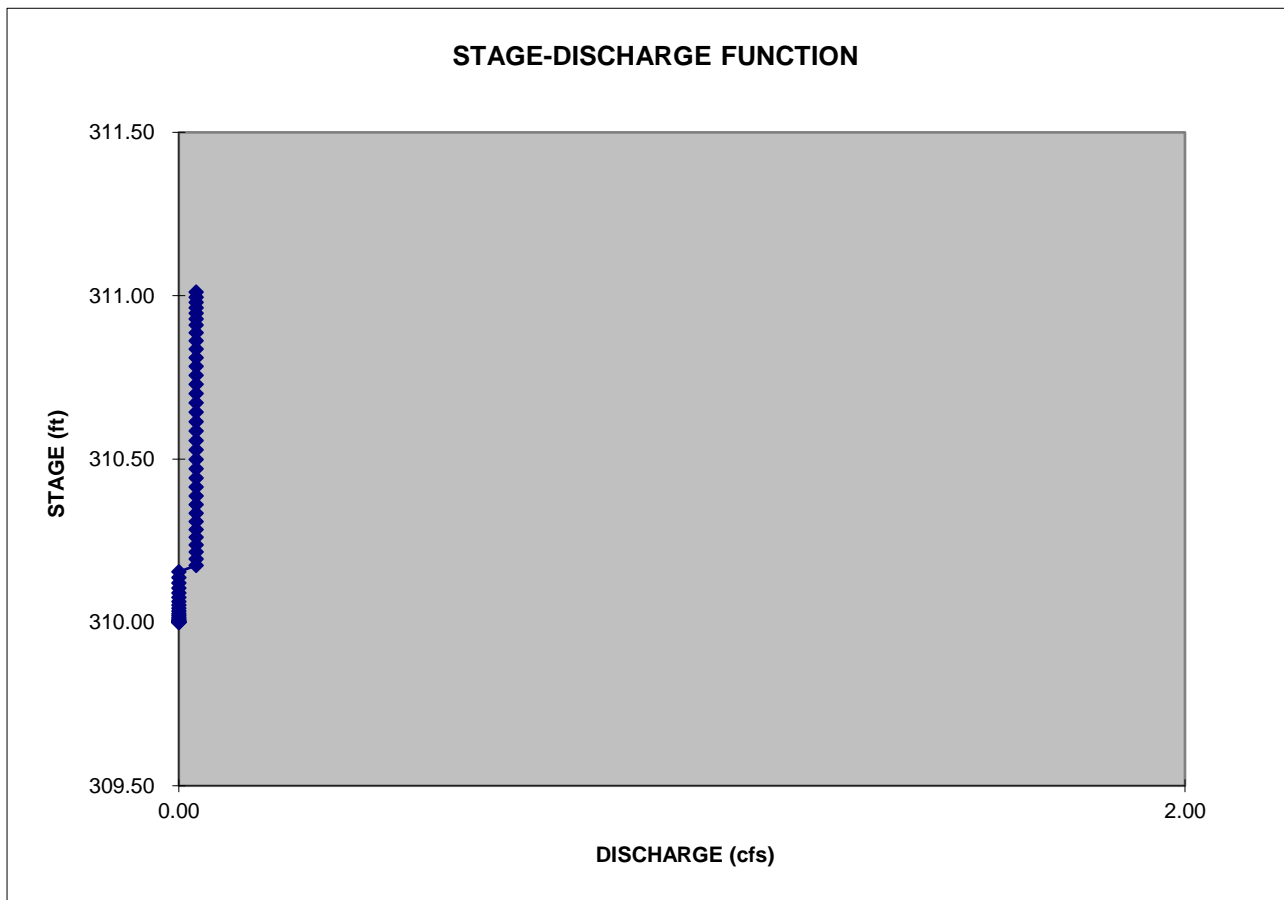
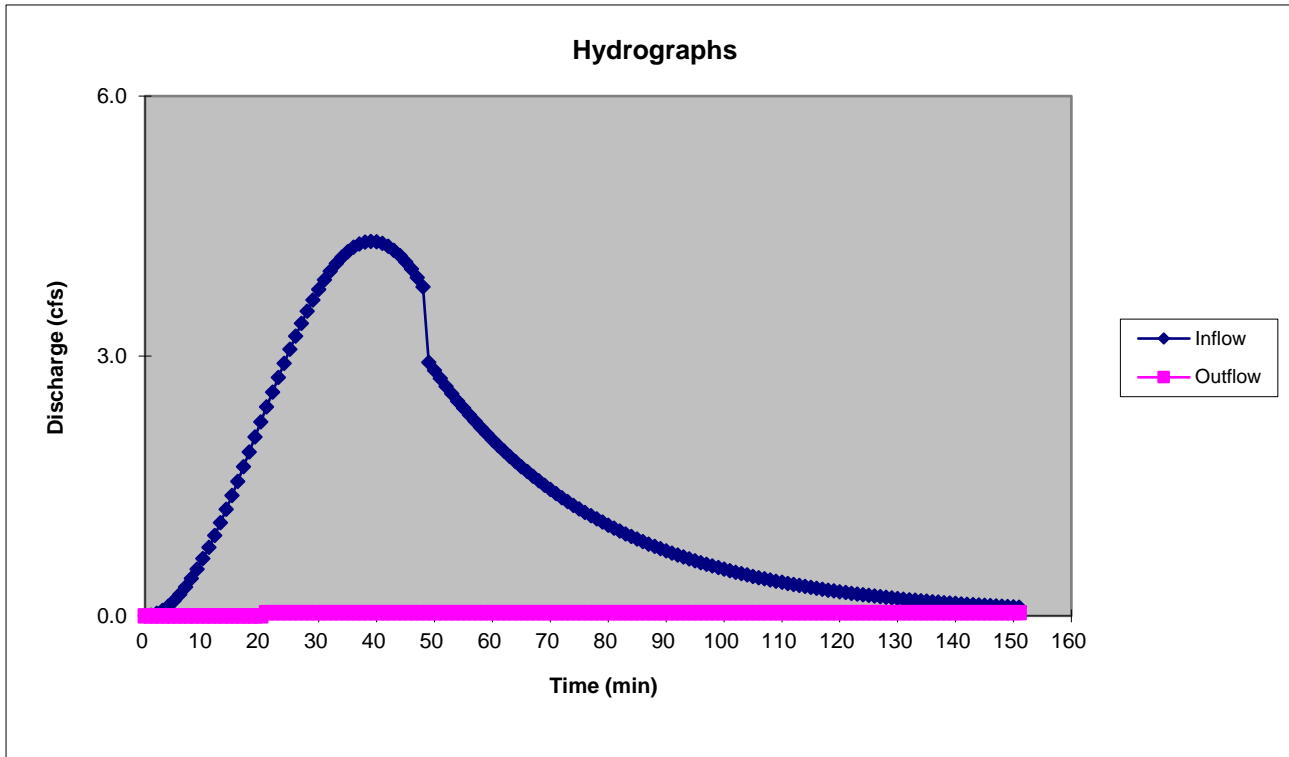
INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft	
Qp = 4.32 cfs	N = 1		
Tp = 39.1 min	L = 16 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Number = 1 Ea	
Ks = 8192		Diameter = 2.00 Inches	
b = 1.18		Head = 1.35 inches	
Z <sub>0</sub> = 310.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	0.42	310.00	0.0	0.0	0.00		
3	0	2.09	310.00	0.0	0.0	0.00		
4	0	5.83	310.00	0.0	0.0	0.00		
5	0	12.46	310.00	0.0	0.0	0.00		
6	0	22.77	310.01	0.0	0.0	0.00		
7	0	37.53	310.01	0.0	0.0	0.00		
8	0	57.47	310.01	0.0	0.0	0.00		
9	1	83.32	310.02	0.0	0.0	0.00		
10	1	115.72	310.03	0.0	0.0	0.00		
11	1	155.32	310.03	0.0	0.0	0.00		
12	1	202.69	310.04	0.0	0.0	0.00		
13	1	258.37	310.05	0.0	0.0	0.00		
14	1	322.82	310.06	0.0	0.0	0.00		
15	1	396.47	310.08	0.0	0.0	0.00		
16	2	479.68	310.09	0.0	0.0	0.00		
17	2	572.75	310.10	0.0	0.0	0.00		
18	2	675.92	310.12	0.0	0.0	0.00		
19	2	789.35	310.14	0.0	0.0	0.00		
20	2	913.16	310.16	0.0	0.0	0.00		
21	2	1047.37	310.17	0.0	0.0	0.03		
22	3	1189.88	310.19	0.0	0.0	0.03		
23	3	1342.68	310.22	0.0	0.0	0.03		
24	3	1505.60	310.24	0.0	0.0	0.03		
25	3	1678.42	310.26	0.0	0.0	0.03		
26	3	1860.83	310.28	0.0	0.0	0.03		
27	3	2052.49	310.31	0.0	0.0	0.03		
28	4	2253.00	310.33	0.0	0.0	0.03		
29	4	2461.86	310.36	0.0	0.0	0.03		
30	4	2678.57	310.39	0.0	0.0	0.03		
31	4	2902.55	310.41	0.0	0.0	0.03		
32	4	3133.18	310.44	0.0	0.0	0.03		
33	4	3369.79	310.47	0.0	0.0	0.03		
34	4	3611.68	310.50	0.0	0.0	0.03		
35	4	3858.11	310.53	0.0	0.0	0.03		
36	4	4108.33	310.56	0.0	0.0	0.03		
37	4	4361.54	310.59	0.0	0.0	0.03		
38	4	4616.92	310.61	0.0	0.0	0.03		
39	4	4873.67	310.64	0.0	0.0	0.03		
40	4	5130.93	310.67	0.0	0.0	0.03		
41	4	5387.89	310.70	0.0	0.0	0.03		
42	4	5643.70	310.73	0.0	0.0	0.03		
43	4	5897.55	310.76	0.0	0.0	0.03		
44	4	6148.61	310.78	0.0	0.0	0.03		
45	4	6396.09	310.81	0.0	0.0	0.03		
46	4	6639.22	310.84	0.0	0.0	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	4	6877.25	310.86	0.0	0.0	0.03		
48	4	7109.48	310.89	0.0	0.0	0.03		
49	3	7335.23	310.91	0.0	0.0	0.03		
50	3	7508.75	310.93	0.0	0.0	0.03		
51	3	7676.54	310.95	0.0	0.0	0.03		
52	3	7838.78	310.96	0.0	0.0	0.03		
53	3	7995.65	310.98	0.0	0.0	0.03		
54	2	8147.33	311.00	0.0	0.0	0.03		
55	2	8293.98	311.01	0.0	0.0	0.03		
56	2	8435.77	311.03	0.0	0.0	0.03		
57	2	8572.86	311.04	0.0	0.0	0.03		
58	2	8705.40	311.05	0.0	0.0	0.03		
59	2	8833.54	311.07	0.0	0.0	0.03		
60	2	8957.43	311.08	0.0	0.0	0.03		
61	2	9077.20	311.09	0.0	0.0	0.03		
62	2	9192.99	311.10	0.0	0.0	0.03		
63	2	9304.92	311.11	0.0	0.0	0.03		
64	2	9413.14	311.13	0.0	0.0	0.03		
65	2	9517.74	311.14	0.0	0.0	0.03		
66	2	9618.87	311.15	0.0	0.0	0.03		
67	2	9716.62	311.16	0.0	0.0	0.03		
68	2	9811.10	311.17	0.0	0.0	0.03		
69	2	9902.44	311.17	0.0	0.0	0.03		
70	1	9990.72	311.18	0.0	0.0	0.03		
71	1	10076.04	311.19	0.0	0.0	0.03		
72	1	10158.51	311.20	0.0	0.0	0.03		
73	1	10238.22	311.21	0.0	0.0	0.03		
74	1	10315.26	311.22	0.0	0.0	0.03		
75	1	10389.71	311.22	0.0	0.0	0.03		
76	1	10461.66	311.23	0.0	0.0	0.03		
77	1	10531.19	311.24	0.0	0.0	0.03		
78	1	10598.38	311.24	0.0	0.0	0.03		
79	1	10663.31	311.25	0.0	0.0	0.03		
80	1	10726.04	311.26	0.0	0.0	0.03		
81	1	10786.66	311.26	0.0	0.0	0.03		
82	1	10845.23	311.27	0.0	0.0	0.03		
83	1	10901.82	311.27	0.0	0.0	0.03		
84	1	10956.50	311.28	0.0	0.0	0.03		
85	1	11009.31	311.29	0.0	0.0	0.03		
86	1	11060.34	311.29	0.0	0.0	0.03		
87	1	11109.63	311.30	0.0	0.0	0.03		
88	1	11157.24	311.30	0.0	0.0	0.03		
89	1	11203.22	311.30	0.0	0.0	0.03		
90	1	11247.64	311.31	0.0	0.0	0.03		
91	1	11290.54	311.31	0.0	0.0	0.03		
92	1	11331.96	311.32	0.0	0.0	0.03		
93	1	11371.97	311.32	0.0	0.0	0.03		
94	1	11410.60	311.32	0.0	0.0	0.03		
95	1	11447.90	311.33	0.0	0.0	0.03		
96	1	11483.91	311.33	0.0	0.0	0.03		
97	1	11518.68	311.34	0.0	0.0	0.03		
98	1	11552.25	311.34	0.0	0.0	0.03		
99	1	11584.65	311.34	0.0	0.0	0.03		
100	1	11615.92	311.35	0.0	0.0	0.03		
101	1	11646.11	311.35	0.0	0.0	0.03		
102	1	11675.24	311.35	0.0	0.0	0.03		
103	0	11703.35	311.35	0.0	0.0	0.03		
104	0	11730.47	311.36	0.0	0.0	0.03		
105	0	11756.64	311.36	0.0	0.0	0.03		
106	0	11781.89	311.36	0.0	0.0	0.03		
107	0	11806.24	311.36	0.0	0.0	0.03		
108	0	11829.73	311.37	0.0	0.0	0.03		
109	0	11852.38	311.37	0.0	0.0	0.03		
110	0	11874.23	311.37	0.0	0.0	0.03		
111	0	11895.30	311.37	0.0	0.0	0.03		
112	0	11915.60	311.37	0.0	0.0	0.03		
113	0	11935.18	311.38	0.0	0.0	0.03		
114	0	11954.05	311.38	0.0	0.0	0.03		
115	0	11972.24	311.38	0.0	0.0	0.03		
116	0	11989.76	311.38	0.0	0.0	0.03		
117	0	12006.64	311.38	0.0	0.0	0.03		
118	0	12022.91	311.39	0.0	0.0	0.03		
119	0	12038.57	311.39	0.0	0.0	0.03		
120	0	12053.65	311.39	0.0	0.0	0.03		
121	0	12068.18	311.39	0.0	0.0	0.03		
122	0	12082.16	311.39	0.0	0.0	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	12095.62	311.39	0.0	0.0	0.03		
124	0	12108.56	311.39	0.0	0.0	0.03		
125	0	12121.02	311.39	0.0	0.0	0.03		
126	0	12133.01	311.40	0.0	0.0	0.03		
127	0	12144.53	311.40	0.0	0.0	0.03		
128	0	12155.61	311.40	0.0	0.0	0.03		
129	0	12166.26	311.40	0.0	0.0	0.03		
130	0	12176.49	311.40	0.0	0.0	0.03		
131	0	12186.32	311.40	0.0	0.0	0.03		
132	0	12195.76	311.40	0.0	0.0	0.03		
133	0	12204.83	311.40	0.0	0.0	0.03		
134	0	12213.53	311.40	0.0	0.0	0.03		
135	0	12221.88	311.40	0.0	0.0	0.03		
136	0	12229.89	311.41	0.0	0.0	0.03		
137	0	12237.57	311.41	0.0	0.0	0.03		
138	0	12244.93	311.41	0.0	0.0	0.03		
139	0	12251.98	311.41	0.0	0.0	0.03		
140	0	12258.74	311.41	0.0	0.0	0.03		
141	0	12265.20	311.41	0.0	0.0	0.03		
142	0	12271.39	311.41	0.0	0.0	0.03		
143	0	12277.31	311.41	0.0	0.0	0.03		
144	0	12282.96	311.41	0.0	0.0	0.03		
145	0	12288.36	311.41	0.0	0.0	0.03		
146	0	12293.52	311.41	0.0	0.0	0.03		
147	0	12298.45	311.41	0.0	0.0	0.03		
148	0	12303.14	311.41	0.0	0.0	0.03		
149	0	12307.61	311.41	0.0	0.0	0.03		
150	0	12311.87	311.41	0.0	0.0	0.03		
151	0	12315.92	311.41	0.0	0.0	0.03		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	4.48	ft	
Peak Stage =	311.52	ft	
Rise =	1.52	ft	
Peak Outflow =	0.03	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	316.00 ft
Qp = 4.96 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.1 min	L = 16 ft	Number = 1.00 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Head = 1.35 inches	
Ks = 8192			
b = 1.18			
Z <sub>0</sub> = 310.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	0.48	310.00	0.0	0.0	0.00		
3	0	2.39	310.00	0.0	0.0	0.00		
4	0	6.69	310.00	0.0	0.0	0.00		
5	0	14.29	310.00	0.0	0.0	0.00		
6	0	26.12	310.01	0.0	0.0	0.00		
7	0	43.05	310.01	0.0	0.0	0.00		
8	0	65.94	310.02	0.0	0.0	0.00		
9	1	95.59	310.02	0.0	0.0	0.00		
10	1	132.77	310.03	0.0	0.0	0.00		
11	1	178.20	310.04	0.0	0.0	0.00		
12	1	232.54	310.05	0.0	0.0	0.00		
13	1	296.41	310.06	0.0	0.0	0.00		
14	1	370.36	310.07	0.0	0.0	0.00		
15	2	454.85	310.09	0.0	0.0	0.00		
16	2	550.32	310.10	0.0	0.0	0.00		
17	2	657.09	310.12	0.0	0.0	0.00		
18	2	775.45	310.14	0.0	0.0	0.00		
19	2	905.59	310.15	0.0	0.0	0.00		
20	3	1047.62	310.17	0.0	0.0	0.03		
21	3	1199.52	310.20	0.0	0.0	0.03		
22	3	1363.33	310.22	0.0	0.0	0.03		
23	3	1538.93	310.24	0.0	0.0	0.03		
24	3	1726.15	310.27	0.0	0.0	0.03		
25	4	1924.71	310.29	0.0	0.0	0.03		
26	4	2134.30	310.32	0.0	0.0	0.03		
27	4	2354.49	310.35	0.0	0.0	0.03		
28	4	2584.82	310.38	0.0	0.0	0.03		
29	4	2824.75	310.40	0.0	0.0	0.03		
30	4	3073.67	310.43	0.0	0.0	0.03		
31	4	3330.94	310.47	0.0	0.0	0.03		
32	5	3595.83	310.50	0.0	0.0	0.03		
33	5	3867.59	310.53	0.0	0.0	0.03		
34	5	4145.40	310.56	0.0	0.0	0.03		
35	5	4428.43	310.59	0.0	0.0	0.03		
36	5	4715.80	310.63	0.0	0.0	0.03		
37	5	5006.60	310.66	0.0	0.0	0.03		
38	5	5299.90	310.69	0.0	0.0	0.03		
39	5	5594.75	310.72	0.0	0.0	0.03		
40	5	5890.21	310.76	0.0	0.0	0.03		
41	5	6185.31	310.79	0.0	0.0	0.03		
42	5	6479.10	310.82	0.0	0.0	0.03		
43	5	6770.63	310.85	0.0	0.0	0.03		
44	5	7058.96	310.88	0.0	0.0	0.03		
45	5	7343.19	310.91	0.0	0.0	0.03		
46	5	7622.43	310.94	0.0	0.0	0.03		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	4	7895.82	310.97	0.0	0.0	0.03		
48	4	8162.55	311.00	0.0	0.0	0.03		
49	3	8421.84	311.02	0.0	0.0	0.03		
50	3	8595.37	311.04	0.0	0.0	0.03		
51	3	8763.16	311.06	0.0	0.0	0.03		
52	3	8925.39	311.08	0.0	0.0	0.03		
53	3	9082.26	311.09	0.0	0.0	0.03		
54	2	9233.94	311.11	0.0	0.0	0.03		
55	2	9380.59	311.12	0.0	0.0	0.03		
56	2	9522.38	311.14	0.0	0.0	0.03		
57	2	9659.47	311.15	0.0	0.0	0.03		
58	2	9792.01	311.16	0.0	0.0	0.03		
59	2	9920.15	311.18	0.0	0.0	0.03		
60	2	10044.04	311.19	0.0	0.0	0.03		
61	2	10163.81	311.20	0.0	0.0	0.03		
62	2	10279.60	311.21	0.0	0.0	0.03		
63	2	10391.54	311.22	0.0	0.0	0.03		
64	2	10499.75	311.23	0.0	0.0	0.03		
65	2	10604.36	311.25	0.0	0.0	0.03		
66	2	10705.48	311.26	0.0	0.0	0.03		
67	2	10803.23	311.26	0.0	0.0	0.03		
68	2	10897.72	311.27	0.0	0.0	0.03		
69	2	10989.05	311.28	0.0	0.0	0.03		
70	1	11077.33	311.29	0.0	0.0	0.03		
71	1	11162.66	311.30	0.0	0.0	0.03		
72	1	11245.13	311.31	0.0	0.0	0.03		
73	1	11324.83	311.32	0.0	0.0	0.03		
74	1	11401.87	311.32	0.0	0.0	0.03		
75	1	11476.32	311.33	0.0	0.0	0.03		
76	1	11548.27	311.34	0.0	0.0	0.03		
77	1	11617.80	311.35	0.0	0.0	0.03		
78	1	11684.99	311.35	0.0	0.0	0.03		
79	1	11749.92	311.36	0.0	0.0	0.03		
80	1	11812.66	311.36	0.0	0.0	0.03		
81	1	11873.28	311.37	0.0	0.0	0.03		
82	1	11931.85	311.38	0.0	0.0	0.03		
83	1	11988.44	311.38	0.0	0.0	0.03		
84	1	12043.11	311.39	0.0	0.0	0.03		
85	1	12095.93	311.39	0.0	0.0	0.03		
86	1	12146.95	311.40	0.0	0.0	0.03		
87	1	12196.24	311.40	0.0	0.0	0.03		
88	1	12243.85	311.41	0.0	0.0	0.03		
89	1	12289.84	311.41	0.0	0.0	0.03		
90	1	12334.25	311.42	0.0	0.0	0.03		
91	1	12377.15	311.42	0.0	0.0	0.03		
92	1	12418.58	311.42	0.0	0.0	0.03		
93	1	12458.58	311.43	0.0	0.0	0.03		
94	1	12497.21	311.43	0.0	0.0	0.03		
95	1	12534.51	311.43	0.0	0.0	0.03		
96	1	12570.53	311.44	0.0	0.0	0.03		
97	1	12605.30	311.44	0.0	0.0	0.03		
98	1	12638.86	311.45	0.0	0.0	0.03		
99	1	12671.26	311.45	0.0	0.0	0.03		
100	1	12702.54	311.45	0.0	0.0	0.03		
101	1	12732.72	311.45	0.0	0.0	0.03		
102	1	12761.85	311.46	0.0	0.0	0.03		
103	0	12789.96	311.46	0.0	0.0	0.03		
104	0	12817.08	311.46	0.0	0.0	0.03		
105	0	12843.25	311.46	0.0	0.0	0.03		
106	0	12868.50	311.47	0.0	0.0	0.03		
107	0	12892.85	311.47	0.0	0.0	0.03		
108	0	12916.34	311.47	0.0	0.0	0.03		
109	0	12939.00	311.47	0.0	0.0	0.03		
110	0	12960.84	311.48	0.0	0.0	0.03		
111	0	12981.91	311.48	0.0	0.0	0.03		
112	0	13002.22	311.48	0.0	0.0	0.03		
113	0	13021.79	311.48	0.0	0.0	0.03		
114	0	13040.66	311.48	0.0	0.0	0.03		
115	0	13058.85	311.49	0.0	0.0	0.03		
116	0	13076.37	311.49	0.0	0.0	0.03		
117	0	13093.26	311.49	0.0	0.0	0.03		
118	0	13109.52	311.49	0.0	0.0	0.03		
119	0	13125.18	311.49	0.0	0.0	0.03		
120	0	13140.27	311.49	0.0	0.0	0.03		
121	0	13154.79	311.50	0.0	0.0	0.03		
122	0	13168.77	311.50	0.0	0.0	0.03		

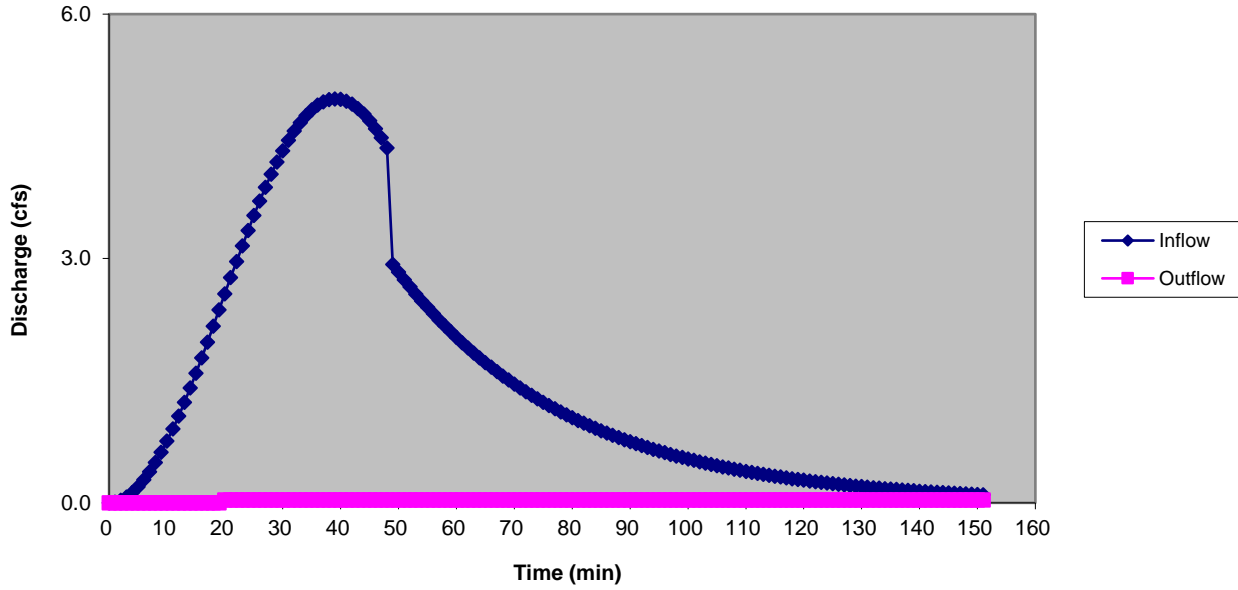
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	13182.23	311.50	0.0	0.0	0.03		
124	0	13195.18	311.50	0.0	0.0	0.03		
125	0	13207.64	311.50	0.0	0.0	0.03		
126	0	13219.62	311.50	0.0	0.0	0.03		
127	0	13231.14	311.50	0.0	0.0	0.03		
128	0	13242.22	311.50	0.0	0.0	0.03		
129	0	13252.87	311.50	0.0	0.0	0.03		
130	0	13263.10	311.51	0.0	0.0	0.03		
131	0	13272.94	311.51	0.0	0.0	0.03		
132	0	13282.38	311.51	0.0	0.0	0.03		
133	0	13291.44	311.51	0.0	0.0	0.03		
134	0	13300.15	311.51	0.0	0.0	0.03		
135	0	13308.50	311.51	0.0	0.0	0.03		
136	0	13316.50	311.51	0.0	0.0	0.03		
137	0	13324.18	311.51	0.0	0.0	0.03		
138	0	13331.54	311.51	0.0	0.0	0.03		
139	0	13338.60	311.51	0.0	0.0	0.03		
140	0	13345.35	311.51	0.0	0.0	0.03		
141	0	13351.82	311.51	0.0	0.0	0.03		
142	0	13358.00	311.51	0.0	0.0	0.03		
143	0	13363.92	311.52	0.0	0.0	0.03		
144	0	13369.58	311.52	0.0	0.0	0.03		
145	0	13374.98	311.52	0.0	0.0	0.03		
146	0	13380.14	311.52	0.0	0.0	0.03		
147	0	13385.06	311.52	0.0	0.0	0.03		
148	0	13389.75	311.52	0.0	0.0	0.03		
149	0	13394.22	311.52	0.0	0.0	0.03		
150	0	13398.48	311.52	0.0	0.0	0.03		
151	0	13402.53	311.52	0.0	0.0	0.03		



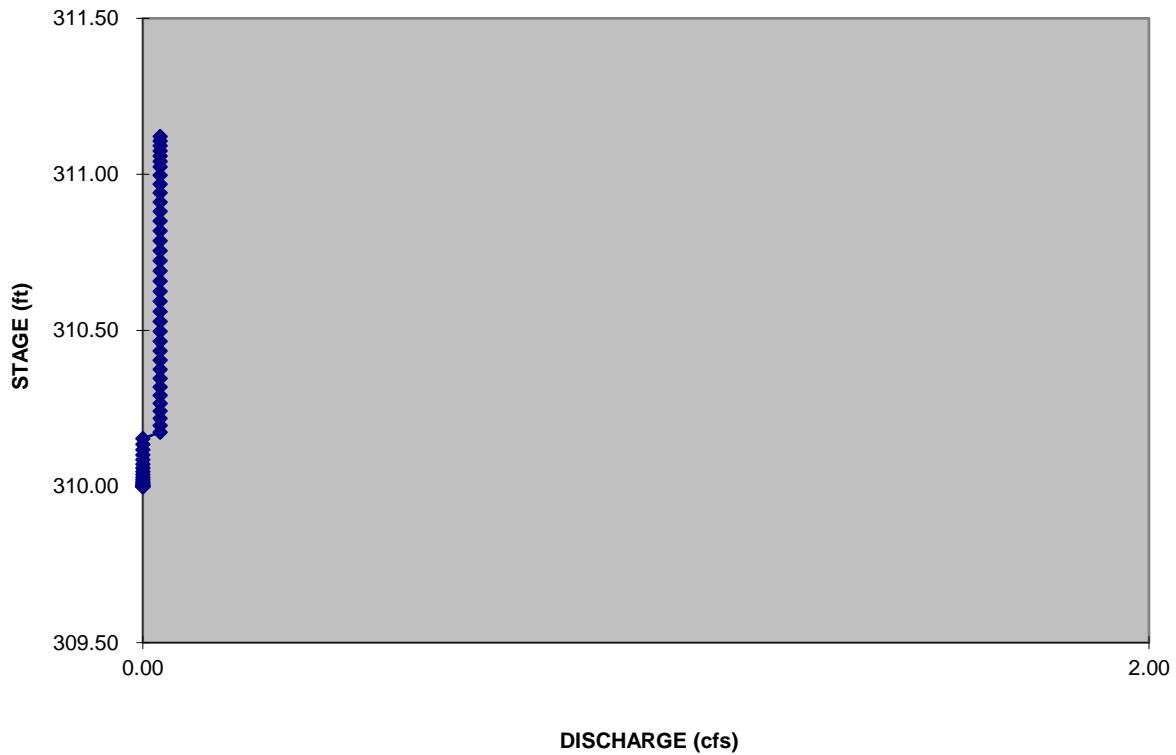
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM-E  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-E

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	9.51 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	4.75 ac.	0.25
Undisturbed grassy area	0.00 ac.	4.76 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>9.51 ac.</b>	<b>9.51 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.30</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	1063	feet	
Height of watershed =	39	feet	
Calculated t(c) =	6.0	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	6.0	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	5.51 in/hr
10	195	22	6.97 in/hr
25	232	23	8.01 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.51 in/hr	23.6 cfs	15.71 cfs
10-year storm	6.97 in/hr	29.8 cfs	19.89 cfs
25-year storm	8.01 in/hr	34.3 cfs	22.85 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	190
Bottom width =	94
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	20
Height of berm =	6
Top of trap length =	226
Top of trap width =	130
Bottom elevation =	276

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	34236 cu. ft.	85790 cu. ft.	
Sediment surface area =	12980 sq. ft.	25250 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W	
Spillway length =	10.0 ft. (min)	20	
10-Year flow depth over spillway =	0.50 ft. (max)	0.06 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.94 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 34,236 cubic feet

24 HOURS	Best Option =	2 - 4 inch Skimmers with a 3.7 inch orifice
2 DAYS	Best Option =	1 - 4 inch Skimmers with a 3.7 inch orifice
3 DAYS	Best Option =	1 - 4 inch Skimmers with a 3 inch orifice
4 DAYS	Best Option =	1 - 3 inch Skimmers with a 2.8 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

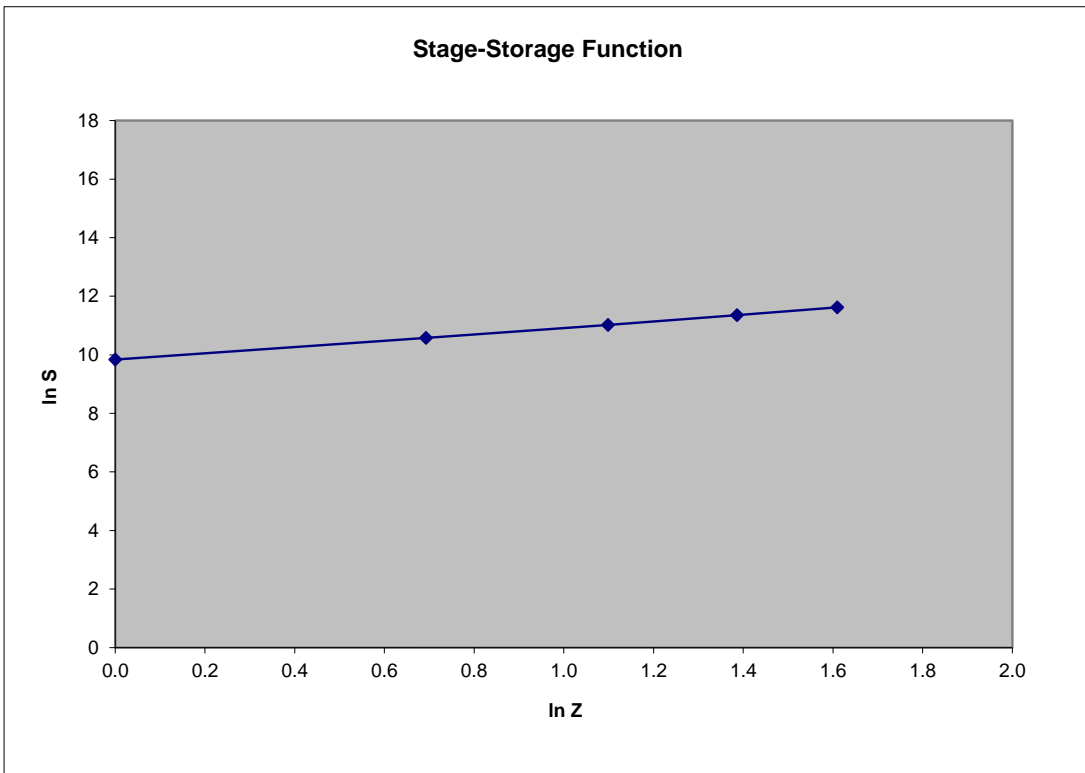
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
276	17888		0	0			
277	19563	18726	18725.5	1	9.8376	0.0000	1.01
278	21295	20429	39154.5	2	10.5753	0.6931	1.97
279	23083	22189	61343.5	3	11.0242	1.0986	2.96
280	24928	24005.5	85349	4	11.3545	1.3863	4.00
281	26829	25878.5	111227.5	5	11.6193	1.6094	5.08

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.10$  and  $K_s = 18482$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	9.51 ac	<b>Use Kirpich Equation:</b> Tc = 6.0 min
Disturbed Area =	9.51 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.51 in/hr i <sub>10</sub> = 6.97 in/hr i <sub>25</sub> = 8.01 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	1063 ft	
Vertical Fall =	39 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 23.6 cfs Q <sub>10</sub> = 29.8 cfs Q <sub>25</sub> = 34.3 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 30.7 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 40.5 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 43.4 minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	<b>Peak Stage =</b>	<b>278.84</b>	<b>ft</b>
	<b>Rise =</b>	<b>2.84</b>	<b>ft</b>
<b>Freeboard =</b>	<b>3.16</b>		
	<b>Peak Outflow =</b>	<b>0.21</b>	<b>cfs</b>

INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	
Qp =	<b>23.57</b> cfs	N =	<b>1</b>
Tp =	<b>30.7</b> min	L =	<b>20</b> ft
dT =	<b>1.0</b> min	Cw =	<b>3.00</b>
<b>Stage-Storage Results:</b>		Zcr =	<b>280.00</b> ft
Ks =	<b>18482</b>	<b>Skimmer Orifice:</b>	
b =	<b>1.10</b>	Number =	<b>1.00</b> Ea
Z <sub>0</sub> =	<b>276.0</b> ft (inv)	Diameter =	<b>4.00</b> Inches
<b>Initial Water Level:</b>		Head =	<b>3.01</b> inches
Z <sub>i</sub> =	<b>276.00</b> ft		
		<b>Top of Dam:</b> style="text-align: center;"> <b>282.00</b> ft	

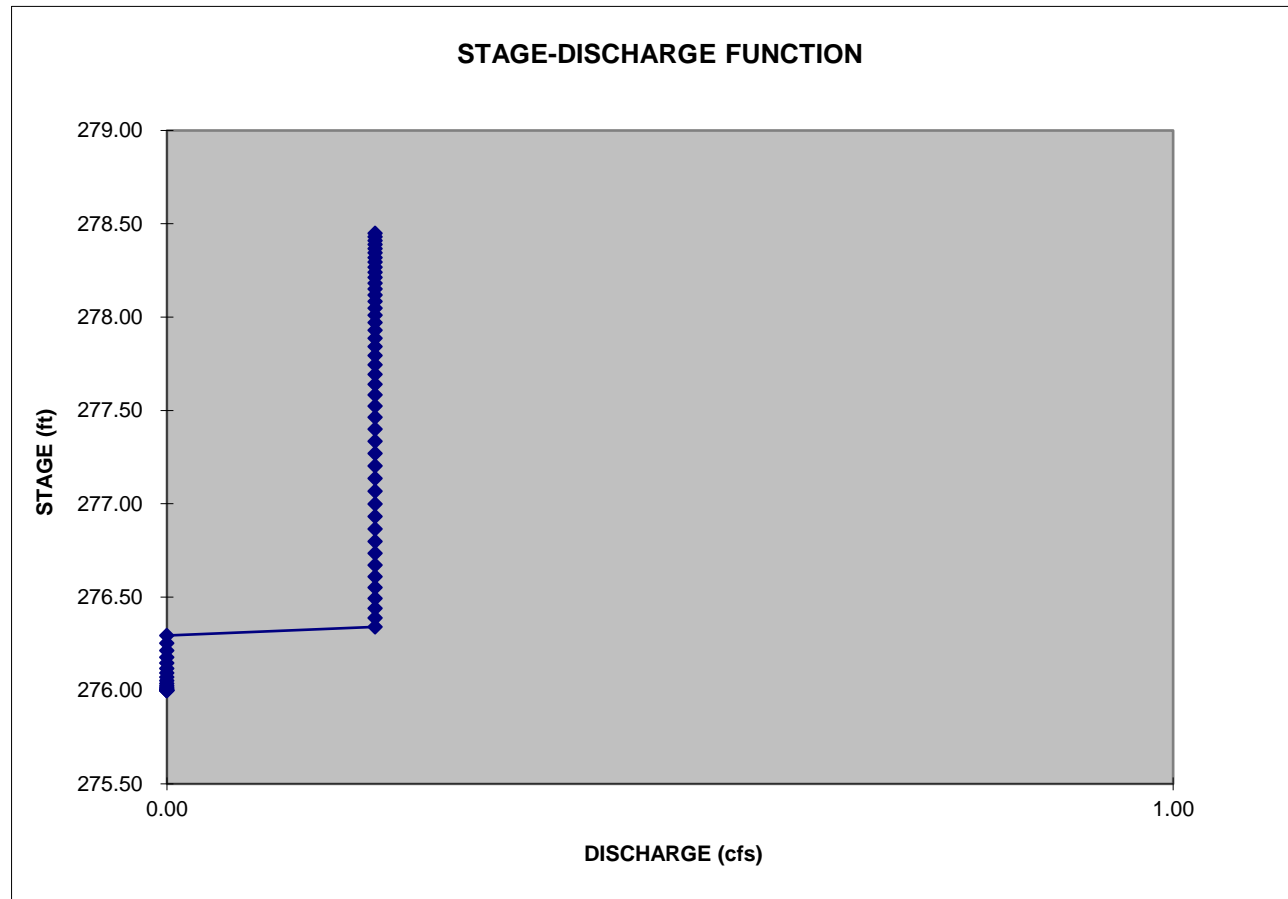
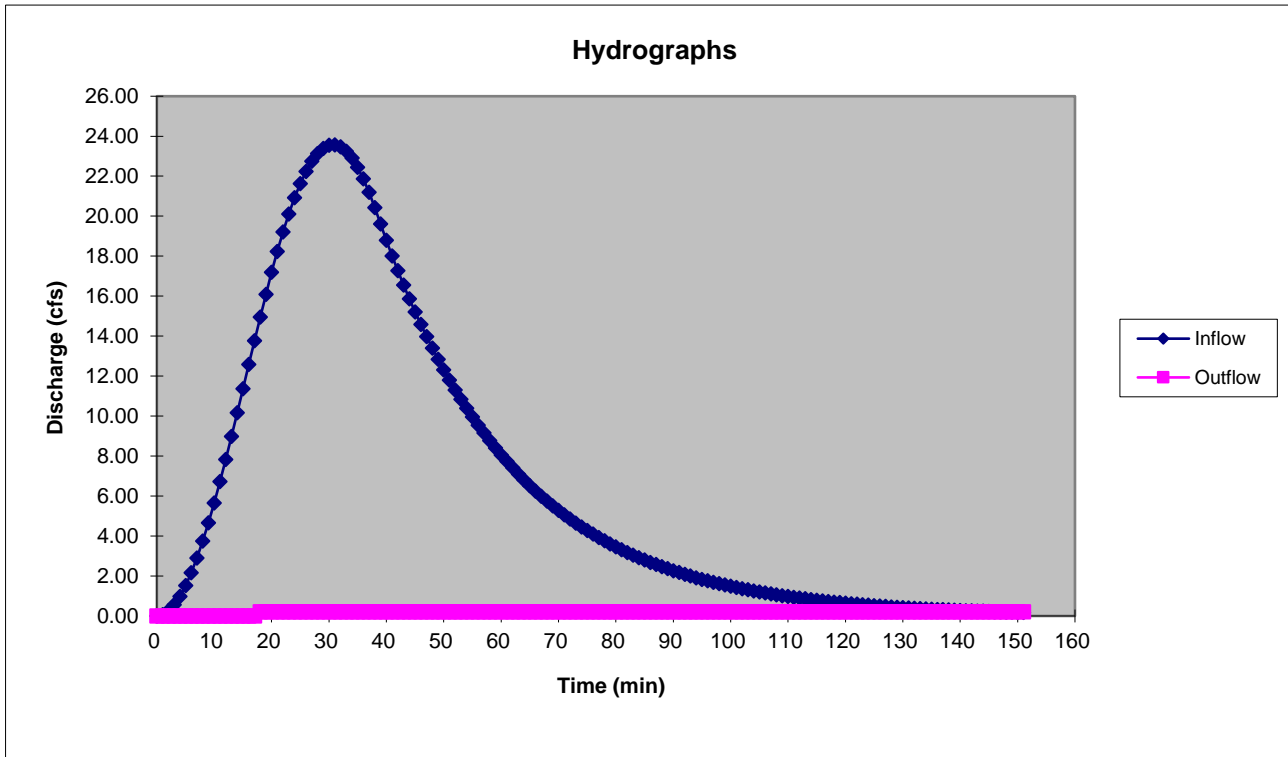
OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	276.00	0.0	0.0	0.0		
1	0	0.00	276.00	0.0	0.0	0.00		
2	0	3.70	276.00	0.0	0.0	0.00		
3	1	18.47	276.00	0.0	0.0	0.00		
4	1	51.55	276.00	0.0	0.0	0.00		
5	2	109.99	276.01	0.0	0.0	0.00		
6	2	200.60	276.02	0.0	0.0	0.00		
7	3	329.82	276.03	0.0	0.0	0.00		
8	4	503.70	276.04	0.0	0.0	0.00		
9	5	727.82	276.05	0.0	0.0	0.00		
10	6	1007.25	276.07	0.0	0.0	0.00		
11	7	1346.45	276.09	0.0	0.0	0.00		
12	8	1749.28	276.12	0.0	0.0	0.00		
13	9	2218.93	276.15	0.0	0.0	0.00		
14	10	2757.87	276.18	0.0	0.0	0.00		
15	11	3367.87	276.21	0.0	0.0	0.00		
16	13	4049.94	276.25	0.0	0.0	0.00		
17	14	4804.35	276.30	0.0	0.0	0.00		
18	15	5630.60	276.34	0.2	0.0	0.21		
19	16	6515.02	276.39	0.2	0.0	0.21		
20	17	7468.05	276.44	0.2	0.0	0.21		
21	18	8486.97	276.49	0.2	0.0	0.21		
22	19	9568.40	276.55	0.2	0.0	0.21		
23	20	10708.28	276.61	0.2	0.0	0.21		
24	21	11901.96	276.67	0.2	0.0	0.21		
25	22	13144.20	276.73	0.2	0.0	0.21		
26	22	14429.27	276.80	0.2	0.0	0.21		
27	23	15751.00	276.87	0.2	0.0	0.21		
28	23	17102.82	276.93	0.2	0.0	0.21		
29	23	18477.84	277.00	0.2	0.0	0.21		
30	24	19868.95	277.07	0.2	0.0	0.21		
31	24	21268.85	277.14	0.2	0.0	0.21		
32	23	22670.15	277.20	0.2	0.0	0.21		
33	23	24065.47	277.27	0.2	0.0	0.21		
34	23	25447.45	277.34	0.2	0.0	0.21		
35	22	26808.91	277.40	0.2	0.0	0.21		
36	22	28142.87	277.46	0.2	0.0	0.21		
37	21	29442.63	277.52	0.2	0.0	0.21		
38	20	30701.86	277.58	0.2	0.0	0.21		
39	20	31914.64	277.64	0.2	0.0	0.21		
40	19	33078.49	277.69	0.2	0.0	0.21		





# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.94	ft	
Peak Stage =	280.06	ft	
Rise =	4.06	ft	
Peak Outflow =	1.05	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 282.00 ft	
Qp = 29.84 cfs	N = 1		
Tp = 40.5 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 280.00 ft	Number = 1 Ea	
Ks = 18482		Diameter = 4.00 Inches	
b = 1.10		Head = 3.01 inches	
Z <sub>0</sub> = 276.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 276.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	276.00	0.0	0.0	0.0		
1	0	0.00	276.00	0.0	0.0	0.00		
2	0	2.69	276.00	0.0	0.0	0.00		
3	0	13.42	276.00	0.0	0.0	0.00		
4	1	37.51	276.00	0.0	0.0	0.00		
5	1	80.19	276.01	0.0	0.0	0.00		
6	2	146.57	276.01	0.0	0.0	0.00		
7	2	241.63	276.02	0.0	0.0	0.00		
8	3	370.18	276.03	0.0	0.0	0.00		
9	3	536.81	276.04	0.0	0.0	0.00		
10	4	745.91	276.05	0.0	0.0	0.00		
11	5	1001.59	276.07	0.0	0.0	0.00		
12	6	1307.69	276.09	0.0	0.0	0.00		
13	7	1667.75	276.11	0.0	0.0	0.00		
14	8	2084.98	276.14	0.0	0.0	0.00		
15	9	2562.25	276.17	0.0	0.0	0.00		
16	10	3102.07	276.20	0.0	0.0	0.00		
17	11	3706.57	276.23	0.0	0.0	0.00		
18	12	4377.50	276.27	0.0	0.0	0.00		
19	13	5116.20	276.31	0.0	0.0	0.00		
20	15	5923.61	276.36	0.2	0.0	0.21		
21	16	6787.85	276.40	0.2	0.0	0.21		
22	17	7721.43	276.45	0.2	0.0	0.21		
23	18	8724.06	276.51	0.2	0.0	0.21		
24	19	9795.01	276.56	0.2	0.0	0.21		
25	20	10933.15	276.62	0.2	0.0	0.21		
26	21	12136.96	276.68	0.2	0.0	0.21		
27	22	13404.50	276.75	0.2	0.0	0.21		
28	23	14733.46	276.81	0.2	0.0	0.21		
29	24	16121.16	276.88	0.2	0.0	0.21		
30	25	17564.57	276.95	0.2	0.0	0.21		
31	26	19060.32	277.03	0.2	0.0	0.21		
32	27	20604.73	277.10	0.2	0.0	0.21		
33	27	22193.83	277.18	0.2	0.0	0.21		
34	28	23823.38	277.26	0.2	0.0	0.21		
35	28	25488.88	277.34	0.2	0.0	0.21		
36	29	27185.65	277.42	0.2	0.0	0.21		
37	29	28908.79	277.50	0.2	0.0	0.21		
38	30	30653.26	277.58	0.2	0.0	0.21		
39	30	32413.88	277.66	0.2	0.0	0.21		
40	30	34185.38	277.75	0.2	0.0	0.21		
41	30	35962.42	277.83	0.2	0.0	0.21		
42	30	37739.63	277.91	0.2	0.0	0.21		
43	30	39511.65	277.99	0.2	0.0	0.21		
44	29	41273.13	278.07	0.2	0.0	0.21		
45	29	43018.79	278.15	0.2	0.0	0.21		
46	29	44743.45	278.23	0.2	0.0	0.21		

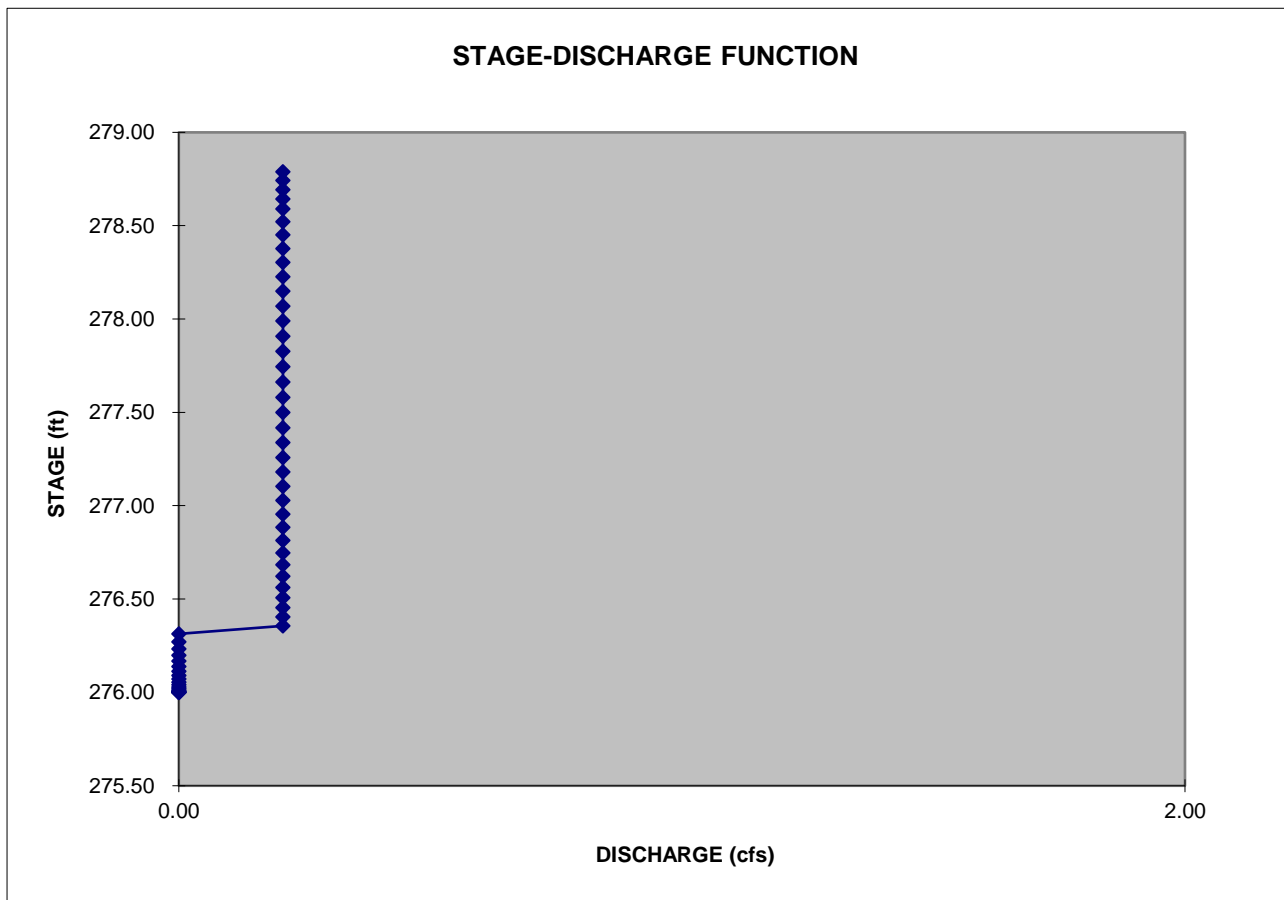
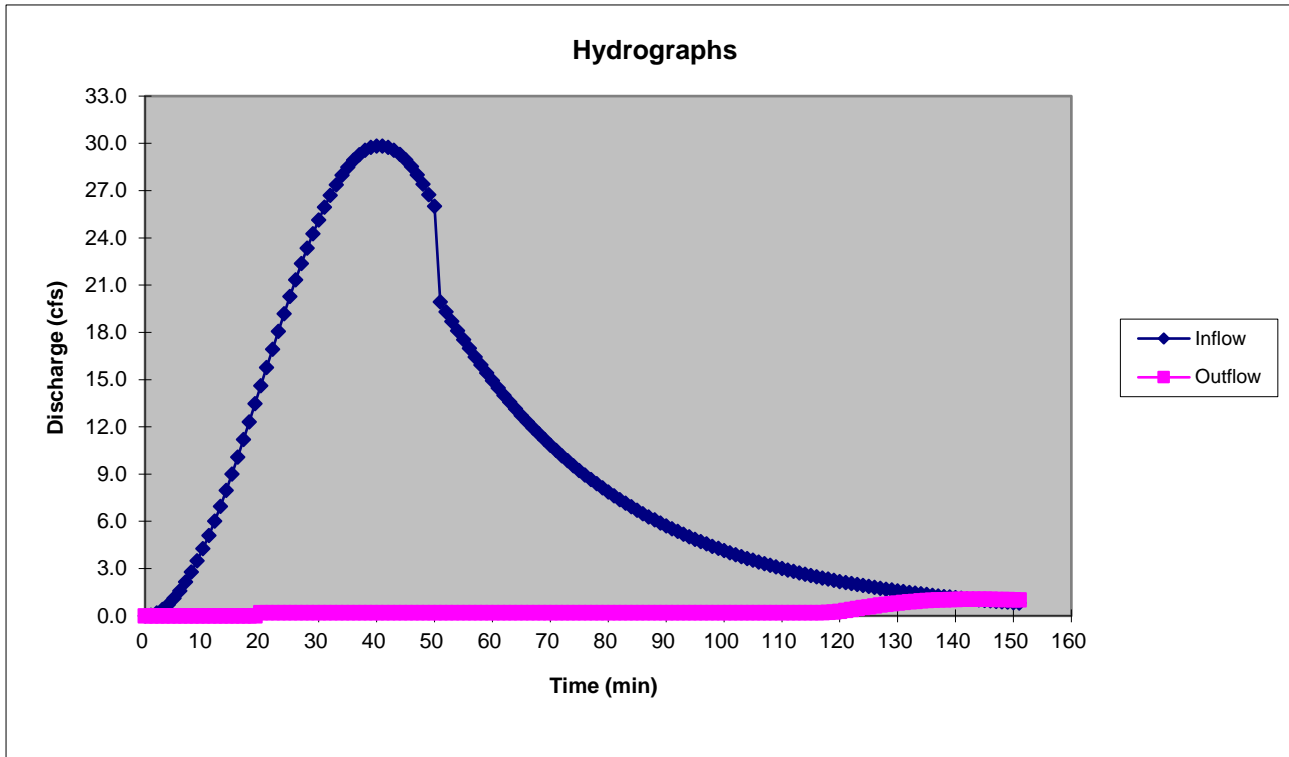
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	28	46442.06	278.30	0.2	0.0	0.21		
48	27	48109.72	278.38	0.2	0.0	0.21		
49	27	49741.72	278.45	0.2	0.0	0.21		
50	26	51333.55	278.52	0.2	0.0	0.21		
51	20	52880.95	278.59	0.2	0.0	0.21		
52	19	54064.09	278.64	0.2	0.0	0.21		
53	19	55209.50	278.69	0.2	0.0	0.21		
54	18	56318.35	278.74	0.2	0.0	0.21		
55	18	57391.82	278.79	0.2	0.0	0.21		
56	17	58431.01	278.84	0.2	0.0	0.21		
57	16	59437.00	278.88	0.2	0.0	0.21		
58	16	60410.85	278.92	0.2	0.0	0.21		
59	15	61353.57	278.96	0.2	0.0	0.21		
60	15	62266.14	279.00	0.2	0.0	0.21		
61	14	63149.51	279.04	0.2	0.0	0.21		
62	14	64004.61	279.08	0.2	0.0	0.21		
63	14	64832.32	279.11	0.2	0.0	0.21		
64	13	65633.52	279.15	0.2	0.0	0.21		
65	13	66409.04	279.18	0.2	0.0	0.21		
66	12	67159.68	279.22	0.2	0.0	0.21		
67	12	67886.24	279.25	0.2	0.0	0.21		
68	12	68589.47	279.28	0.2	0.0	0.21		
69	11	69270.12	279.31	0.2	0.0	0.21		
70	11	69928.88	279.34	0.2	0.0	0.21		
71	10	70566.47	279.36	0.2	0.0	0.21		
72	10	71183.53	279.39	0.2	0.0	0.21		
73	10	71780.73	279.42	0.2	0.0	0.21		
74	10	72358.68	279.44	0.2	0.0	0.21		
75	9	72918.00	279.46	0.2	0.0	0.21		
76	9	73459.27	279.49	0.2	0.0	0.21		
77	9	73983.07	279.51	0.2	0.0	0.21		
78	8	74489.93	279.53	0.2	0.0	0.21		
79	8	74980.41	279.55	0.2	0.0	0.21		
80	8	75455.02	279.57	0.2	0.0	0.21		
81	8	75914.25	279.59	0.2	0.0	0.21		
82	7	76358.59	279.61	0.2	0.0	0.21		
83	7	76788.52	279.63	0.2	0.0	0.21		
84	7	77204.49	279.65	0.2	0.0	0.21		
85	7	77606.93	279.67	0.2	0.0	0.21		
86	6	77996.28	279.68	0.2	0.0	0.21		
87	6	78372.94	279.70	0.2	0.0	0.21		
88	6	78737.33	279.71	0.2	0.0	0.21		
89	6	79089.82	279.73	0.2	0.0	0.21		
90	6	79430.79	279.74	0.2	0.0	0.21		
91	6	79760.61	279.76	0.2	0.0	0.21		
92	5	80079.63	279.77	0.2	0.0	0.21		
93	5	80388.18	279.78	0.2	0.0	0.21		
94	5	80686.60	279.80	0.2	0.0	0.21		
95	5	80975.21	279.81	0.2	0.0	0.21		
96	5	81254.32	279.82	0.2	0.0	0.21		
97	5	81524.23	279.83	0.2	0.0	0.21		
98	4	81785.22	279.84	0.2	0.0	0.21		
99	4	82037.59	279.85	0.2	0.0	0.21		
100	4	82281.60	279.86	0.2	0.0	0.21		
101	4	82517.51	279.87	0.2	0.0	0.21		
102	4	82745.59	279.88	0.2	0.0	0.21		
103	4	82966.07	279.89	0.2	0.0	0.21		
104	4	83179.20	279.90	0.2	0.0	0.21		
105	4	83385.21	279.91	0.2	0.0	0.21		
106	3	83584.33	279.92	0.2	0.0	0.21		
107	3	83776.77	279.93	0.2	0.0	0.21		
108	3	83962.75	279.94	0.2	0.0	0.21		
109	3	84142.46	279.94	0.2	0.0	0.21		
110	3	84316.11	279.95	0.2	0.0	0.21		
111	3	84483.88	279.96	0.2	0.0	0.21		
112	3	84645.97	279.97	0.2	0.0	0.21		
113	3	84802.55	279.97	0.2	0.0	0.21		
114	3	84953.79	279.98	0.2	0.0	0.21		
115	3	85099.87	279.98	0.2	0.0	0.21		
116	2	85240.94	279.99	0.2	0.0	0.21		
117	2	85377.17	280.00	0.2	0.0	0.21		
118	2	85508.71	280.00	0.2	0.0	0.21		
119	2	85635.40	280.01	0.2	0.0	0.21		
120	2	85755.74	280.01	0.3	0.1	0.21		
121	2	85869.10	280.02	0.3	0.1	0.21		
122	2	85975.17	280.02	0.4	0.2	0.21		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	2	86073.87	280.03	0.5	0.2	0.21		
124	2	86165.22	280.03	0.5	0.3	0.21		
125	2	86249.34	280.03	0.6	0.4	0.21		
126	2	86326.41	280.04	0.6	0.4	0.21		
127	2	86396.67	280.04	0.7	0.5	0.21		
128	2	86460.40	280.04	0.7	0.5	0.21		
129	2	86517.88	280.04	0.8	0.6	0.21		
130	2	86569.43	280.05	0.8	0.6	0.21		
131	2	86615.37	280.05	0.9	0.6	0.21		
132	1	86656.01	280.05	0.9	0.7	0.21		
133	1	86691.69	280.05	0.9	0.7	0.21		
134	1	86722.71	280.05	0.9	0.7	0.21		
135	1	86749.39	280.05	1.0	0.8	0.21		
136	1	86772.01	280.06	1.0	0.8	0.21		
137	1	86790.87	280.06	1.0	0.8	0.21		
138	1	86806.25	280.06	1.0	0.8	0.21		
139	1	86818.39	280.06	1.0	0.8	0.21		
140	1	86827.56	280.06	1.0	0.8	0.21		
141	1	86833.98	280.06	1.0	0.8	0.21		
142	1	86837.87	280.06	1.0	0.8	0.21		
143	1	86839.45	280.06	1.1	0.8	0.21		
144	1	86838.90	280.06	1.1	0.8	0.21		
145	1	86836.40	280.06	1.0	0.8	0.21		
146	1	86832.14	280.06	1.0	0.8	0.21		
147	1	86826.25	280.06	1.0	0.8	0.21		
148	1	86818.90	280.06	1.0	0.8	0.21		
149	1	86810.21	280.06	1.0	0.8	0.21		
150	1	86800.32	280.06	1.0	0.8	0.21		
151	1	86789.34	280.06	1.0	0.8	0.21		

# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.86	ft	
Peak Stage =	280.14	ft	
Rise =	4.14	ft	
Peak Outflow =	3.50	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 282.00 ft	
Qp = 34.27 cfs	N = 1		
Tp = 40.5 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 280.00 ft	Number = 1.00 Ea	
Ks = 18482		Diameter = 4.00 Inches	
b = 1.10		Head = 3.01 inches	
Z <sub>0</sub> = 276.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 276.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	276.00	0.0	0.0	0.0		
1	0	0.00	276.00	0.0	0.0	0.00		
2	0	3.09	276.00	0.0	0.0	0.00		
3	0	15.42	276.00	0.0	0.0	0.00		
4	1	43.09	276.00	0.0	0.0	0.00		
5	1	92.11	276.01	0.0	0.0	0.00		
6	2	168.36	276.01	0.0	0.0	0.00		
7	2	277.55	276.02	0.0	0.0	0.00		
8	3	425.21	276.03	0.0	0.0	0.00		
9	4	616.62	276.05	0.0	0.0	0.00		
10	5	856.81	276.06	0.0	0.0	0.00		
11	6	1150.50	276.08	0.0	0.0	0.00		
12	7	1502.11	276.10	0.0	0.0	0.00		
13	8	1915.70	276.13	0.0	0.0	0.00		
14	9	2394.96	276.16	0.0	0.0	0.00		
15	10	2943.19	276.19	0.0	0.0	0.00		
16	12	3563.26	276.23	0.0	0.0	0.00		
17	13	4257.64	276.26	0.0	0.0	0.00		
18	14	5028.32	276.31	0.0	0.0	0.00		
19	15	5876.84	276.35	0.2	0.0	0.21		
20	17	6791.88	276.40	0.2	0.0	0.21		
21	18	7786.44	276.46	0.2	0.0	0.21		
22	19	8860.67	276.51	0.2	0.0	0.21		
23	21	10014.21	276.57	0.2	0.0	0.21		
24	22	11246.23	276.64	0.2	0.0	0.21		
25	23	12555.44	276.70	0.2	0.0	0.21		
26	25	13940.06	276.77	0.2	0.0	0.21		
27	26	15397.90	276.85	0.2	0.0	0.21		
28	27	16926.28	276.92	0.2	0.0	0.21		
29	28	18522.15	277.00	0.2	0.0	0.21		
30	29	20182.00	277.08	0.2	0.0	0.21		
31	30	21901.98	277.17	0.2	0.0	0.21		
32	31	23677.85	277.25	0.2	0.0	0.21		
33	31	25505.05	277.34	0.2	0.0	0.21		
34	32	27378.72	277.43	0.2	0.0	0.21		
35	33	29293.69	277.52	0.2	0.0	0.21		
36	33	31244.57	277.61	0.2	0.0	0.21		
37	34	33225.74	277.70	0.2	0.0	0.21		
38	34	35231.41	277.79	0.2	0.0	0.21		
39	34	37255.63	277.89	0.2	0.0	0.21		
40	34	39292.35	277.98	0.2	0.0	0.21		
41	34	41335.44	278.07	0.2	0.0	0.21		
42	34	43378.73	278.16	0.2	0.0	0.21		
43	34	45416.04	278.26	0.2	0.0	0.21		
44	34	47441.25	278.35	0.2	0.0	0.21		
45	33	49448.29	278.44	0.2	0.0	0.21		
46	33	51431.21	278.53	0.2	0.0	0.21		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	32	53384.21	278.61	0.2	0.0	0.21		
48	31	55301.66	278.70	0.2	0.0	0.21		
49	31	57178.13	278.78	0.2	0.0	0.21		
50	30	59008.47	278.86	0.2	0.0	0.21		
51	20	60787.78	278.94	0.2	0.0	0.21		
52	19	61970.92	278.99	0.2	0.0	0.21		
53	19	63116.32	279.04	0.2	0.0	0.21		
54	18	64225.18	279.09	0.2	0.0	0.21		
55	18	65298.64	279.14	0.2	0.0	0.21		
56	17	66337.83	279.18	0.2	0.0	0.21		
57	16	67343.83	279.22	0.2	0.0	0.21		
58	16	68317.68	279.27	0.2	0.0	0.21		
59	15	69260.39	279.31	0.2	0.0	0.21		
60	15	70172.96	279.35	0.2	0.0	0.21		
61	14	71056.34	279.38	0.2	0.0	0.21		
62	14	71911.44	279.42	0.2	0.0	0.21		
63	14	72739.15	279.46	0.2	0.0	0.21		
64	13	73540.35	279.49	0.2	0.0	0.21		
65	13	74315.86	279.52	0.2	0.0	0.21		
66	12	75066.51	279.56	0.2	0.0	0.21		
67	12	75793.07	279.59	0.2	0.0	0.21		
68	12	76496.30	279.62	0.2	0.0	0.21		
69	11	77176.94	279.65	0.2	0.0	0.21		
70	11	77835.71	279.68	0.2	0.0	0.21		
71	10	78473.29	279.70	0.2	0.0	0.21		
72	10	79090.36	279.73	0.2	0.0	0.21		
73	10	79687.55	279.75	0.2	0.0	0.21		
74	10	80265.51	279.78	0.2	0.0	0.21		
75	9	80824.83	279.80	0.2	0.0	0.21		
76	9	81366.10	279.83	0.2	0.0	0.21		
77	9	81889.89	279.85	0.2	0.0	0.21		
78	8	82396.76	279.87	0.2	0.0	0.21		
79	8	82887.24	279.89	0.2	0.0	0.21		
80	8	83361.84	279.91	0.2	0.0	0.21		
81	8	83821.08	279.93	0.2	0.0	0.21		
82	7	84265.42	279.95	0.2	0.0	0.21		
83	7	84695.35	279.97	0.2	0.0	0.21		
84	7	85111.31	279.99	0.2	0.0	0.21		
85	7	85513.76	280.00	0.2	0.0	0.21		
86	6	85902.74	280.02	0.4	0.2	0.21		
87	6	86270.25	280.03	0.6	0.4	0.21		
88	6	86611.87	280.05	0.9	0.6	0.21		
89	6	86925.73	280.06	1.1	0.9	0.21		
90	6	87211.21	280.07	1.4	1.2	0.21		
91	6	87468.54	280.08	1.7	1.5	0.21		
92	5	87698.51	280.09	2.0	1.7	0.21		
93	5	87902.30	280.10	2.2	2.0	0.21		
94	5	88081.32	280.11	2.4	2.2	0.21		
95	5	88237.17	280.12	2.6	2.4	0.21		
96	5	88371.51	280.12	2.8	2.6	0.21		
97	5	88486.03	280.13	3.0	2.7	0.21		
98	4	88582.38	280.13	3.1	2.9	0.21		
99	4	88662.18	280.14	3.2	3.0	0.21		
100	4	88726.97	280.14	3.3	3.1	0.21		
101	4	88778.20	280.14	3.4	3.2	0.21		
102	4	88817.24	280.14	3.4	3.2	0.21		
103	4	88845.35	280.14	3.5	3.2	0.21		
104	4	88863.68	280.14	3.5	3.3	0.21		
105	4	88873.31	280.14	3.5	3.3	0.21		
106	3	88875.22	280.14	3.5	3.3	0.21		
107	3	88870.28	280.14	3.5	3.3	0.21		
108	3	88859.31	280.14	3.5	3.3	0.21		
109	3	88843.02	280.14	3.5	3.2	0.21		
110	3	88822.07	280.14	3.4	3.2	0.21		
111	3	88797.05	280.14	3.4	3.2	0.21		
112	3	88768.49	280.14	3.3	3.1	0.21		
113	3	88736.86	280.14	3.3	3.1	0.21		
114	3	88702.59	280.14	3.3	3.0	0.21		
115	3	88666.05	280.14	3.2	3.0	0.21		
116	2	88627.58	280.13	3.1	2.9	0.21		
117	2	88587.49	280.13	3.1	2.9	0.21		
118	2	88546.04	280.13	3.0	2.8	0.21		
119	2	88503.47	280.13	3.0	2.8	0.21		
120	2	88459.99	280.13	2.9	2.7	0.21		
121	2	88415.80	280.12	2.9	2.6	0.21		
122	2	88371.05	280.12	2.8	2.6	0.21		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	2	88325.90	280.12	2.7	2.5	0.21		
124	2	88280.48	280.12	2.7	2.5	0.21		
125	2	88234.90	280.12	2.6	2.4	0.21		
126	2	88189.27	280.12	2.6	2.4	0.21		
127	2	88143.67	280.11	2.5	2.3	0.21		
128	2	88098.19	280.11	2.4	2.2	0.21		
129	2	88052.89	280.11	2.4	2.2	0.21		
130	2	88007.83	280.11	2.3	2.1	0.21		
131	2	87963.06	280.11	2.3	2.1	0.21		
132	1	87918.64	280.10	2.2	2.0	0.21		
133	1	87874.61	280.10	2.2	2.0	0.21		
134	1	87830.99	280.10	2.1	1.9	0.21		
135	1	87787.81	280.10	2.1	1.9	0.21		
136	1	87745.11	280.10	2.0	1.8	0.21		
137	1	87702.89	280.09	2.0	1.8	0.21		
138	1	87661.19	280.09	1.9	1.7	0.21		
139	1	87620.01	280.09	1.9	1.7	0.21		
140	1	87579.37	280.09	1.8	1.6	0.21		
141	1	87539.28	280.09	1.8	1.6	0.21		
142	1	87499.74	280.09	1.7	1.5	0.21		
143	1	87460.75	280.08	1.7	1.5	0.21		
144	1	87422.33	280.08	1.6	1.4	0.21		
145	1	87384.48	280.08	1.6	1.4	0.21		
146	1	87347.19	280.08	1.6	1.4	0.21		
147	1	87310.47	280.08	1.5	1.3	0.21		
148	1	87274.31	280.08	1.5	1.3	0.21		
149	1	87238.71	280.08	1.4	1.2	0.21		
150	1	87203.67	280.07	1.4	1.2	0.21		
151	1	87169.19	280.07	1.4	1.2	0.21		

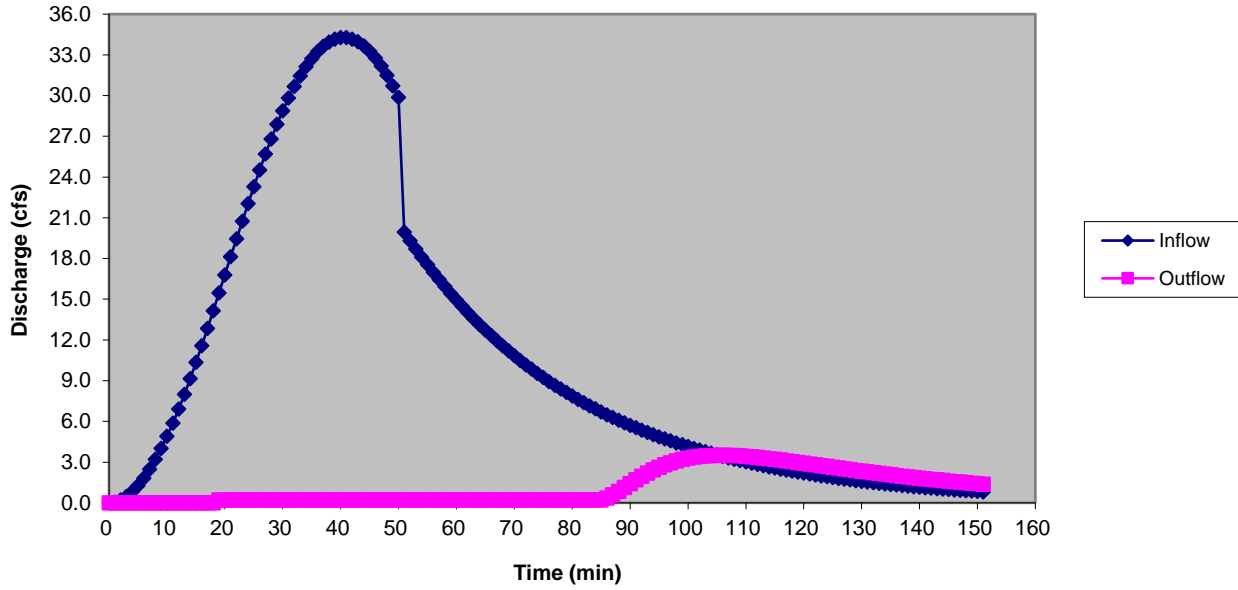




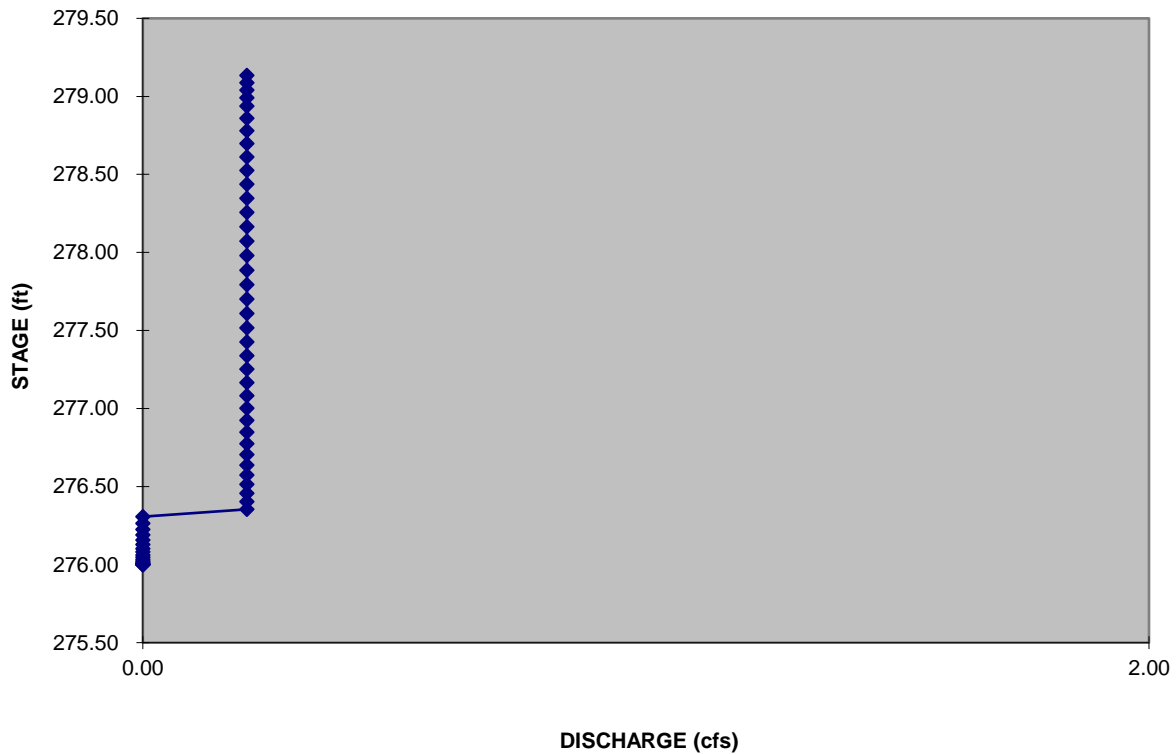
# McADAMS

## GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM-F  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-F

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	8.31 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	4.99 ac.	0.25
Undisturbed grassy area	0.00 ac.	3.32 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>8.31 ac.</b>	<b>8.31 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.29</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	731	feet	
Height of watershed =	22	feet	
Calculated t(c) =	4.8	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	5.0	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	5.74 in/hr
10	195	22	7.22 in/hr
25	232	23	8.29 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.74 in/hr	21.5 cfs	13.83 cfs
10-year storm	7.22 in/hr	27.0 cfs	17.40 cfs
25-year storm	8.29 in/hr	31.0 cfs	19.96 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	304
Bottom width =	58
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	16
Height of berm =	6
Top of trap length =	340
Top of trap width =	94
Bottom elevation =	299

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	29916 cu. ft.	88410 cu. ft.	
Sediment surface area =	11750 sq. ft.	26900 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	5.2L:1W	
Spillway length =	10.0 ft. (min)	16	
10-Year flow depth over spillway =	0.50 ft. (max)	-0.44 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	2.44 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 29,916 cubic feet

24 HOURS	Best Option =	1 - 5 inch Skimmers with a 4.8 inch orifice
2 DAYS	Best Option =	1 - 4 inch Skimmers with a 3.4 inch orifice
3 DAYS	Best Option =	1 - 4 inch Skimmers with a 2.8 inch orifice
4 DAYS	Best Option =	1 - 3 inch Skimmers with a 2.6 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

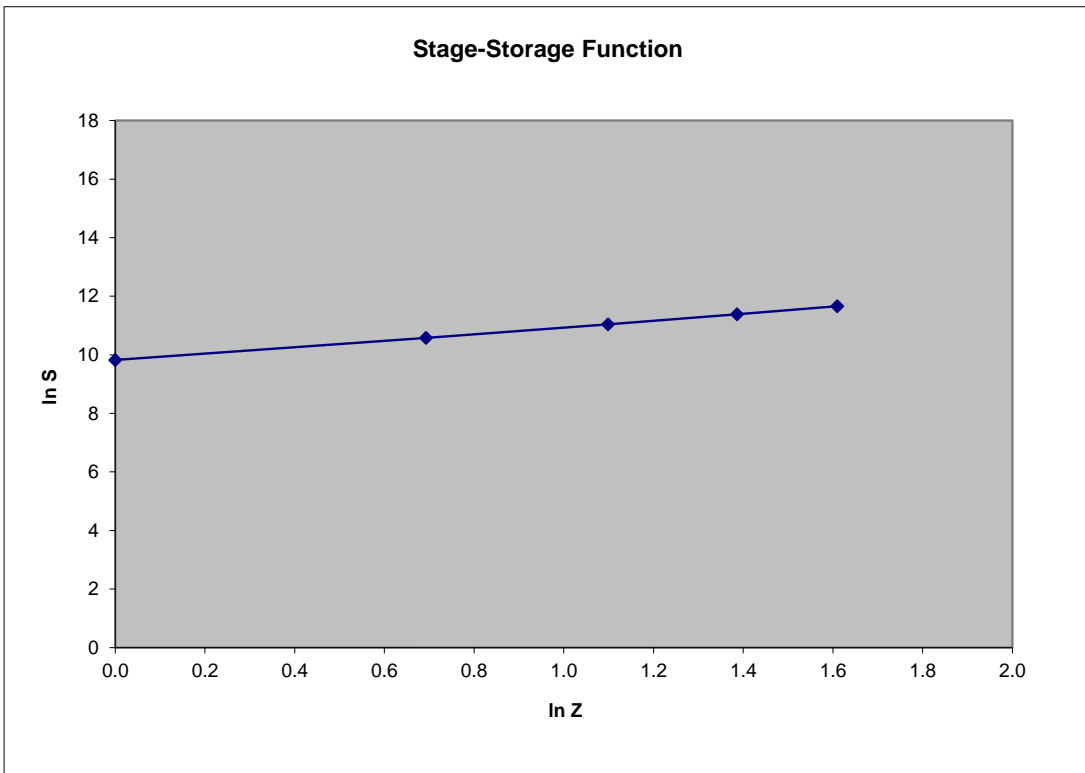
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
299	17190		0	0			
300	19733	18462	18461.5	1	9.8234	0.0000	1.01
301	21982	20857.5	39319	2	10.5795	0.6931	1.97
302	24287	23134.5	62453.5	3	11.0422	1.0986	2.96
303	26650	25468.5	87922	4	11.3842	1.3863	3.99
304	29068	27859	115781	5	11.6595	1.6094	5.09

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.14$  and  $K_s = 18182$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	8.31 ac	<b>Use Kirpich Equation:</b> Tc = 5.0 min
Disturbed Area =	8.31 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.74 in/hr i <sub>10</sub> = 7.22 in/hr i <sub>25</sub> = 8.29 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	731 ft	
Vertical Fall =	22 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 21.5 cfs Q <sub>10</sub> = 27.0 cfs Q <sub>25</sub> = 31.0 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 29.4 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 39.1 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 42.0 minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	301.48	ft
	Rise =	2.48	ft
Freeboard =		3.52	ft
	Peak Outflow =	0.20	cfs

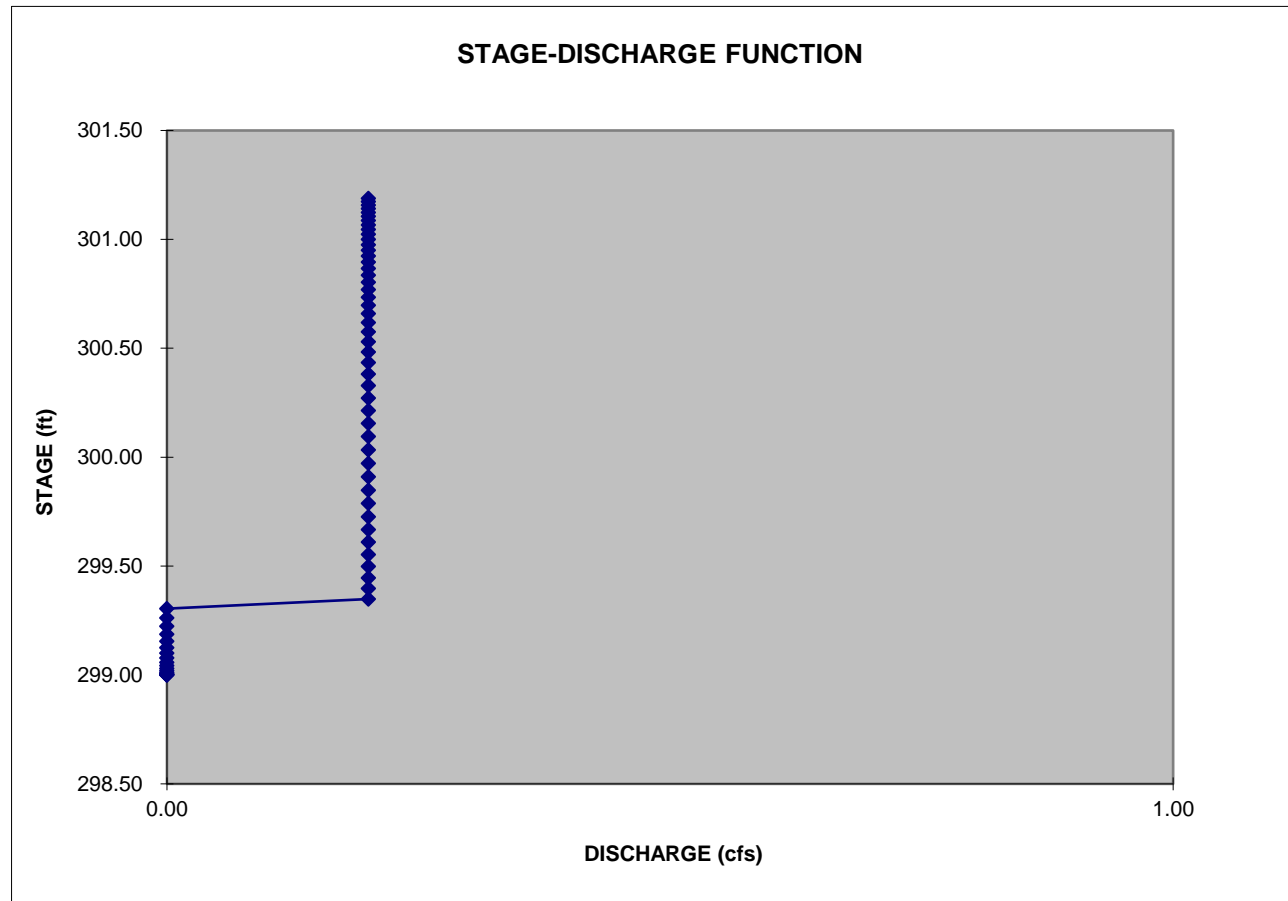
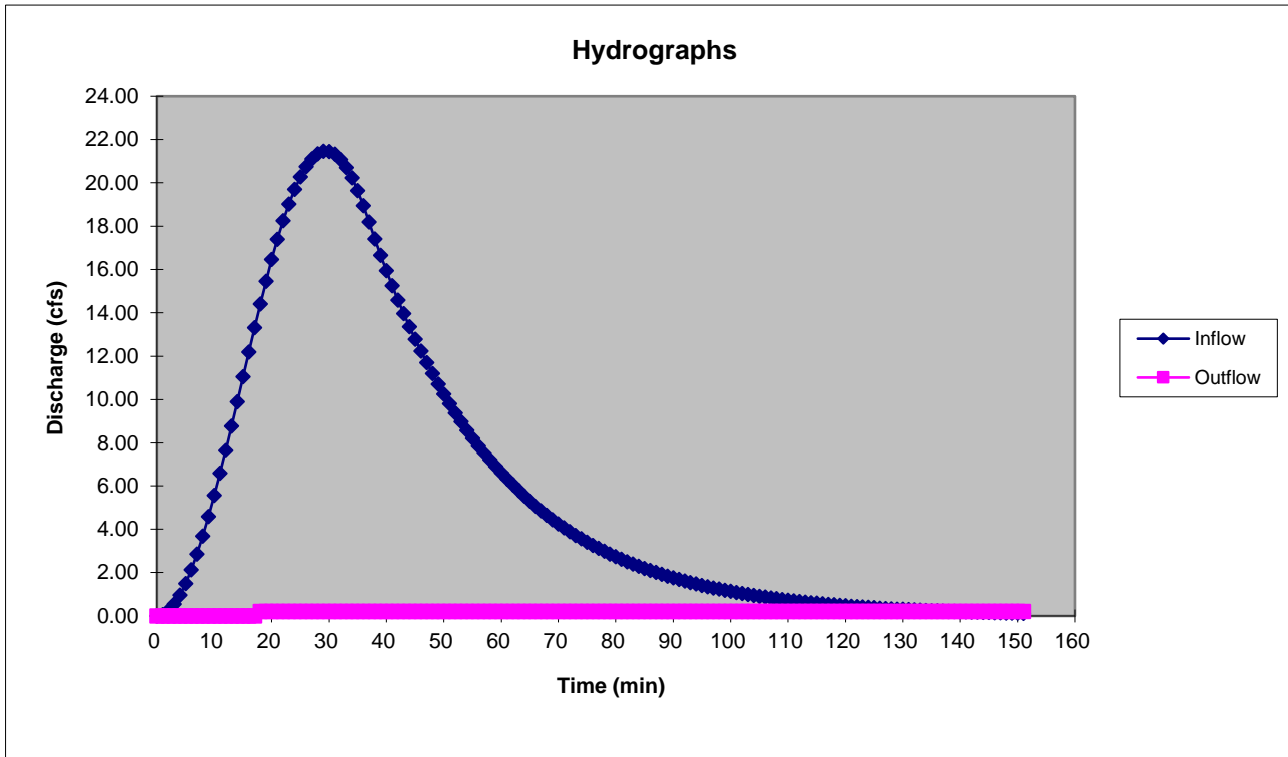
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 305.00 ft
Qp =	21.46	cfs	N =
	29.4	min	L =
dT =	1.0	min	Cw =
<b>Stage-Storage Results:</b>		Zcr =	<b>Skimmer Orifice:</b>
Ks =	18182		Number =
b =	1.14		Diameter =
Z <sub>0</sub> =	299.0	ft (inv)	Head =
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	299.00	ft	

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	299.00	0.0	0.0	0.0		
1	0	0.00	299.00	0.0	0.0	0.00		
2	0	3.66	299.00	0.0	0.0	0.00		
3	1	18.26	299.00	0.0	0.0	0.00		
4	1	50.95	299.01	0.0	0.0	0.00		
5	1	108.68	299.01	0.0	0.0	0.00		
6	2	198.11	299.02	0.0	0.0	0.00		
7	3	325.56	299.03	0.0	0.0	0.00		
8	4	496.88	299.04	0.0	0.0	0.00		
9	5	717.45	299.06	0.0	0.0	0.00		
10	6	992.08	299.08	0.0	0.0	0.00		
11	7	1324.97	299.10	0.0	0.0	0.00		
12	8	1719.66	299.13	0.0	0.0	0.00		
13	9	2178.97	299.15	0.0	0.0	0.00		
14	10	2705.02	299.19	0.0	0.0	0.00		
15	11	3299.12	299.22	0.0	0.0	0.00		
16	12	3961.86	299.26	0.0	0.0	0.00		
17	13	4693.01	299.30	0.0	0.0	0.00		
18	14	5491.59	299.35	0.2	0.0	0.20		
19	15	6343.82	299.40	0.2	0.0	0.20		
20	16	7259.21	299.45	0.2	0.0	0.20		
21	17	8234.53	299.50	0.2	0.0	0.20		
22	18	9265.89	299.55	0.2	0.0	0.20		
23	19	10348.73	299.61	0.2	0.0	0.20		
24	20	11477.93	299.67	0.2	0.0	0.20		
25	20	12647.83	299.73	0.2	0.0	0.20		
26	21	13852.33	299.79	0.2	0.0	0.20		
27	21	15084.90	299.85	0.2	0.0	0.20		
28	21	16338.71	299.91	0.2	0.0	0.20		
29	21	17606.71	299.97	0.2	0.0	0.20		
30	21	18881.65	300.03	0.2	0.0	0.20		
31	21	20156.22	300.09	0.2	0.0	0.20		
32	21	21423.11	300.16	0.2	0.0	0.20		
33	21	22675.11	300.21	0.2	0.0	0.20		
34	20	23905.17	300.27	0.2	0.0	0.20		
35	20	25106.47	300.33	0.2	0.0	0.20		
36	19	26272.56	300.38	0.2	0.0	0.20		
37	18	27397.35	300.43	0.2	0.0	0.20		
38	17	28476.71	300.48	0.2	0.0	0.20		
39	17	29508.94	300.53	0.2	0.0	0.20		
40	16	30496.08	300.58	0.2	0.0	0.20		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	2.44	ft	
Peak Stage =	302.56	ft	
Rise =	3.56	ft	
Peak Outflow =	0.20	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	305.00 ft
Qp = 27.01 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.1 min	L = 16 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 4.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 303.00 ft	Head = 2.82 inches	
Ks = 18182			
b = 1.14			
Z <sub>0</sub> = 299.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 299.00 ft			

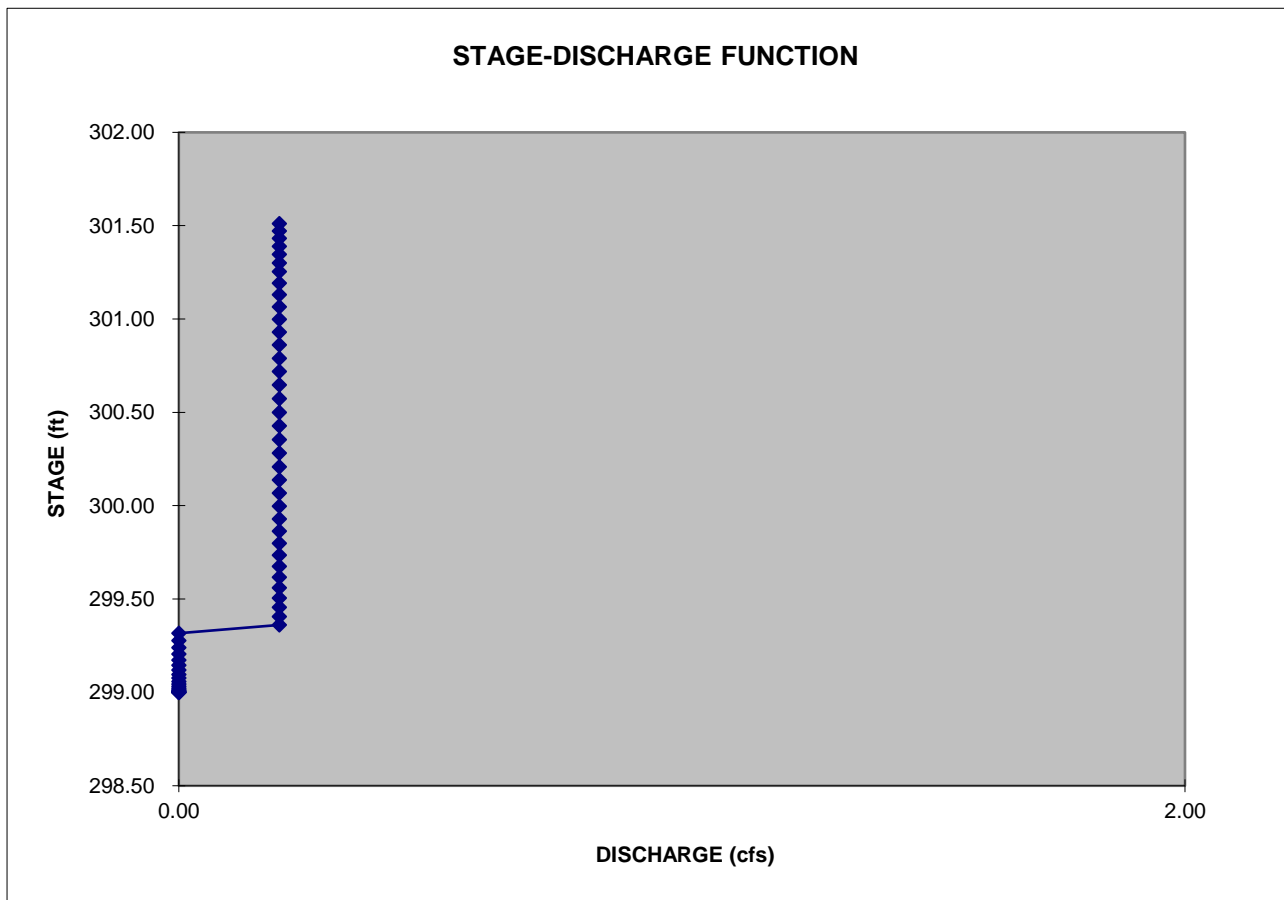
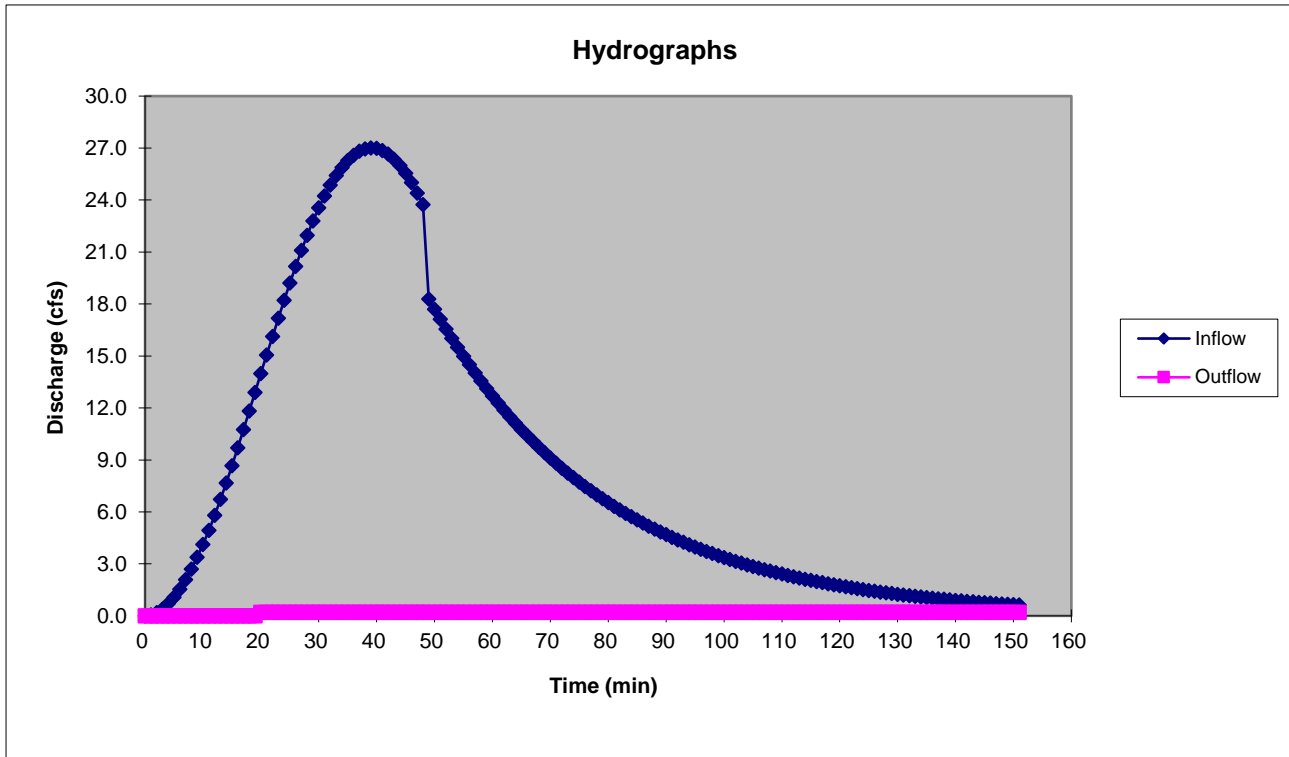
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	299.00	0.0	0.0	0.0		
1	0	0.00	299.00	0.0	0.0	0.00		
2	0	2.61	299.00	0.0	0.0	0.00		
3	0	13.03	299.00	0.0	0.0	0.00		
4	1	36.42	299.00	0.0	0.0	0.00		
5	1	77.85	299.01	0.0	0.0	0.00		
6	2	142.27	299.01	0.0	0.0	0.00		
7	2	234.48	299.02	0.0	0.0	0.00		
8	3	359.11	299.03	0.0	0.0	0.00		
9	3	520.58	299.04	0.0	0.0	0.00		
10	4	723.06	299.06	0.0	0.0	0.00		
11	5	970.48	299.08	0.0	0.0	0.00		
12	6	1266.46	299.10	0.0	0.0	0.00		
13	7	1614.32	299.12	0.0	0.0	0.00		
14	8	2017.02	299.14	0.0	0.0	0.00		
15	9	2477.20	299.17	0.0	0.0	0.00		
16	10	2997.12	299.21	0.0	0.0	0.00		
17	11	3578.63	299.24	0.0	0.0	0.00		
18	12	4223.23	299.28	0.0	0.0	0.00		
19	13	4931.97	299.32	0.0	0.0	0.00		
20	14	5705.50	299.36	0.2	0.0	0.20		
21	15	6532.06	299.41	0.2	0.0	0.20		
22	16	7423.48	299.46	0.2	0.0	0.20		
23	17	8379.14	299.51	0.2	0.0	0.20		
24	18	9398.04	299.56	0.2	0.0	0.20		
25	19	10478.75	299.62	0.2	0.0	0.20		
26	20	11619.46	299.67	0.2	0.0	0.20		
27	21	12817.96	299.74	0.2	0.0	0.20		
28	22	14071.67	299.80	0.2	0.0	0.20		
29	23	15377.65	299.86	0.2	0.0	0.20		
30	24	16732.63	299.93	0.2	0.0	0.20		
31	24	18133.03	300.00	0.2	0.0	0.20		
32	25	19574.96	300.07	0.2	0.0	0.20		
33	25	21054.29	300.14	0.2	0.0	0.20		
34	26	22566.61	300.21	0.2	0.0	0.20		
35	26	24107.33	300.28	0.2	0.0	0.20		
36	27	25671.67	300.35	0.2	0.0	0.20		
37	27	27254.69	300.43	0.2	0.0	0.20		
38	27	28851.33	300.50	0.2	0.0	0.20		
39	27	30456.44	300.57	0.2	0.0	0.20		
40	27	32064.84	300.65	0.2	0.0	0.20		
41	27	33671.30	300.72	0.2	0.0	0.20		
42	27	35270.61	300.79	0.2	0.0	0.20		
43	26	36857.61	300.86	0.2	0.0	0.20		
44	26	38427.22	300.93	0.2	0.0	0.20		
45	26	39974.46	301.00	0.2	0.0	0.20		
46	25	41494.52	301.06	0.2	0.0	0.20		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	24	42982.75	301.13	0.2	0.0	0.20		
48	24	44434.69	301.19	0.2	0.0	0.20		
49	18	45846.13	301.25	0.2	0.0	0.20		
50	18	46931.31	301.30	0.2	0.0	0.20		
51	17	47980.63	301.35	0.2	0.0	0.20		
52	17	48995.27	301.39	0.2	0.0	0.20		
53	16	49976.36	301.43	0.2	0.0	0.20		
54	15	50925.00	301.47	0.2	0.0	0.20		
55	15	51842.24	301.51	0.2	0.0	0.20		
56	14	52729.12	301.55	0.2	0.0	0.20		
57	14	53586.63	301.59	0.2	0.0	0.20		
58	14	54415.72	301.62	0.2	0.0	0.20		
59	13	55217.32	301.65	0.2	0.0	0.20		
60	13	55992.34	301.69	0.2	0.0	0.20		
61	12	56741.64	301.72	0.2	0.0	0.20		
62	12	57466.06	301.75	0.2	0.0	0.20		
63	11	58166.42	301.78	0.2	0.0	0.20		
64	11	58843.50	301.81	0.2	0.0	0.20		
65	11	59498.06	301.83	0.2	0.0	0.20		
66	10	60130.83	301.86	0.2	0.0	0.20		
67	10	60742.54	301.89	0.2	0.0	0.20		
68	10	61333.87	301.91	0.2	0.0	0.20		
69	9	61905.47	301.93	0.2	0.0	0.20		
70	9	62458.01	301.96	0.2	0.0	0.20		
71	9	62992.10	301.98	0.2	0.0	0.20		
72	9	63508.35	302.00	0.2	0.0	0.20		
73	8	64007.33	302.02	0.2	0.0	0.20		
74	8	64489.61	302.04	0.2	0.0	0.20		
75	8	64955.74	302.06	0.2	0.0	0.20		
76	7	65406.25	302.08	0.2	0.0	0.20		
77	7	65841.64	302.10	0.2	0.0	0.20		
78	7	66262.41	302.12	0.2	0.0	0.20		
79	7	66669.04	302.13	0.2	0.0	0.20		
80	7	67061.99	302.15	0.2	0.0	0.20		
81	6	67441.70	302.16	0.2	0.0	0.20		
82	6	67808.62	302.18	0.2	0.0	0.20		
83	6	68163.15	302.19	0.2	0.0	0.20		
84	6	68505.70	302.21	0.2	0.0	0.20		
85	6	68836.67	302.22	0.2	0.0	0.20		
86	5	69156.43	302.23	0.2	0.0	0.20		
87	5	69465.35	302.25	0.2	0.0	0.20		
88	5	69763.78	302.26	0.2	0.0	0.20		
89	5	70052.07	302.27	0.2	0.0	0.20		
90	5	70330.54	302.28	0.2	0.0	0.20		
91	5	70599.52	302.29	0.2	0.0	0.20		
92	4	70859.32	302.30	0.2	0.0	0.20		
93	4	71110.23	302.32	0.2	0.0	0.20		
94	4	71352.56	302.33	0.2	0.0	0.20		
95	4	71586.57	302.33	0.2	0.0	0.20		
96	4	71812.55	302.34	0.2	0.0	0.20		
97	4	72030.74	302.35	0.2	0.0	0.20		
98	4	72241.42	302.36	0.2	0.0	0.20		
99	3	72444.81	302.37	0.2	0.0	0.20		
100	3	72641.17	302.38	0.2	0.0	0.20		
101	3	72830.72	302.39	0.2	0.0	0.20		
102	3	73013.68	302.39	0.2	0.0	0.20		
103	3	73190.27	302.40	0.2	0.0	0.20		
104	3	73360.70	302.41	0.2	0.0	0.20		
105	3	73525.17	302.41	0.2	0.0	0.20		
106	3	73683.87	302.42	0.2	0.0	0.20		
107	3	73836.99	302.43	0.2	0.0	0.20		
108	3	73984.71	302.43	0.2	0.0	0.20		
109	2	74127.22	302.44	0.2	0.0	0.20		
110	2	74264.67	302.44	0.2	0.0	0.20		
111	2	74397.25	302.45	0.2	0.0	0.20		
112	2	74525.09	302.45	0.2	0.0	0.20		
113	2	74648.37	302.46	0.2	0.0	0.20		
114	2	74767.23	302.46	0.2	0.0	0.20		
115	2	74881.80	302.47	0.2	0.0	0.20		
116	2	74992.25	302.47	0.2	0.0	0.20		
117	2	75098.69	302.48	0.2	0.0	0.20		
118	2	75201.26	302.48	0.2	0.0	0.20		
119	2	75300.08	302.49	0.2	0.0	0.20		
120	2	75395.29	302.49	0.2	0.0	0.20		
121	2	75486.99	302.49	0.2	0.0	0.20		
122	2	75575.30	302.50	0.2	0.0	0.20		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	2	75660.33	302.50	0.2	0.0	0.20		
124	2	75742.20	302.50	0.2	0.0	0.20		
125	1	75820.99	302.51	0.2	0.0	0.20		
126	1	75896.82	302.51	0.2	0.0	0.20		
127	1	75969.77	302.51	0.2	0.0	0.20		
128	1	76039.96	302.52	0.2	0.0	0.20		
129	1	76107.45	302.52	0.2	0.0	0.20		
130	1	76172.35	302.52	0.2	0.0	0.20		
131	1	76234.73	302.52	0.2	0.0	0.20		
132	1	76294.68	302.53	0.2	0.0	0.20		
133	1	76352.28	302.53	0.2	0.0	0.20		
134	1	76407.61	302.53	0.2	0.0	0.20		
135	1	76460.73	302.53	0.2	0.0	0.20		
136	1	76511.73	302.54	0.2	0.0	0.20		
137	1	76560.67	302.54	0.2	0.0	0.20		
138	1	76607.62	302.54	0.2	0.0	0.20		
139	1	76652.64	302.54	0.2	0.0	0.20		
140	1	76695.79	302.54	0.2	0.0	0.20		
141	1	76737.15	302.54	0.2	0.0	0.20		
142	1	76776.76	302.55	0.2	0.0	0.20		
143	1	76814.68	302.55	0.2	0.0	0.20		
144	1	76850.97	302.55	0.2	0.0	0.20		
145	1	76885.68	302.55	0.2	0.0	0.20		
146	1	76918.87	302.55	0.2	0.0	0.20		
147	1	76950.58	302.55	0.2	0.0	0.20		
148	1	76980.86	302.55	0.2	0.0	0.20		
149	1	77009.76	302.56	0.2	0.0	0.20		
150	1	77037.32	302.56	0.2	0.0	0.20		
151	1	77063.59	<b>302.56</b>	0.2	0.0	0.20		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	2.17	ft	
Peak Stage =	302.83	ft	
Rise =	3.83	ft	
Peak Outflow =	0.20	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	305.00 ft
Qp = 30.98 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.1 min	L = 16 ft	Number = 1.00 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 4.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 303.00 ft	Head = 2.82 inches	
Ks = 18182			
b = 1.14			
Z <sub>0</sub> = 299.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 299.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	299.00	0.0	0.0	0.0		
1	0	0.00	299.00	0.0	0.0	0.00		
2	0	2.99	299.00	0.0	0.0	0.00		
3	0	14.95	299.00	0.0	0.0	0.00		
4	1	41.79	299.00	0.0	0.0	0.00		
5	1	89.31	299.01	0.0	0.0	0.00		
6	2	163.21	299.02	0.0	0.0	0.00		
7	2	269.00	299.02	0.0	0.0	0.00		
8	3	411.99	299.04	0.0	0.0	0.00		
9	4	597.23	299.05	0.0	0.0	0.00		
10	5	829.54	299.07	0.0	0.0	0.00		
11	6	1113.39	299.09	0.0	0.0	0.00		
12	7	1452.95	299.11	0.0	0.0	0.00		
13	8	1852.03	299.13	0.0	0.0	0.00		
14	9	2314.03	299.16	0.0	0.0	0.00		
15	10	2841.98	299.20	0.0	0.0	0.00		
16	11	3438.45	299.23	0.0	0.0	0.00		
17	12	4105.60	299.27	0.0	0.0	0.00		
18	14	4845.11	299.31	0.0	0.0	0.00		
19	15	5658.21	299.36	0.2	0.0	0.20		
20	16	6533.64	299.41	0.2	0.0	0.20		
21	17	7483.69	299.46	0.2	0.0	0.20		
22	19	8508.13	299.51	0.2	0.0	0.20		
23	20	9606.29	299.57	0.2	0.0	0.20		
24	21	10776.99	299.63	0.2	0.0	0.20		
25	22	12018.61	299.70	0.2	0.0	0.20		
26	23	13329.06	299.76	0.2	0.0	0.20		
27	24	14705.81	299.83	0.2	0.0	0.20		
28	25	16145.90	299.90	0.2	0.0	0.20		
29	26	17645.95	299.97	0.2	0.0	0.20		
30	27	19202.23	300.05	0.2	0.0	0.20		
31	28	20810.61	300.13	0.2	0.0	0.20		
32	29	22466.64	300.20	0.2	0.0	0.20		
33	29	24165.57	300.28	0.2	0.0	0.20		
34	30	25902.35	300.36	0.2	0.0	0.20		
35	30	27671.72	300.45	0.2	0.0	0.20		
36	30	29468.18	300.53	0.2	0.0	0.20		
37	31	31286.07	300.61	0.2	0.0	0.20		
38	31	33119.58	300.69	0.2	0.0	0.20		
39	31	34962.83	300.78	0.2	0.0	0.20		
40	31	36809.84	300.86	0.2	0.0	0.20		
41	31	38654.62	300.94	0.2	0.0	0.20		
42	31	40491.20	301.02	0.2	0.0	0.20		
43	30	42313.66	301.10	0.2	0.0	0.20		
44	30	44116.16	301.18	0.2	0.0	0.20		
45	29	45893.02	301.26	0.2	0.0	0.20		
46	29	47638.68	301.33	0.2	0.0	0.20		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	28	49347.81	301.40	0.2	0.0	0.20		
48	27	51015.32	301.48	0.2	0.0	0.20		
49	18	52636.37	301.55	0.2	0.0	0.20		
50	18	53721.55	301.59	0.2	0.0	0.20		
51	17	54770.87	301.64	0.2	0.0	0.20		
52	17	55785.51	301.68	0.2	0.0	0.20		
53	16	56766.60	301.72	0.2	0.0	0.20		
54	15	57715.24	301.76	0.2	0.0	0.20		
55	15	58632.49	301.80	0.2	0.0	0.20		
56	14	59519.37	301.84	0.2	0.0	0.20		
57	14	60376.87	301.87	0.2	0.0	0.20		
58	14	61205.96	301.91	0.2	0.0	0.20		
59	13	62007.57	301.94	0.2	0.0	0.20		
60	13	62782.59	301.97	0.2	0.0	0.20		
61	12	63531.89	302.00	0.2	0.0	0.20		
62	12	64256.31	302.03	0.2	0.0	0.20		
63	11	64956.66	302.06	0.2	0.0	0.20		
64	11	65633.74	302.09	0.2	0.0	0.20		
65	11	66288.30	302.12	0.2	0.0	0.20		
66	10	66921.08	302.14	0.2	0.0	0.20		
67	10	67532.78	302.17	0.2	0.0	0.20		
68	10	68124.11	302.19	0.2	0.0	0.20		
69	9	68695.72	302.22	0.2	0.0	0.20		
70	9	69248.26	302.24	0.2	0.0	0.20		
71	9	69782.35	302.26	0.2	0.0	0.20		
72	9	70298.59	302.28	0.2	0.0	0.20		
73	8	70797.57	302.30	0.2	0.0	0.20		
74	8	71279.85	302.32	0.2	0.0	0.20		
75	8	71745.98	302.34	0.2	0.0	0.20		
76	7	72196.49	302.36	0.2	0.0	0.20		
77	7	72631.88	302.38	0.2	0.0	0.20		
78	7	73052.65	302.39	0.2	0.0	0.20		
79	7	73459.28	302.41	0.2	0.0	0.20		
80	7	73852.23	302.43	0.2	0.0	0.20		
81	6	74231.95	302.44	0.2	0.0	0.20		
82	6	74598.86	302.46	0.2	0.0	0.20		
83	6	74953.39	302.47	0.2	0.0	0.20		
84	6	75295.95	302.49	0.2	0.0	0.20		
85	6	75626.91	302.50	0.2	0.0	0.20		
86	5	75946.67	302.51	0.2	0.0	0.20		
87	5	76255.59	302.53	0.2	0.0	0.20		
88	5	76554.02	302.54	0.2	0.0	0.20		
89	5	76842.31	302.55	0.2	0.0	0.20		
90	5	77120.78	302.56	0.2	0.0	0.20		
91	5	77389.76	302.57	0.2	0.0	0.20		
92	4	77649.56	302.58	0.2	0.0	0.20		
93	4	77900.48	302.59	0.2	0.0	0.20		
94	4	78142.80	302.60	0.2	0.0	0.20		
95	4	78376.82	302.61	0.2	0.0	0.20		
96	4	78602.79	302.62	0.2	0.0	0.20		
97	4	78820.99	302.63	0.2	0.0	0.20		
98	4	79031.66	302.64	0.2	0.0	0.20		
99	3	79235.06	302.65	0.2	0.0	0.20		
100	3	79431.42	302.65	0.2	0.0	0.20		
101	3	79620.96	302.66	0.2	0.0	0.20		
102	3	79803.93	302.67	0.2	0.0	0.20		
103	3	79980.52	302.68	0.2	0.0	0.20		
104	3	80150.95	302.68	0.2	0.0	0.20		
105	3	80315.41	302.69	0.2	0.0	0.20		
106	3	80474.11	302.70	0.2	0.0	0.20		
107	3	80627.23	302.70	0.2	0.0	0.20		
108	3	80774.96	302.71	0.2	0.0	0.20		
109	2	80917.46	302.71	0.2	0.0	0.20		
110	2	81054.92	302.72	0.2	0.0	0.20		
111	2	81187.49	302.72	0.2	0.0	0.20		
112	2	81315.34	302.73	0.2	0.0	0.20		
113	2	81438.61	302.73	0.2	0.0	0.20		
114	2	81557.47	302.74	0.2	0.0	0.20		
115	2	81672.05	302.74	0.2	0.0	0.20		
116	2	81782.49	302.75	0.2	0.0	0.20		
117	2	81888.93	302.75	0.2	0.0	0.20		
118	2	81991.50	302.76	0.2	0.0	0.20		
119	2	82090.33	302.76	0.2	0.0	0.20		
120	2	82185.53	302.76	0.2	0.0	0.20		
121	2	82277.23	302.77	0.2	0.0	0.20		
122	2	82365.55	302.77	0.2	0.0	0.20		

**OUTPUT**

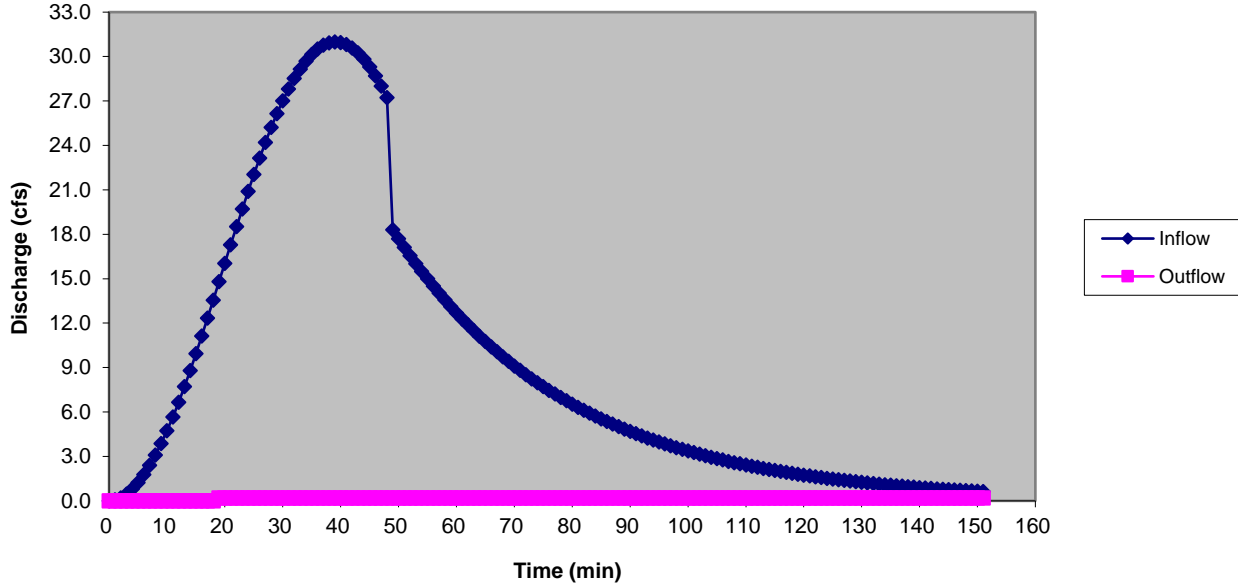
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	2	82450.58	302.78	0.2	0.0	0.20		
124	2	82532.44	302.78	0.2	0.0	0.20		
125	1	82611.23	302.78	0.2	0.0	0.20		
126	1	82687.06	302.78	0.2	0.0	0.20		
127	1	82760.02	302.79	0.2	0.0	0.20		
128	1	82830.20	302.79	0.2	0.0	0.20		
129	1	82897.69	302.79	0.2	0.0	0.20		
130	1	82962.59	302.80	0.2	0.0	0.20		
131	1	83024.97	302.80	0.2	0.0	0.20		
132	1	83084.93	302.80	0.2	0.0	0.20		
133	1	83142.53	302.80	0.2	0.0	0.20		
134	1	83197.85	302.81	0.2	0.0	0.20		
135	1	83250.98	302.81	0.2	0.0	0.20		
136	1	83301.98	302.81	0.2	0.0	0.20		
137	1	83350.91	302.81	0.2	0.0	0.20		
138	1	83397.86	302.81	0.2	0.0	0.20		
139	1	83442.88	302.82	0.2	0.0	0.20		
140	1	83486.04	302.82	0.2	0.0	0.20		
141	1	83527.39	302.82	0.2	0.0	0.20		
142	1	83567.00	302.82	0.2	0.0	0.20		
143	1	83604.92	302.82	0.2	0.0	0.20		
144	1	83641.21	302.82	0.2	0.0	0.20		
145	1	83675.93	302.82	0.2	0.0	0.20		
146	1	83709.11	302.83	0.2	0.0	0.20		
147	1	83740.82	302.83	0.2	0.0	0.20		
148	1	83771.10	302.83	0.2	0.0	0.20		
149	1	83800.00	302.83	0.2	0.0	0.20		
150	1	83827.56	302.83	0.2	0.0	0.20		
151	1	83853.83	<b>302.83</b>	0.2	0.0	0.20		



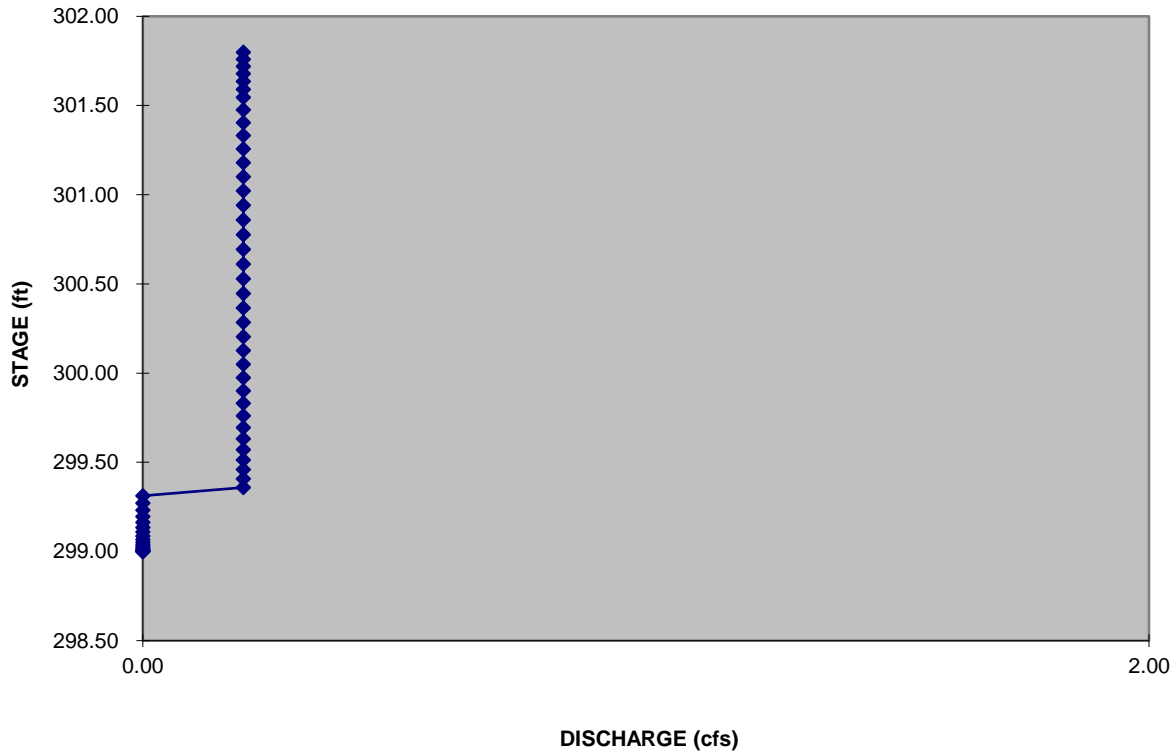
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM-G  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm



# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-G

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	5.76 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	5.76 ac.	0.25
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>5.76 ac.</b>	<b>5.76 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.25</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		<i>K Values</i>	
Length of flow path =	845 feet		Overland on grassed surfaces:	2.0
Height of watershed =	21 feet		Overland on paved surfaces:	0.4
Calculated t(c) =	5.8 minutes		Channel in natural channels:	1.0
Minimum t(c) =	5.0 minutes		Channel in mixed urban setting:	1.1
Time of concentration =	5.8 minutes		Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	5.54 in/hr
10	195	22	7.01 in/hr
25	232	23	8.05 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.54 in/hr	14.4 cfs	7.98 cfs
10-year storm	7.01 in/hr	18.2 cfs	10.10 cfs
25-year storm	8.05 in/hr	20.9 cfs	11.60 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	160
Bottom width =	80
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	16
Height of berm =	6
Top of trap length =	196
Top of trap width =	116
Bottom elevation =	302

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	20736 cu. ft.	63460 cu. ft.	
Sediment surface area =	7910 sq. ft.	19140 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W	
Spillway length =	10.0 ft. (min)	16	
10-Year flow depth over spillway =	0.50 ft. (max)	-0.07 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	2.07 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 20,736 cubic feet

24 HOURS	Best Option =	1 - 5 inch Skimmers with a 4 inch orifice
2 DAYS	Best Option =	1 - 4 inch Skimmers with a 2.9 inch orifice
3 DAYS	Best Option =	1 - 3 inch Skimmers with a 2.5 inch orifice
4 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.3 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

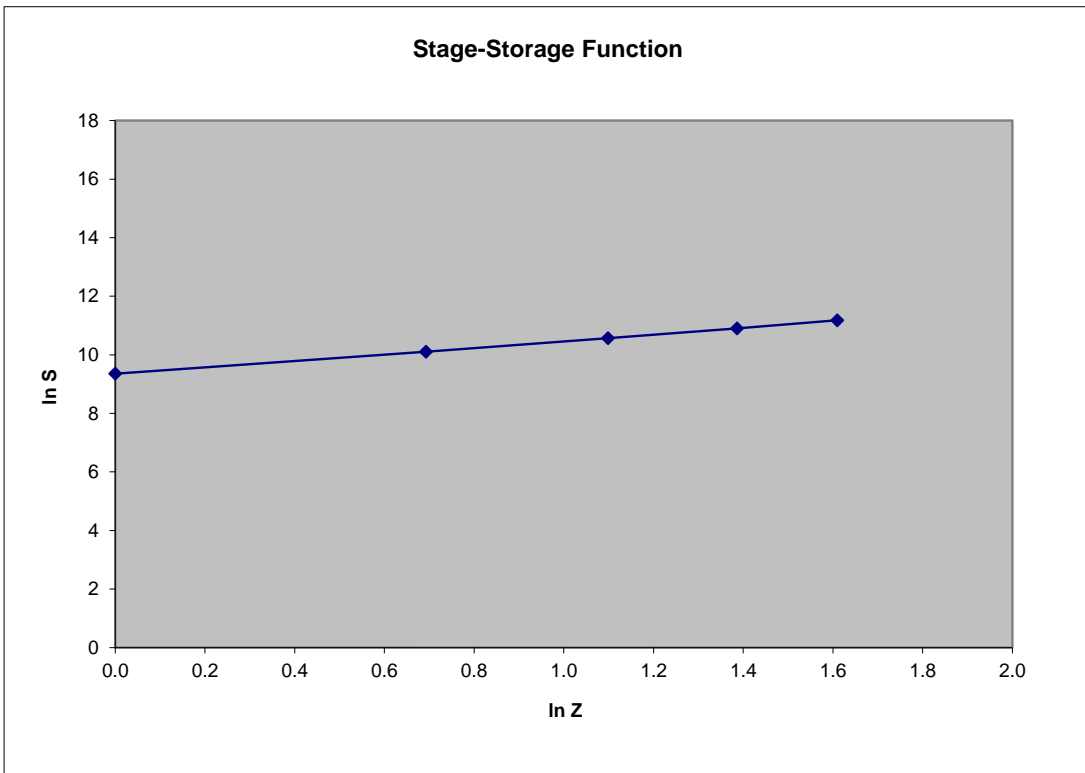
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
302	10884		0	0			
303	12195	11540	11539.5	1	9.3535	0.0000	1.01
304	13563	12879	24418.5	2	10.1031	0.6931	1.97
305	14987	14275	38693.5	3	10.5634	1.0986	2.95
306	16468	15727.5	54421	4	10.9045	1.3863	3.99
307	18005	17236.5	71657.5	5	11.1797	1.6094	5.09

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.13$  and  $K_s = 11354$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT	OUTPUT
Drainage Area, DA = <span style="border: 1px solid black; padding: 2px;">5.76</span> ac Disturbed Area = <span style="border: 1px solid black; padding: 2px;">5.76</span> ac Undisturbed Woods = <span style="border: 1px solid black; padding: 2px;">0.00</span> ac Undisturbed Grass = <span style="border: 1px solid black; padding: 2px;">0.00</span> ac Hydraulic Length = <span style="border: 1px solid black; padding: 2px;">845</span> ft Vertical Fall = <span style="border: 1px solid black; padding: 2px;">21</span> ft For 1 yr Storm, g = <span style="border: 1px solid black; padding: 2px;">132</span> h = <span style="border: 1px solid black; padding: 2px;">18</span> For 10 yr Storm, g = <span style="border: 1px solid black; padding: 2px;">195</span> h = <span style="border: 1px solid black; padding: 2px;">22</span> For 25 yr Storm, g = <span style="border: 1px solid black; padding: 2px;">232</span> h = <span style="border: 1px solid black; padding: 2px;">23</span>	<b>Use Kirpich Equation:</b> Tc = <span style="border: 1px solid black; padding: 2px;">5.8 min</span>  <b>Use Malcom Method:</b> i <sub>1</sub> = <span style="border: 1px solid black; padding: 2px;">5.54 in/hr</span> i <sub>10</sub> = <span style="border: 1px solid black; padding: 2px;">7.01 in/hr</span> i <sub>25</sub> = <span style="border: 1px solid black; padding: 2px;">8.05 in/hr</span>  <b>Use Rational Method:</b> Q <sub>2</sub> = <span style="border: 1px solid black; padding: 2px;">14.4 cfs</span> Q <sub>10</sub> = <span style="border: 1px solid black; padding: 2px;">18.2 cfs</span> Q <sub>25</sub> = <span style="border: 1px solid black; padding: 2px;">20.9 cfs</span>
"C" CALCULATION - See Sediment Trap Sizing  "C" = <span style="border-bottom: 3px double black; padding: 0 20px;">0.45</span>	

**SOLUTION - Tp:**

INPUT	OUTPUT
P <sub>n6</sub> = <span style="border: 1px solid black; padding: 2px;">2.7</span> in (2 Year Storm) CN = <span style="border: 1px solid black; padding: 2px;">91</span> (newly graded areas)	S = <span style="border: 1px solid black; padding: 2px;">0.99</span> in Q* = <span style="border: 1px solid black; padding: 2px;">1.75</span> in  T <sub>p2</sub> = <span style="border: 1px solid black; padding: 2px;">30.5</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid black; padding: 2px;">3.9</span> in (10 Year Storm)	Q* = <span style="border: 1px solid black; padding: 2px;">2.92</span> in  T <sub>p10</sub> = <span style="border: 1px solid black; padding: 2px;">40.3</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid black; padding: 2px;">4.6</span> in (25 Year Storm)	Q* = <span style="border: 1px solid black; padding: 2px;">3.59</span> in  T <sub>p25</sub> = <span style="border: 1px solid black; padding: 2px;">43.2</span> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	304.75	ft
	Rise =	2.75	ft
Freeboard =	3.25	ft	Peak Outflow =
			0.11
			cfs

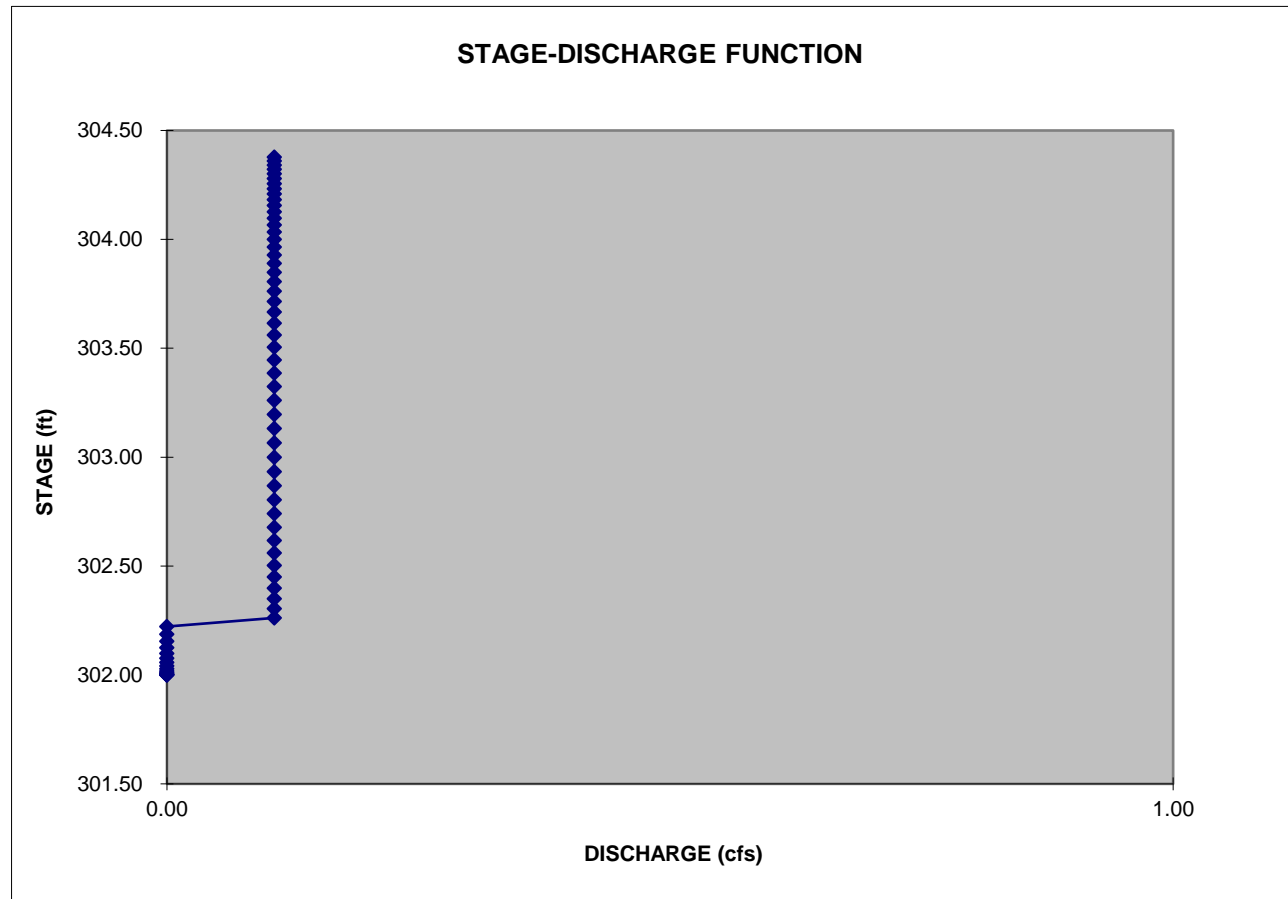
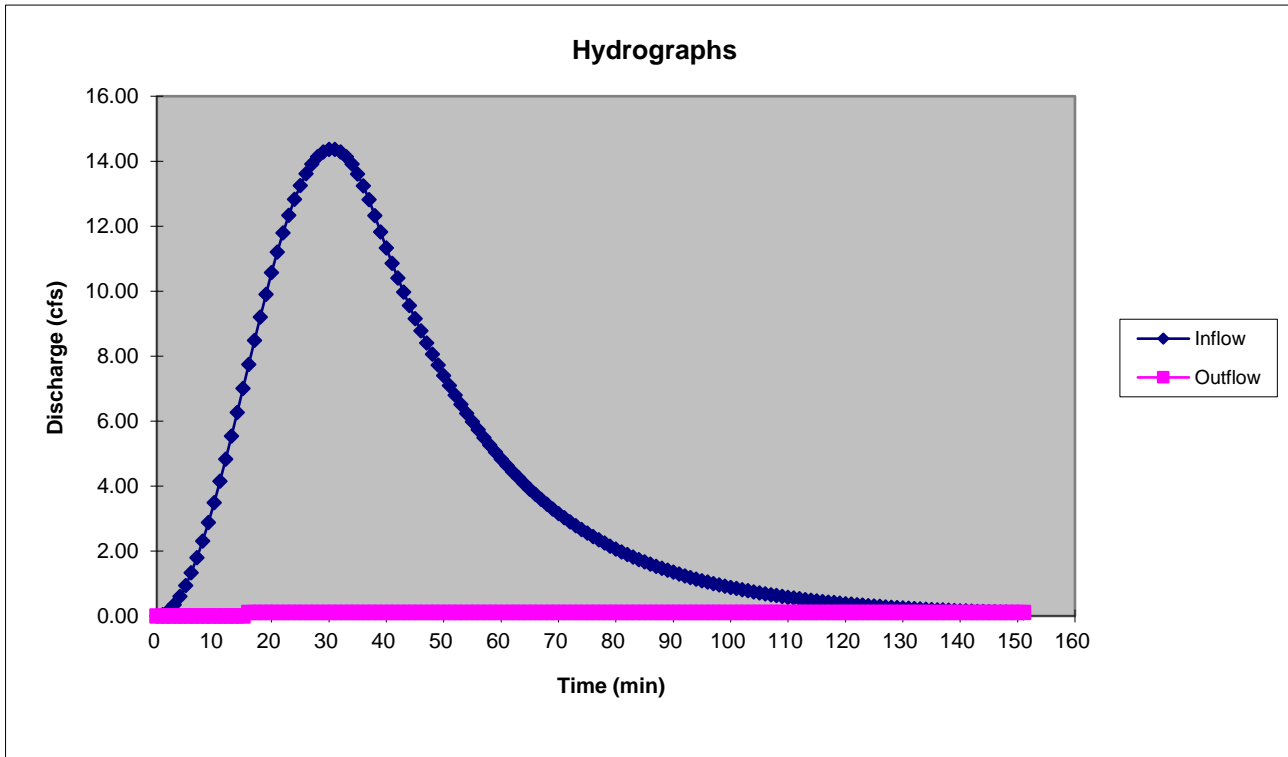
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 308.00 ft
Qp =	14.37	cfs	N =
			1
Tp =	30.5	min	L =
			16
dT =	1.0	min	Cw =
			3.00
<b>Stage-Storage Results:</b>			Zcr =
Ks =	11354		306.00
b =	1.13		
Z <sub>0</sub> =	302.0	ft (inv)	
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	302.00	ft	
			<b>Skimmer Orifice:</b>
			Number =
			1.00
			Diameter =
			3.00
			Head =
			2.52
			inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	302.00	0.0	0.0	0.0		
1	0	0.00	302.00	0.0	0.0	0.0		
2	0	2.29	302.00	0.0	0.0	0.0		
3	0	11.41	302.00	0.0	0.0	0.0		
4	1	31.84	302.01	0.0	0.0	0.0		
5	1	67.94	302.01	0.0	0.0	0.0		
6	1	123.90	302.02	0.0	0.0	0.0		
7	2	203.70	302.03	0.0	0.0	0.0		
8	2	311.06	302.04	0.0	0.0	0.0		
9	3	449.41	302.06	0.0	0.0	0.0		
10	3	621.87	302.08	0.0	0.0	0.0		
11	4	831.18	302.10	0.0	0.0	0.0		
12	5	1079.69	302.13	0.0	0.0	0.0		
13	6	1369.33	302.15	0.0	0.0	0.0		
14	6	1701.60	302.19	0.0	0.0	0.0		
15	7	2077.57	302.22	0.0	0.0	0.0		
16	8	2497.80	302.26	0.1	0.0	0.11		
17	8	2956.02	302.30	0.1	0.0	0.11		
18	9	3458.26	302.35	0.1	0.0	0.11		
19	10	4003.72	302.40	0.1	0.0	0.11		
20	11	4591.09	302.45	0.1	0.0	0.11		
21	11	5218.67	302.50	0.1	0.0	0.11		
22	12	5884.29	302.56	0.1	0.0	0.11		
23	12	6585.40	302.62	0.1	0.0	0.11		
24	13	7319.07	302.68	0.1	0.0	0.11		
25	13	8082.01	302.74	0.1	0.0	0.11		
26	14	8870.65	302.80	0.1	0.0	0.11		
27	14	9681.12	302.87	0.1	0.0	0.11		
28	14	10509.32	302.93	0.1	0.0	0.11		
29	14	11350.98	303.00	0.1	0.0	0.11		
30	14	12201.68	303.07	0.1	0.0	0.11		
31	14	13056.88	303.13	0.1	0.0	0.11		
32	14	13912.04	303.20	0.1	0.0	0.11		
33	14	14762.57	303.26	0.1	0.0	0.11		
34	14	15603.97	303.32	0.1	0.0	0.11		
35	14	16431.80	303.39	0.1	0.0	0.11		
36	13	17241.80	303.45	0.1	0.0	0.11		
37	13	18029.88	303.50	0.1	0.0	0.11		
38	12	18792.18	303.56	0.1	0.0	0.11		
39	12	19525.12	303.61	0.1	0.0	0.11		
40	11	20228.09	303.67	0.1	0.0	0.11		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	2.07	ft	
Peak Stage =	305.93	ft	
Rise =	3.93	ft	
Peak Outflow =	0.11	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	308.00 ft
Qp = 18.17 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 40.3 min	L = 16 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 3.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 306.00 ft	Head = 2.52 inches	
Ks = 11354			
b = 1.13			
Z <sub>0</sub> = 302.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 302.00 ft			

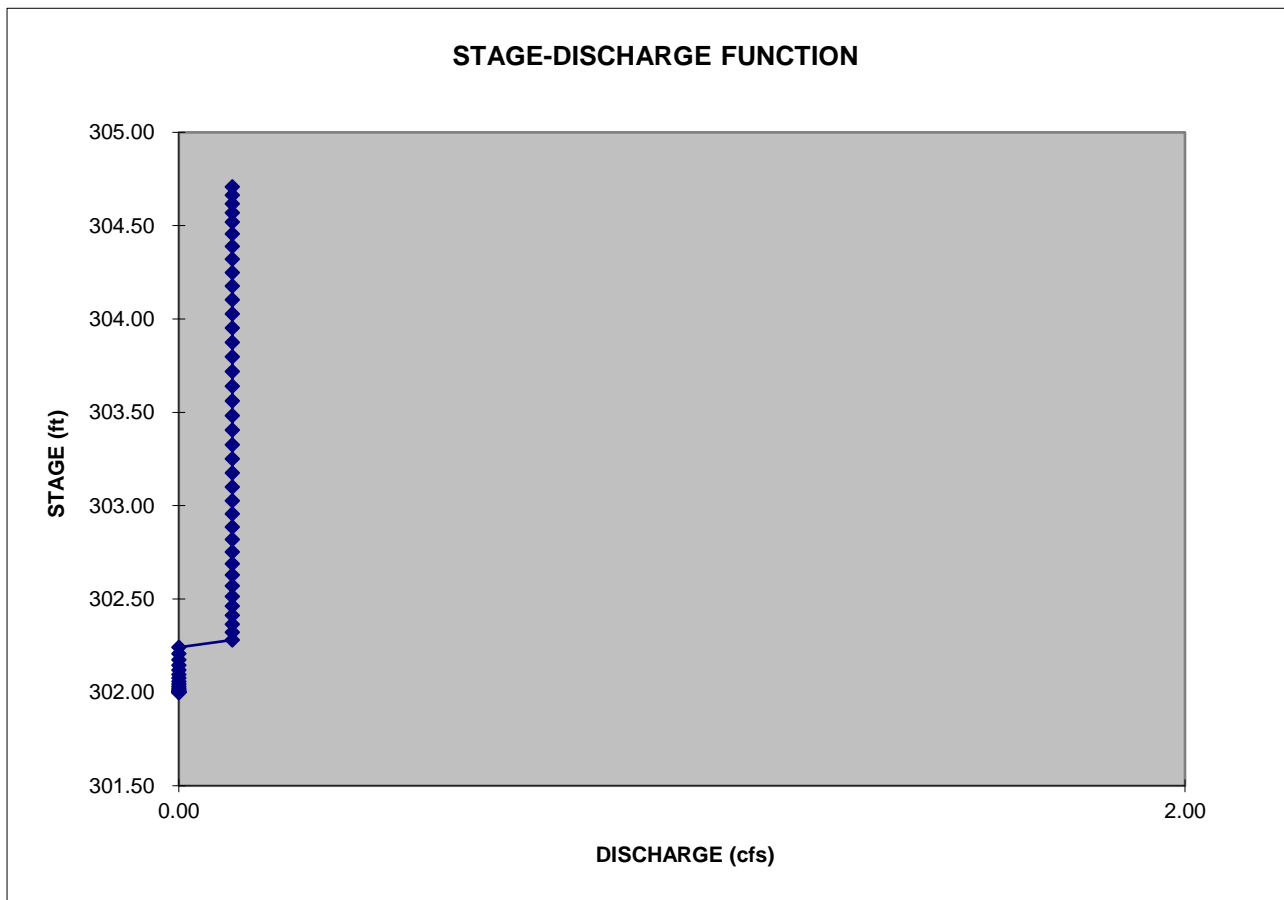
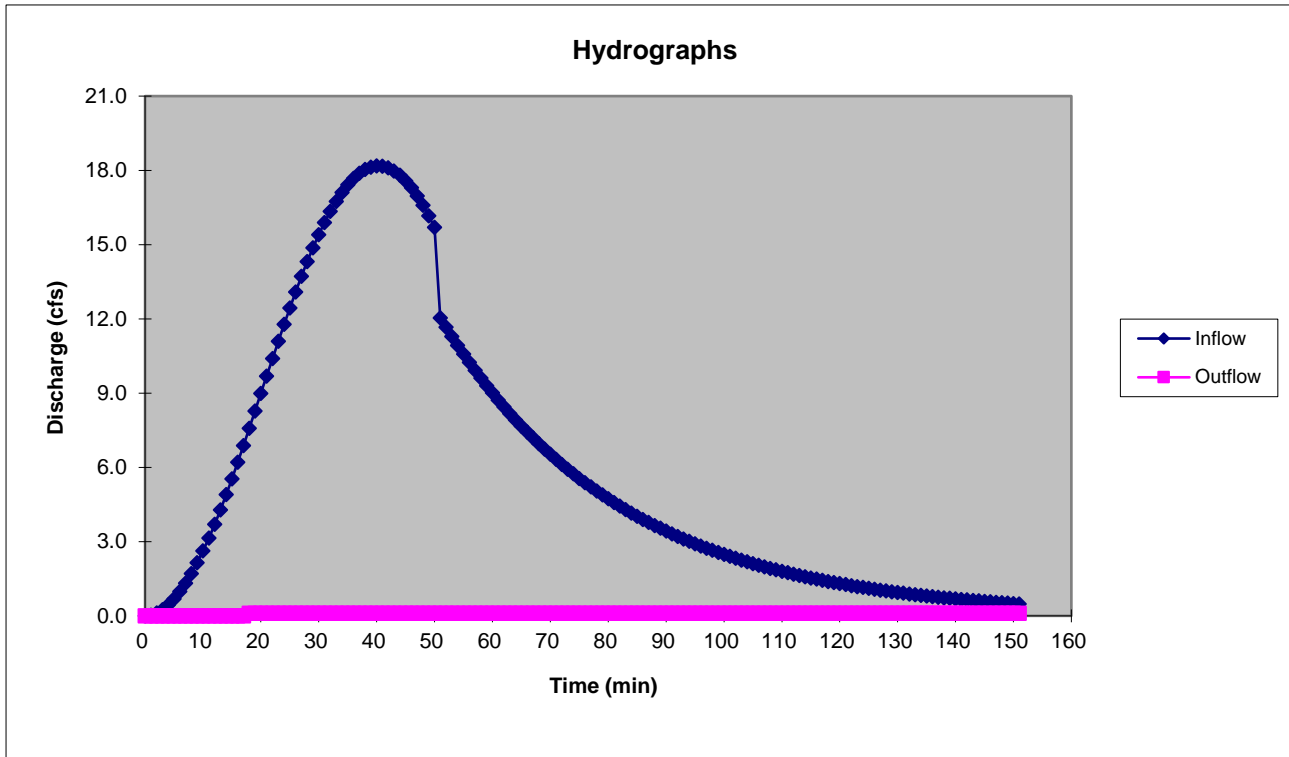
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	302.00	0.0	0.0	0.0		
1	0	0.00	302.00	0.0	0.0	0.00		
2	0	1.66	302.00	0.0	0.0	0.00		
3	0	8.27	302.00	0.0	0.0	0.00		
4	0	23.11	302.00	0.0	0.0	0.00		
5	1	49.39	302.01	0.0	0.0	0.00		
6	1	90.28	302.01	0.0	0.0	0.00		
7	1	148.83	302.02	0.0	0.0	0.00		
8	2	228.00	302.03	0.0	0.0	0.00		
9	2	330.61	302.04	0.0	0.0	0.00		
10	3	459.37	302.06	0.0	0.0	0.00		
11	3	616.78	302.08	0.0	0.0	0.00		
12	4	805.22	302.10	0.0	0.0	0.00		
13	4	1026.84	302.12	0.0	0.0	0.00		
14	5	1283.62	302.15	0.0	0.0	0.00		
15	6	1577.30	302.17	0.0	0.0	0.00		
16	6	1909.41	302.21	0.0	0.0	0.00		
17	7	2281.24	302.24	0.0	0.0	0.00		
18	8	2693.86	302.28	0.1	0.0	0.11		
19	8	3141.66	302.32	0.1	0.0	0.11		
20	9	3631.60	302.37	0.1	0.0	0.11		
21	10	4163.97	302.41	0.1	0.0	0.11		
22	10	4738.82	302.46	0.1	0.0	0.11		
23	11	5355.92	302.51	0.1	0.0	0.11		
24	12	6014.80	302.57	0.1	0.0	0.11		
25	12	6714.72	302.63	0.1	0.0	0.11		
26	13	7454.72	302.69	0.1	0.0	0.11		
27	14	8233.57	302.75	0.1	0.0	0.11		
28	14	9049.81	302.82	0.1	0.0	0.11		
29	15	9901.76	302.89	0.1	0.0	0.11		
30	15	10787.51	302.96	0.1	0.0	0.11		
31	16	11704.96	303.03	0.1	0.0	0.11		
32	16	12651.81	303.10	0.1	0.0	0.11		
33	17	13625.58	303.17	0.1	0.0	0.11		
34	17	14623.63	303.25	0.1	0.0	0.11		
35	17	15643.18	303.33	0.1	0.0	0.11		
36	18	16681.29	303.40	0.1	0.0	0.11		
37	18	17734.95	303.48	0.1	0.0	0.11		
38	18	18801.02	303.56	0.1	0.0	0.11		
39	18	19876.30	303.64	0.1	0.0	0.11		
40	18	20957.54	303.72	0.1	0.0	0.11		
41	18	22041.44	303.80	0.1	0.0	0.11		
42	18	23124.68	303.87	0.1	0.0	0.11		
43	18	24203.97	303.95	0.1	0.0	0.11		
44	18	25276.02	304.03	0.1	0.0	0.11		
45	18	26337.60	304.10	0.1	0.0	0.11		
46	17	27385.52	304.18	0.1	0.0	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	17	28416.71	304.25	0.1	0.0	0.11		
48	17	29428.16	304.32	0.1	0.0	0.11		
49	16	30417.00	304.39	0.1	0.0	0.11		
50	16	31380.52	304.46	0.1	0.0	0.11		
51	12	32316.11	304.52	0.1	0.0	0.11		
52	12	33031.91	304.57	0.1	0.0	0.11		
53	11	33724.79	304.62	0.1	0.0	0.11		
54	11	34395.47	304.66	0.1	0.0	0.11		
55	11	35044.66	304.71	0.1	0.0	0.11		
56	10	35673.04	304.75	0.1	0.0	0.11		
57	10	36281.27	304.79	0.1	0.0	0.11		
58	10	36869.99	304.83	0.1	0.0	0.11		
59	9	37439.83	304.87	0.1	0.0	0.11		
60	9	37991.37	304.91	0.1	0.0	0.11		
61	9	38525.21	304.94	0.1	0.0	0.11		
62	8	39041.90	304.98	0.1	0.0	0.11		
63	8	39541.98	305.01	0.1	0.0	0.11		
64	8	40026.00	305.04	0.1	0.0	0.11		
65	8	40494.44	305.08	0.1	0.0	0.11		
66	7	40947.81	305.11	0.1	0.0	0.11		
67	7	41386.59	305.14	0.1	0.0	0.11		
68	7	41811.24	305.16	0.1	0.0	0.11		
69	7	42222.21	305.19	0.1	0.0	0.11		
70	7	42619.93	305.22	0.1	0.0	0.11		
71	6	43004.82	305.24	0.1	0.0	0.11		
72	6	43377.30	305.27	0.1	0.0	0.11		
73	6	43737.75	305.29	0.1	0.0	0.11		
74	6	44086.55	305.32	0.1	0.0	0.11		
75	6	44424.08	305.34	0.1	0.0	0.11		
76	5	44750.70	305.36	0.1	0.0	0.11		
77	5	45066.74	305.38	0.1	0.0	0.11		
78	5	45372.55	305.40	0.1	0.0	0.11		
79	5	45668.45	305.42	0.1	0.0	0.11		
80	5	45954.76	305.44	0.1	0.0	0.11		
81	5	46231.78	305.46	0.1	0.0	0.11		
82	4	46499.79	305.48	0.1	0.0	0.11		
83	4	46759.10	305.49	0.1	0.0	0.11		
84	4	47009.98	305.51	0.1	0.0	0.11		
85	4	47252.69	305.53	0.1	0.0	0.11		
86	4	47487.49	305.54	0.1	0.0	0.11		
87	4	47714.64	305.56	0.1	0.0	0.11		
88	4	47934.38	305.57	0.1	0.0	0.11		
89	4	48146.94	305.58	0.1	0.0	0.11		
90	3	48352.54	305.60	0.1	0.0	0.11		
91	3	48551.42	305.61	0.1	0.0	0.11		
92	3	48743.79	305.62	0.1	0.0	0.11		
93	3	48929.84	305.64	0.1	0.0	0.11		
94	3	49109.79	305.65	0.1	0.0	0.11		
95	3	49283.82	305.66	0.1	0.0	0.11		
96	3	49452.12	305.67	0.1	0.0	0.11		
97	3	49614.88	305.68	0.1	0.0	0.11		
98	3	49772.27	305.69	0.1	0.0	0.11		
99	3	49924.47	305.70	0.1	0.0	0.11		
100	2	50071.63	305.71	0.1	0.0	0.11		
101	2	50213.91	305.72	0.1	0.0	0.11		
102	2	50351.48	305.73	0.1	0.0	0.11		
103	2	50484.47	305.74	0.1	0.0	0.11		
104	2	50613.04	305.75	0.1	0.0	0.11		
105	2	50737.33	305.75	0.1	0.0	0.11		
106	2	50857.47	305.76	0.1	0.0	0.11		
107	2	50973.60	305.77	0.1	0.0	0.11		
108	2	51085.83	305.78	0.1	0.0	0.11		
109	2	51194.30	305.78	0.1	0.0	0.11		
110	2	51299.13	305.79	0.1	0.0	0.11		
111	2	51400.42	305.80	0.1	0.0	0.11		
112	2	51498.29	305.80	0.1	0.0	0.11		
113	2	51592.86	305.81	0.1	0.0	0.11		
114	2	51684.22	305.82	0.1	0.0	0.11		
115	2	51772.48	305.82	0.1	0.0	0.11		
116	1	51857.74	305.83	0.1	0.0	0.11		
117	1	51940.08	305.83	0.1	0.0	0.11		
118	1	52019.61	305.84	0.1	0.0	0.11		
119	1	52096.41	305.84	0.1	0.0	0.11		
120	1	52170.57	305.85	0.1	0.0	0.11		
121	1	52242.18	305.85	0.1	0.0	0.11		
122	1	52311.30	305.86	0.1	0.0	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	52378.04	305.86	0.1	0.0	0.11		
124	1	52442.45	305.87	0.1	0.0	0.11		
125	1	52504.61	305.87	0.1	0.0	0.11		
126	1	52564.59	305.87	0.1	0.0	0.11		
127	1	52622.47	305.88	0.1	0.0	0.11		
128	1	52678.31	305.88	0.1	0.0	0.11		
129	1	52732.18	305.88	0.1	0.0	0.11		
130	1	52784.13	305.89	0.1	0.0	0.11		
131	1	52834.23	305.89	0.1	0.0	0.11		
132	1	52882.54	305.89	0.1	0.0	0.11		
133	1	52929.11	305.90	0.1	0.0	0.11		
134	1	52973.99	305.90	0.1	0.0	0.11		
135	1	53017.26	305.90	0.1	0.0	0.11		
136	1	53058.94	305.91	0.1	0.0	0.11		
137	1	53099.10	305.91	0.1	0.0	0.11		
138	1	53137.78	305.91	0.1	0.0	0.11		
139	1	53175.03	305.91	0.1	0.0	0.11		
140	1	53210.90	305.92	0.1	0.0	0.11		
141	1	53245.42	305.92	0.1	0.0	0.11		
142	1	53278.65	305.92	0.1	0.0	0.11		
143	1	53310.61	305.92	0.1	0.0	0.11		
144	1	53341.37	305.92	0.1	0.0	0.11		
145	1	53370.94	305.93	0.1	0.0	0.11		
146	1	53399.37	305.93	0.1	0.0	0.11		
147	1	53426.69	305.93	0.1	0.0	0.11		
148	1	53452.94	305.93	0.1	0.0	0.11		
149	1	53478.16	305.93	0.1	0.0	0.11		
150	0	53502.38	305.93	0.1	0.0	0.11		
151	0	53525.62	305.94	0.1	0.0	0.11		



 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.91	ft	
Peak Stage =	306.09	ft	
Rise =	4.09	ft	
Peak Outflow =	1.38	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 308.00 ft	
Qp = 20.87 cfs	N = 1		
Tp = 40.3 min	L = 16 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 306.00 ft	<b>Skimmer Orifice:</b>	
Ks = 11354		Number = 1.00	Ea
b = 1.13		Diameter = 3.00	Inches
Z <sub>0</sub> = 302.0 ft (inv)		Head = 2.52	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 302.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	302.00	0.0	0.0	0.0		
1	0	0.00	302.00	0.0	0.0	0.00		
2	0	1.90	302.00	0.0	0.0	0.00		
3	0	9.49	302.00	0.0	0.0	0.00		
4	1	26.54	302.00	0.0	0.0	0.00		
5	1	56.73	302.01	0.0	0.0	0.00		
6	1	103.68	302.02	0.0	0.0	0.00		
7	2	170.93	302.02	0.0	0.0	0.00		
8	2	261.85	302.04	0.0	0.0	0.00		
9	2	379.69	302.05	0.0	0.0	0.00		
10	3	527.56	302.07	0.0	0.0	0.00		
11	4	708.34	302.09	0.0	0.0	0.00		
12	4	924.75	302.11	0.0	0.0	0.00		
13	5	1179.28	302.14	0.0	0.0	0.00		
14	6	1474.17	302.16	0.0	0.0	0.00		
15	6	1811.45	302.20	0.0	0.0	0.00		
16	7	2192.86	302.23	0.0	0.0	0.00		
17	8	2619.89	302.27	0.1	0.0	0.11		
18	9	3087.36	302.32	0.1	0.0	0.11		
19	10	3602.59	302.36	0.1	0.0	0.11		
20	10	4166.21	302.41	0.1	0.0	0.11		
21	11	4778.56	302.47	0.1	0.0	0.11		
22	12	5439.69	302.52	0.1	0.0	0.11		
23	13	6149.35	302.58	0.1	0.0	0.11		
24	14	6906.99	302.64	0.1	0.0	0.11		
25	14	7711.77	302.71	0.1	0.0	0.11		
26	15	8562.57	302.78	0.1	0.0	0.11		
27	16	9457.98	302.85	0.1	0.0	0.11		
28	16	10396.34	302.93	0.1	0.0	0.11		
29	17	11375.71	303.00	0.1	0.0	0.11		
30	18	12393.90	303.08	0.1	0.0	0.11		
31	18	13448.49	303.16	0.1	0.0	0.11		
32	19	14536.85	303.24	0.1	0.0	0.11		
33	19	15656.13	303.33	0.1	0.0	0.11		
34	20	16803.29	303.41	0.1	0.0	0.11		
35	20	17975.13	303.50	0.1	0.0	0.11		
36	20	19168.31	303.59	0.1	0.0	0.11		
37	21	20379.33	303.68	0.1	0.0	0.11		
38	21	21604.60	303.77	0.1	0.0	0.11		
39	21	22840.46	303.85	0.1	0.0	0.11		
40	21	24083.15	303.94	0.1	0.0	0.11		
41	21	25328.90	304.03	0.1	0.0	0.11		
42	21	26573.90	304.12	0.1	0.0	0.11		
43	21	27814.36	304.21	0.1	0.0	0.11		
44	20	29046.51	304.29	0.1	0.0	0.11		
45	20	30266.62	304.38	0.1	0.0	0.11		
46	20	31471.06	304.46	0.1	0.0	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	19	32656.27	304.54	0.1	0.0	0.11		
48	19	33818.82	304.62	0.1	0.0	0.11		
49	19	34955.41	304.70	0.1	0.0	0.11		
50	18	36062.90	304.78	0.1	0.0	0.11		
51	12	37138.34	304.85	0.1	0.0	0.11		
52	12	37854.13	304.90	0.1	0.0	0.11		
53	11	38547.01	304.95	0.1	0.0	0.11		
54	11	39217.69	304.99	0.1	0.0	0.11		
55	11	39866.88	305.03	0.1	0.0	0.11		
56	10	40495.26	305.08	0.1	0.0	0.11		
57	10	41103.49	305.12	0.1	0.0	0.11		
58	10	41692.22	305.16	0.1	0.0	0.11		
59	9	42262.05	305.19	0.1	0.0	0.11		
60	9	42813.60	305.23	0.1	0.0	0.11		
61	9	43347.43	305.27	0.1	0.0	0.11		
62	8	43864.12	305.30	0.1	0.0	0.11		
63	8	44364.21	305.33	0.1	0.0	0.11		
64	8	44848.22	305.37	0.1	0.0	0.11		
65	8	45316.66	305.40	0.1	0.0	0.11		
66	7	45770.04	305.43	0.1	0.0	0.11		
67	7	46208.82	305.46	0.1	0.0	0.11		
68	7	46633.47	305.48	0.1	0.0	0.11		
69	7	47044.43	305.51	0.1	0.0	0.11		
70	7	47442.15	305.54	0.1	0.0	0.11		
71	6	47827.05	305.56	0.1	0.0	0.11		
72	6	48199.52	305.59	0.1	0.0	0.11		
73	6	48559.97	305.61	0.1	0.0	0.11		
74	6	48908.78	305.63	0.1	0.0	0.11		
75	6	49246.31	305.66	0.1	0.0	0.11		
76	5	49572.92	305.68	0.1	0.0	0.11		
77	5	49888.97	305.70	0.1	0.0	0.11		
78	5	50194.78	305.72	0.1	0.0	0.11		
79	5	50490.68	305.74	0.1	0.0	0.11		
80	5	50776.98	305.76	0.1	0.0	0.11		
81	5	51054.00	305.78	0.1	0.0	0.11		
82	4	51322.02	305.79	0.1	0.0	0.11		
83	4	51581.33	305.81	0.1	0.0	0.11		
84	4	51832.20	305.83	0.1	0.0	0.11		
85	4	52074.91	305.84	0.1	0.0	0.11		
86	4	52309.72	305.86	0.1	0.0	0.11		
87	4	52536.87	305.87	0.1	0.0	0.11		
88	4	52756.60	305.89	0.1	0.0	0.11		
89	4	52969.16	305.90	0.1	0.0	0.11		
90	3	53174.77	305.91	0.1	0.0	0.11		
91	3	53373.65	305.93	0.1	0.0	0.11		
92	3	53566.01	305.94	0.1	0.0	0.11		
93	3	53752.06	305.95	0.1	0.0	0.11		
94	3	53932.01	305.96	0.1	0.0	0.11		
95	3	54106.04	305.97	0.1	0.0	0.11		
96	3	54274.35	305.98	0.1	0.0	0.11		
97	3	54437.11	306.00	0.1	0.0	0.11		
98	3	54594.50	306.01	0.1	0.0	0.11		
99	3	54745.46	306.02	0.2	0.1	0.11		
100	2	54887.09	306.02	0.3	0.2	0.11		
101	2	55018.24	306.03	0.4	0.3	0.11		
102	2	55138.44	306.04	0.5	0.4	0.11		
103	2	55247.61	306.05	0.6	0.5	0.11		
104	2	55345.91	306.05	0.7	0.6	0.11		
105	2	55433.71	306.06	0.8	0.7	0.11		
106	2	55511.49	306.07	0.9	0.8	0.11		
107	2	55579.82	306.07	1.0	0.9	0.11		
108	2	55639.31	306.07	1.1	1.0	0.11		
109	2	55690.59	306.08	1.1	1.0	0.11		
110	2	55734.30	306.08	1.2	1.1	0.11		
111	2	55771.06	306.08	1.2	1.1	0.11		
112	2	55801.49	306.08	1.3	1.2	0.11		
113	2	55826.17	306.09	1.3	1.2	0.11		
114	2	55845.63	306.09	1.3	1.2	0.11		
115	2	55860.40	306.09	1.4	1.2	0.11		
116	1	55870.94	306.09	1.4	1.3	0.11		
117	1	55877.70	306.09	1.4	1.3	0.11		
118	1	55881.08	306.09	1.4	1.3	0.11		
119	1	55881.46	306.09	1.4	1.3	0.11		
120	1	55879.16	306.09	1.4	1.3	0.11		
121	1	55874.50	306.09	1.4	1.3	0.11		
122	1	55867.74	306.09	1.4	1.3	0.11		

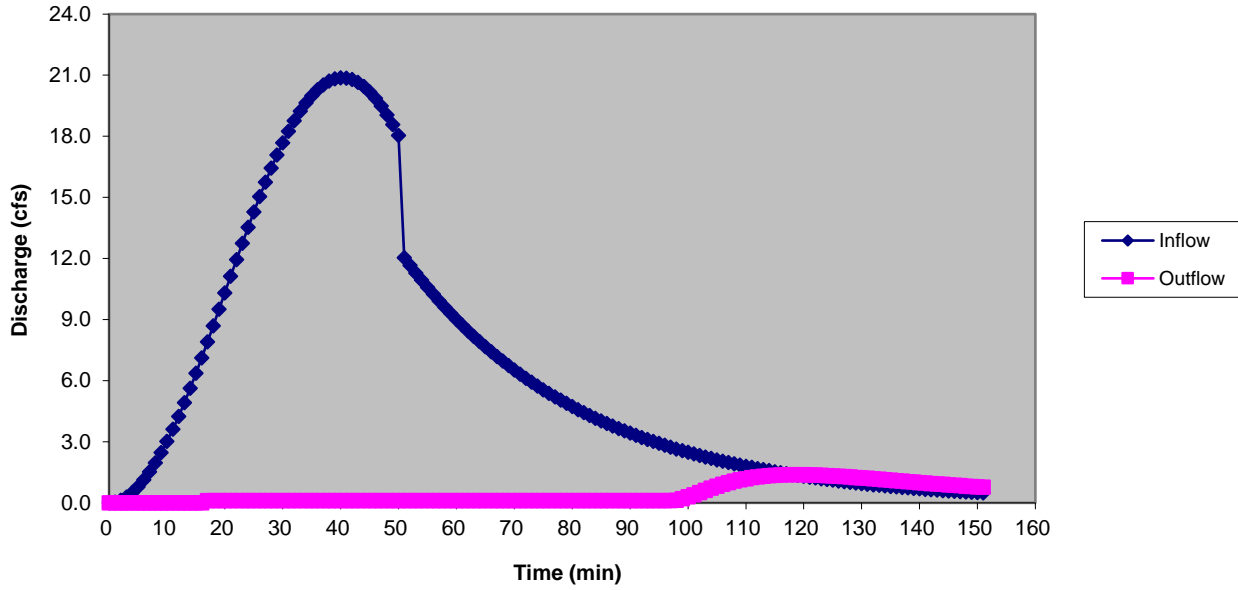
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	55859.16	306.09	1.4	1.2	0.11		
124	1	55848.96	306.09	1.3	1.2	0.11		
125	1	55837.35	306.09	1.3	1.2	0.11		
126	1	55824.53	306.09	1.3	1.2	0.11		
127	1	55810.64	306.08	1.3	1.2	0.11		
128	1	55795.85	306.08	1.3	1.2	0.11		
129	1	55780.28	306.08	1.2	1.1	0.11		
130	1	55764.04	306.08	1.2	1.1	0.11		
131	1	55747.26	306.08	1.2	1.1	0.11		
132	1	55730.01	306.08	1.2	1.1	0.11		
133	1	55712.39	306.08	1.2	1.0	0.11		
134	1	55694.47	306.08	1.1	1.0	0.11		
135	1	55676.32	306.08	1.1	1.0	0.11		
136	1	55657.99	306.07	1.1	1.0	0.11		
137	1	55639.54	306.07	1.1	1.0	0.11		
138	1	55621.01	306.07	1.0	0.9	0.11		
139	1	55602.45	306.07	1.0	0.9	0.11		
140	1	55583.89	306.07	1.0	0.9	0.11		
141	1	55565.36	306.07	1.0	0.9	0.11		
142	1	55546.90	306.07	0.9	0.8	0.11		
143	1	55528.53	306.07	0.9	0.8	0.11		
144	1	55510.26	306.06	0.9	0.8	0.11		
145	1	55492.13	306.06	0.9	0.8	0.11		
146	1	55474.14	306.06	0.9	0.8	0.11		
147	1	55456.30	306.06	0.8	0.7	0.11		
148	1	55438.64	306.06	0.8	0.7	0.11		
149	1	55421.17	306.06	0.8	0.7	0.11		
150	0	55403.88	306.06	0.8	0.7	0.11		
151	0	55386.79	306.06	0.8	0.7	0.11		



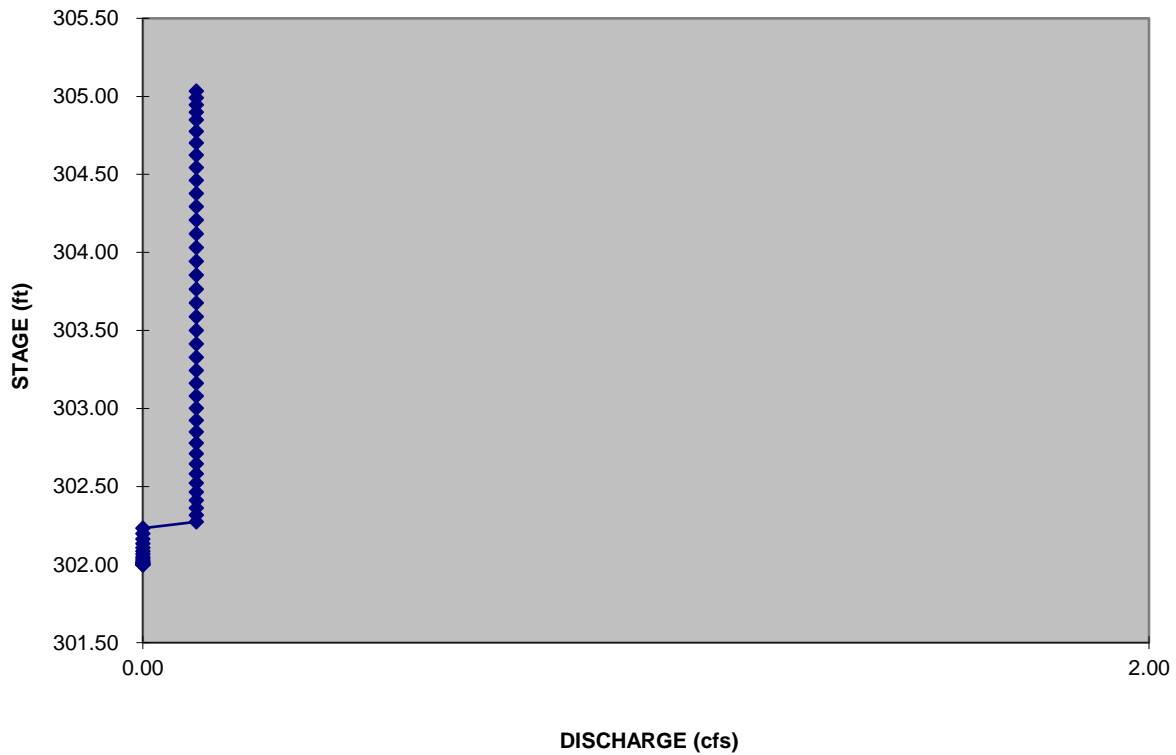
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM-H  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-H

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	2.66 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	2.00 ac.	0.25
Undisturbed grassy area	0.00 ac.	0.66 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>2.66 ac.</b>	<b>2.66 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.27</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		<i>K Values</i>	
Length of flow path =	605	feet	Overland on grassed surfaces:	2.0
Height of watershed =	9	feet	Overland on paved surfaces:	0.4
Calculated t(c) =	5.5	minutes	Channel in natural channels:	1.0
Minimum t(c) =	5.0	minutes	Channel in mixed urban setting:	1.1
Time of concentration =	5.5	minutes	Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for **Raleigh, NC**

Return Period	g	h	Intensity
2	132	18	5.62 in/hr
10	195	22	7.10 in/hr
25	232	23	8.15 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.62 in/hr	6.7 cfs	4.11 cfs
10-year storm	7.10 in/hr	8.5 cfs	5.19 cfs
25-year storm	8.15 in/hr	9.8 cfs	5.96 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	215
Bottom width =	65
Sediment depth =	3.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	3.0
Side slopes =	3.0 H:1V
Spillway length =	16
Height of berm =	5
Top of trap length =	245
Top of trap width =	95
Bottom elevation =	311

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	9576 cu. ft.	49760 cu. ft.
Sediment surface area =	3700 sq. ft.	19340 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	3
Trap bottom length to width ratio =	2.0L:1W (min)	3.3L:1W
Spillway length =	10.0 ft. (min)	16
10-Year flow depth over spillway =	0.50 ft. (max)	-1.37 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	3.37 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = **9,576** cubic feet

24 HOURS	Best Option =	1 - 3 inch Skimmers with a 3 inch orifice
2 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.2 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.9 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.6 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

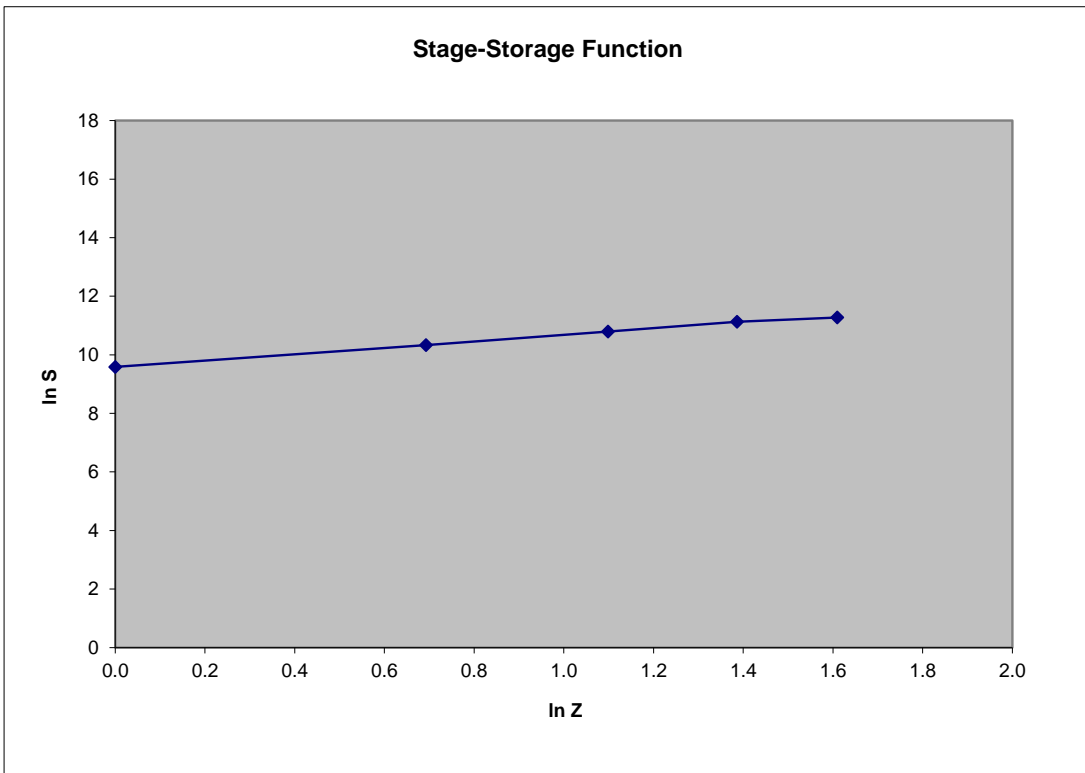
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
311	13780		0	0			
312	15377	14579	14578.5	1	9.5873	0.0000	0.99
313	17037	16207	30785.5	2	10.3348	0.6931	1.99
314	18760	17898.5	48684	3	10.7931	1.0986	3.05
315	20545	19652.5	68336.5	4	11.1322	1.3863	4.18
316	0	10272.5	78609	5	11.2722	1.6094	4.77

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.07$  and  $K_s = 14713$







# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT	OUTPUT
Drainage Area, DA = <span style="border: 1px solid gray; padding: 2px;">2.66</span> ac Disturbed Area = <span style="border: 1px solid gray; padding: 2px;">2.66</span> ac Undisturbed Woods = <span style="border: 1px solid gray; padding: 2px;">0.00</span> ac Undisturbed Grass = <span style="border: 1px solid gray; padding: 2px;">0.00</span> ac Hydraulic Length = <span style="border: 1px solid gray; padding: 2px;">605</span> ft Vertical Fall = <span style="border: 1px solid gray; padding: 2px;">9</span> ft For 1 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">132</span> h = <span style="border: 1px solid gray; padding: 2px;">18</span> For 10 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">195</span> h = <span style="border: 1px solid gray; padding: 2px;">22</span> For 25 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">232</span> h = <span style="border: 1px solid gray; padding: 2px;">23</span>	<b>Use Kirpich Equation:</b> Tc = <span style="border: 1px solid gray; padding: 2px;">5.5 min</span>  <b>Use Malcom Method:</b> i <sub>1</sub> = <span style="border: 1px solid gray; padding: 2px;">5.62 in/hr</span> i <sub>10</sub> = <span style="border: 1px solid gray; padding: 2px;">7.10 in/hr</span> i <sub>25</sub> = <span style="border: 1px solid gray; padding: 2px;">8.15 in/hr</span>  <b>Use Rational Method:</b> Q <sub>2</sub> = <span style="border: 1px solid gray; padding: 2px;">6.7 cfs</span> Q <sub>10</sub> = <span style="border: 1px solid gray; padding: 2px;">8.5 cfs</span> Q <sub>25</sub> = <span style="border: 1px solid gray; padding: 2px;">9.8 cfs</span>
"C" CALCULATION - See Sediment Trap Sizing  <b>"C" = <span style="border-bottom: 3px double orange; padding: 0 20px;">0.45</span></b>	

**SOLUTION - Tp:**

INPUT	OUTPUT
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">2.7</span> in (2 Year Storm) CN = <span style="border: 1px solid gray; padding: 2px;">91</span> (newly graded areas)	S = <span style="border: 1px solid gray; padding: 2px;">0.99</span> in Q* = <span style="border: 1px solid gray; padding: 2px;">1.75</span> in  T <sub>p2</sub> = <span style="border: 1px solid gray; padding: 2px;">30.1</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">3.9</span> in (10 Year Storm)	Q* = <span style="border: 1px solid gray; padding: 2px;">2.92</span> in  T <sub>p10</sub> = <span style="border: 1px solid gray; padding: 2px;">39.8</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">4.6</span> in (25 Year Storm)	Q* = <span style="border: 1px solid gray; padding: 2px;">3.59</span> in  T <sub>p25</sub> = <span style="border: 1px solid gray; padding: 2px;">42.7</span> minutes

# **McADAMS** **ROUTING FOR 2-YEAR STORM**

(REF: Malcom, 1991)

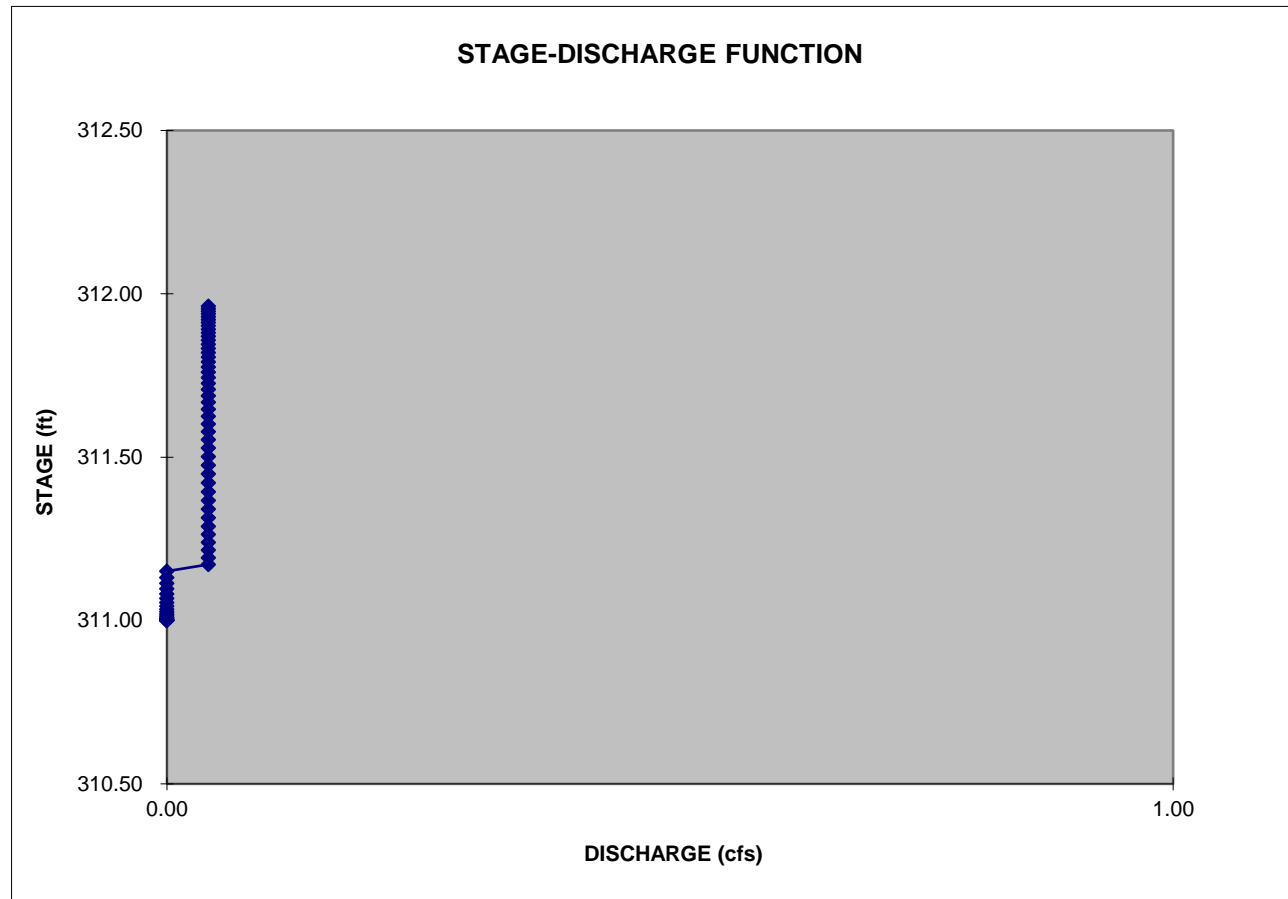
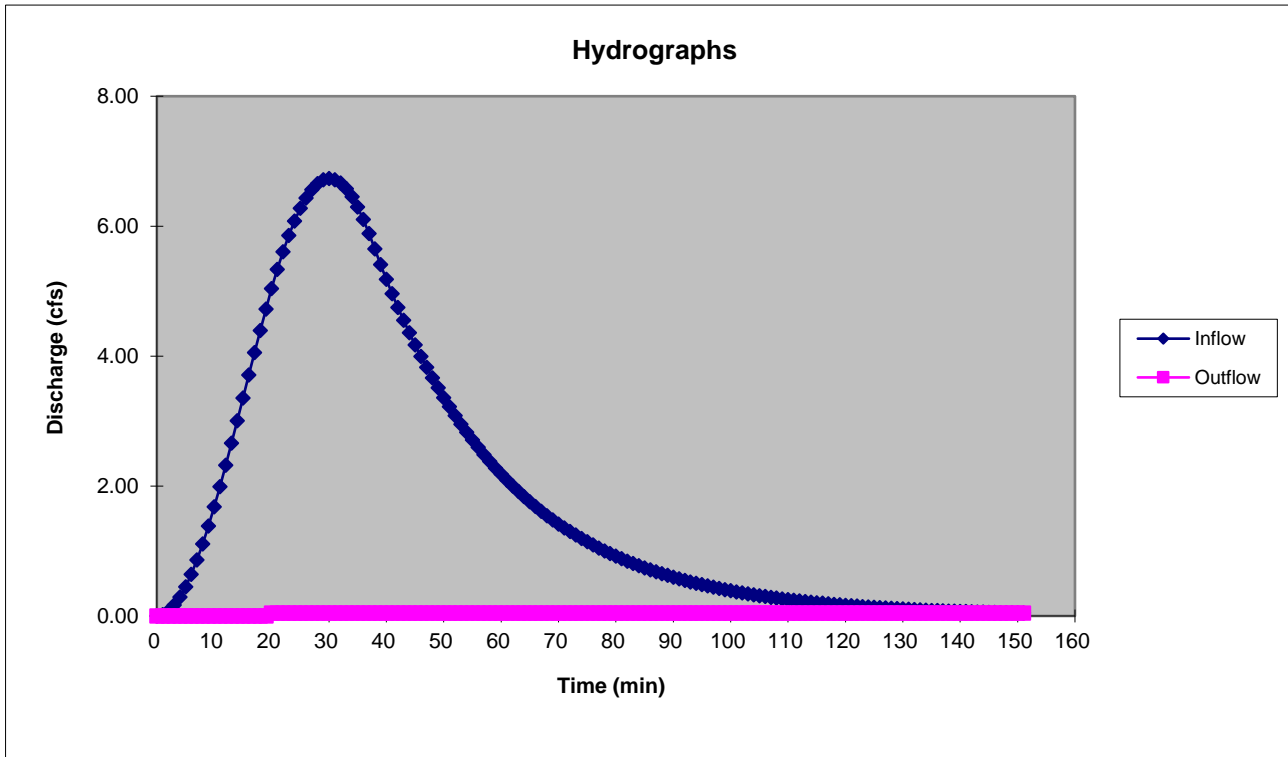
COMPUTED RESULTS			
	<b>Peak Stage =</b>	<b>312.12</b>	<b>ft</b>
	<b>Rise =</b>	<b>1.12</b>	<b>ft</b>
<b>Freeboard =</b>	<b>3.88</b>		
	<b>Peak Outflow =</b>	<b>0.04</b>	<b>cfs</b>

INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> <b>316.00</b> ft
Qp =	<b>6.73</b> cfs	N =	<b>1</b>
Tp =	<b>30.1</b> min	L =	<b>16</b> ft
dT =	<b>1.0</b> min	Cw =	<b>3.00</b>
<b>Stage-Storage Results:</b>		Zcr =	<b>314.00</b> ft
Ks =	<b>14713</b>		
b =	<b>1.07</b>		
Z <sub>0</sub> =	<b>311.0</b> ft (inv)		
<b>Initial Water Level:</b>		<b>Skimmer Orifice:</b>	
Z <sub>i</sub> =	<b>311.00</b> ft	Number =	<b>1.00</b> Ea
		Diameter =	<b>2.00</b> Inches
		Head =	<b>1.90</b> inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	311.00	0.0	0.0	0.0		
1	0	0.00	311.00	0.0	0.0	0.00		
2	0	1.10	311.00	0.0	0.0	0.00		
3	0	5.50	311.00	0.0	0.0	0.00		
4	0	15.34	311.00	0.0	0.0	0.00		
5	0	32.74	311.00	0.0	0.0	0.00		
6	1	59.69	311.01	0.0	0.0	0.00		
7	1	98.11	311.01	0.0	0.0	0.00		
8	1	149.79	311.01	0.0	0.0	0.00		
9	1	216.36	311.02	0.0	0.0	0.00		
10	2	299.31	311.03	0.0	0.0	0.00		
11	2	399.92	311.03	0.0	0.0	0.00		
12	2	519.31	311.04	0.0	0.0	0.00		
13	3	658.38	311.06	0.0	0.0	0.00		
14	3	817.82	311.07	0.0	0.0	0.00		
15	3	998.08	311.08	0.0	0.0	0.00		
16	4	1199.40	311.10	0.0	0.0	0.00		
17	4	1421.80	311.11	0.0	0.0	0.00		
18	4	1665.04	311.13	0.0	0.0	0.00		
19	5	1928.68	311.15	0.0	0.0	0.00		
20	5	2212.04	311.17	0.0	0.0	0.04		
21	5	2511.76	311.19	0.0	0.0	0.04		
22	6	2829.23	311.22	0.0	0.0	0.04		
23	6	3163.14	311.24	0.0	0.0	0.04		
24	6	3512.04	311.26	0.0	0.0	0.04		
25	6	3874.30	311.29	0.0	0.0	0.04		
26	6	4248.13	311.31	0.0	0.0	0.04		
27	7	4631.63	311.34	0.0	0.0	0.04		
28	7	5022.80	311.37	0.0	0.0	0.04		
29	7	5419.53	311.39	0.0	0.0	0.04		
30	7	5819.69	311.42	0.0	0.0	0.04		
31	7	6221.07	311.45	0.0	0.0	0.04		
32	7	6621.48	311.48	0.0	0.0	0.04		
33	7	7018.72	311.50	0.0	0.0	0.04		
34	6	7410.63	311.53	0.0	0.0	0.04		
35	6	7795.11	311.55	0.0	0.0	0.04		
36	6	8170.14	311.58	0.0	0.0	0.04		
37	6	8533.81	311.60	0.0	0.0	0.04		
38	6	8884.33	311.62	0.0	0.0	0.04		
39	5	9220.65	311.65	0.0	0.0	0.04		
40	5	9542.64	311.67	0.0	0.0	0.04		



# GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.37	ft	
Peak Stage =	312.63	ft	
Rise =	1.63	ft	
Peak Outflow =	0.04	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	316.00 ft
Qp = 8.50 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.8 min	L = 16 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Head = 1.90 inches	
Ks = 14713			
b = 1.07			
Z <sub>0</sub> = 311.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 311.00 ft			

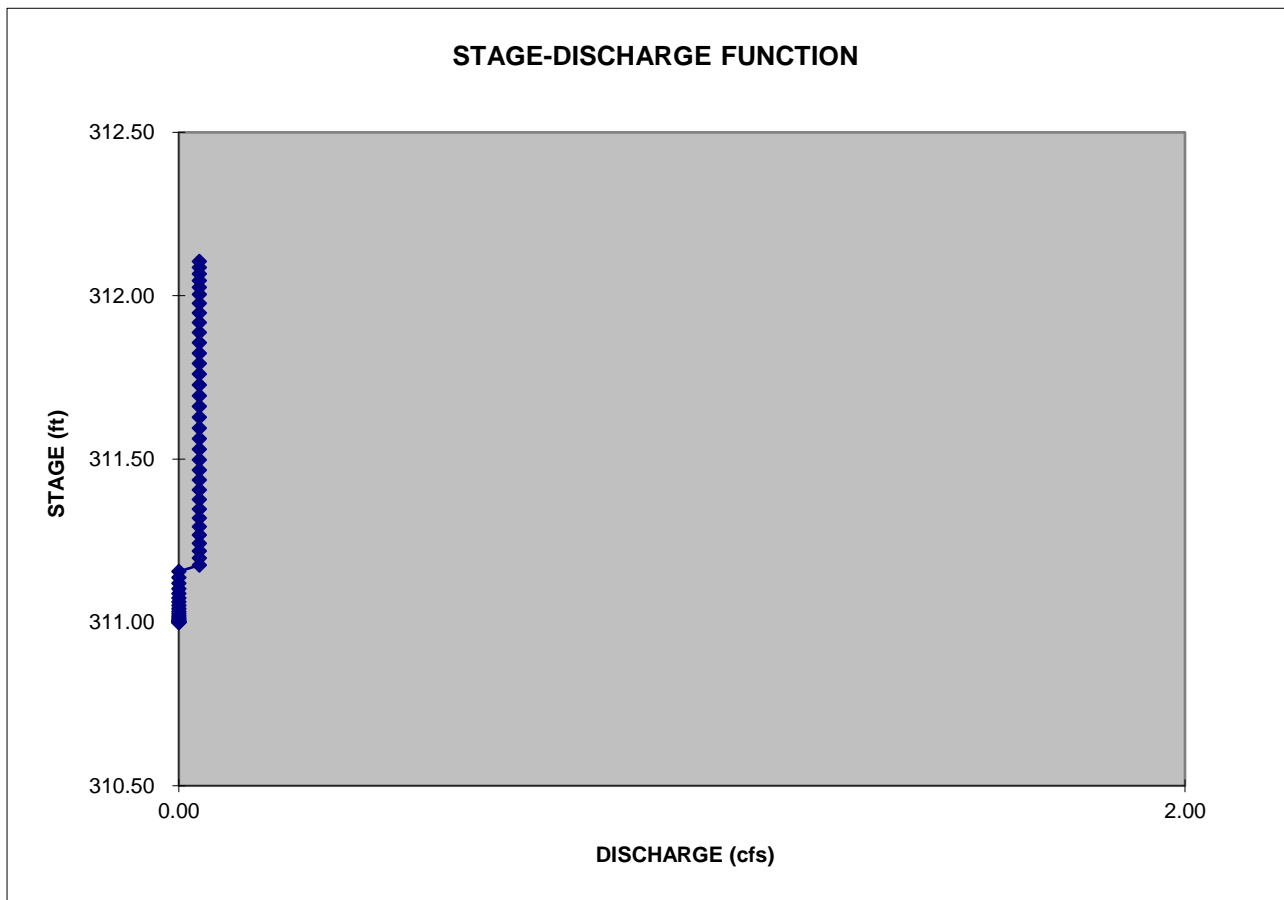
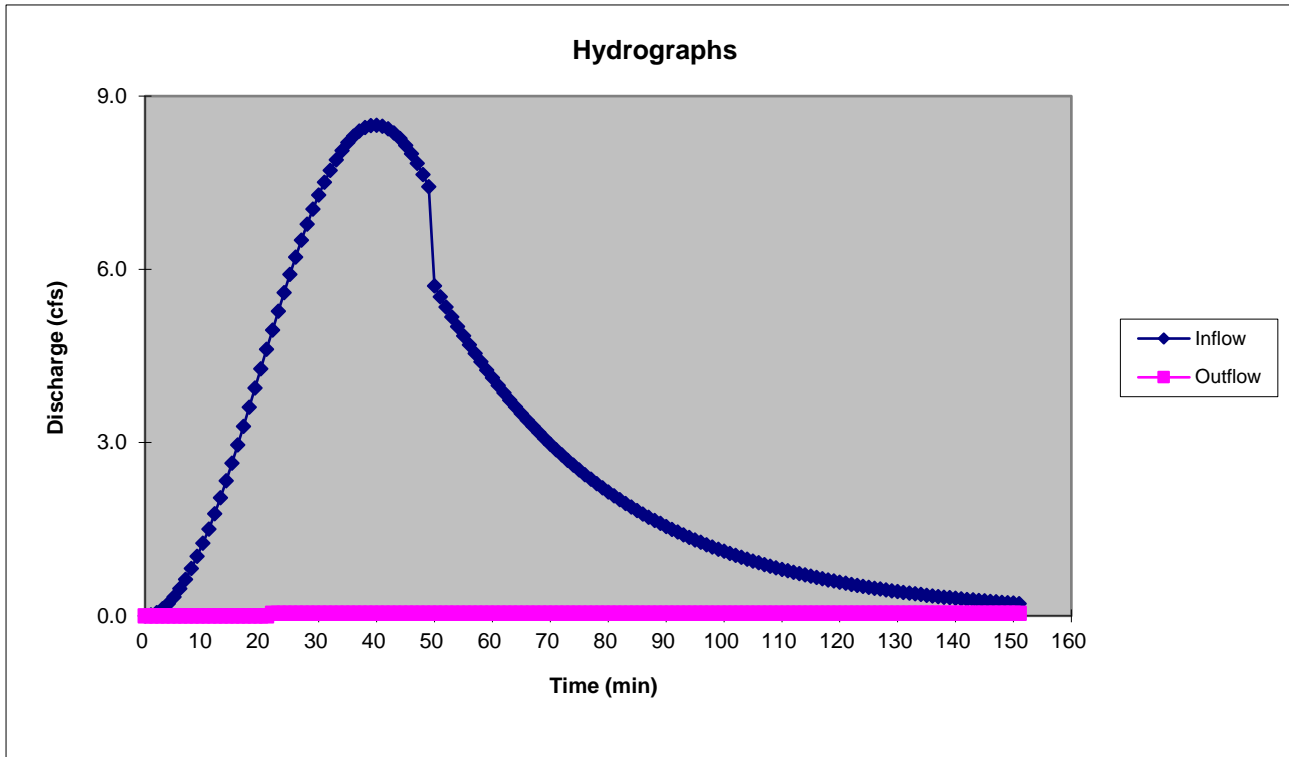
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	311.00	0.0	0.0	0.0		
1	0	0.00	311.00	0.0	0.0	0.00		
2	0	0.79	311.00	0.0	0.0	0.00		
3	0	3.96	311.00	0.0	0.0	0.00		
4	0	11.07	311.00	0.0	0.0	0.00		
5	0	23.66	311.00	0.0	0.0	0.00		
6	0	43.23	311.00	0.0	0.0	0.00		
7	1	71.27	311.01	0.0	0.0	0.00		
8	1	109.16	311.01	0.0	0.0	0.00		
9	1	158.28	311.01	0.0	0.0	0.00		
10	1	219.88	311.02	0.0	0.0	0.00		
11	2	295.19	311.03	0.0	0.0	0.00		
12	2	385.31	311.03	0.0	0.0	0.00		
13	2	491.28	311.04	0.0	0.0	0.00		
14	2	614.00	311.05	0.0	0.0	0.00		
15	3	754.32	311.06	0.0	0.0	0.00		
16	3	912.95	311.07	0.0	0.0	0.00		
17	3	1090.47	311.09	0.0	0.0	0.00		
18	4	1287.37	311.10	0.0	0.0	0.00		
19	4	1504.02	311.12	0.0	0.0	0.00		
20	4	1740.65	311.14	0.0	0.0	0.00		
21	5	1997.38	311.16	0.0	0.0	0.00		
22	5	2274.18	311.18	0.0	0.0	0.04		
23	5	2568.47	311.20	0.0	0.0	0.04		
24	6	2882.44	311.22	0.0	0.0	0.04		
25	6	3215.71	311.24	0.0	0.0	0.04		
26	6	3567.79	311.27	0.0	0.0	0.04		
27	7	3938.04	311.29	0.0	0.0	0.04		
28	7	4325.74	311.32	0.0	0.0	0.04		
29	7	4730.05	311.35	0.0	0.0	0.04		
30	7	5150.01	311.38	0.0	0.0	0.04		
31	8	5584.59	311.41	0.0	0.0	0.04		
32	8	6032.65	311.44	0.0	0.0	0.04		
33	8	6492.98	311.47	0.0	0.0	0.04		
34	8	6964.28	311.50	0.0	0.0	0.04		
35	8	7445.19	311.53	0.0	0.0	0.04		
36	8	7934.29	311.56	0.0	0.0	0.04		
37	8	8430.10	311.60	0.0	0.0	0.04		
38	8	8931.11	311.63	0.0	0.0	0.04		
39	8	9435.77	311.66	0.0	0.0	0.04		
40	8	9942.52	311.69	0.0	0.0	0.04		
41	8	10449.77	311.73	0.0	0.0	0.04		
42	8	10955.94	311.76	0.0	0.0	0.04		
43	8	11459.44	311.79	0.0	0.0	0.04		
44	8	11958.71	311.82	0.0	0.0	0.04		
45	8	12452.22	311.86	0.0	0.0	0.04		
46	8	12938.47	311.89	0.0	0.0	0.04		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	8	13416.01	311.92	0.0	0.0	0.04		
48	8	13883.42	311.95	0.0	0.0	0.04		
49	7	14339.38	311.98	0.0	0.0	0.04		
50	6	14782.61	312.00	0.0	0.0	0.04		
51	6	15122.69	312.03	0.0	0.0	0.04		
52	5	15451.77	312.05	0.0	0.0	0.04		
53	5	15770.19	312.07	0.0	0.0	0.04		
54	5	16078.31	312.09	0.0	0.0	0.04		
55	5	16376.45	312.10	0.0	0.0	0.04		
56	5	16664.94	312.12	0.0	0.0	0.04		
57	5	16944.08	312.14	0.0	0.0	0.04		
58	4	17214.17	312.16	0.0	0.0	0.04		
59	4	17475.51	312.17	0.0	0.0	0.04		
60	4	17728.37	312.19	0.0	0.0	0.04		
61	4	17973.03	312.21	0.0	0.0	0.04		
62	4	18209.76	312.22	0.0	0.0	0.04		
63	4	18438.79	312.23	0.0	0.0	0.04		
64	4	18660.40	312.25	0.0	0.0	0.04		
65	3	18874.80	312.26	0.0	0.0	0.04		
66	3	19082.24	312.27	0.0	0.0	0.04		
67	3	19282.93	312.29	0.0	0.0	0.04		
68	3	19477.10	312.30	0.0	0.0	0.04		
69	3	19664.96	312.31	0.0	0.0	0.04		
70	3	19846.69	312.32	0.0	0.0	0.04		
71	3	20022.52	312.33	0.0	0.0	0.04		
72	3	20192.61	312.34	0.0	0.0	0.04		
73	3	20357.16	312.35	0.0	0.0	0.04		
74	3	20516.35	312.36	0.0	0.0	0.04		
75	3	20670.34	312.37	0.0	0.0	0.04		
76	2	20819.31	312.38	0.0	0.0	0.04		
77	2	20963.41	312.39	0.0	0.0	0.04		
78	2	21102.80	312.40	0.0	0.0	0.04		
79	2	21237.64	312.41	0.0	0.0	0.04		
80	2	21368.07	312.42	0.0	0.0	0.04		
81	2	21494.22	312.42	0.0	0.0	0.04		
82	2	21616.25	312.43	0.0	0.0	0.04		
83	2	21734.28	312.44	0.0	0.0	0.04		
84	2	21848.43	312.45	0.0	0.0	0.04		
85	2	21958.84	312.45	0.0	0.0	0.04		
86	2	22065.63	312.46	0.0	0.0	0.04		
87	2	22168.90	312.47	0.0	0.0	0.04		
88	2	22268.78	312.47	0.0	0.0	0.04		
89	2	22365.37	312.48	0.0	0.0	0.04		
90	2	22458.78	312.48	0.0	0.0	0.04		
91	1	22549.10	312.49	0.0	0.0	0.04		
92	1	22636.45	312.49	0.0	0.0	0.04		
93	1	22720.91	312.50	0.0	0.0	0.04		
94	1	22802.58	312.50	0.0	0.0	0.04		
95	1	22881.55	312.51	0.0	0.0	0.04		
96	1	22957.90	312.51	0.0	0.0	0.04		
97	1	23031.72	312.52	0.0	0.0	0.04		
98	1	23103.09	312.52	0.0	0.0	0.04		
99	1	23172.08	312.53	0.0	0.0	0.04		
100	1	23238.78	312.53	0.0	0.0	0.04		
101	1	23303.26	312.54	0.0	0.0	0.04		
102	1	23365.59	312.54	0.0	0.0	0.04		
103	1	23425.84	312.54	0.0	0.0	0.04		
104	1	23484.07	312.55	0.0	0.0	0.04		
105	1	23540.35	312.55	0.0	0.0	0.04		
106	1	23594.75	312.55	0.0	0.0	0.04		
107	1	23647.32	312.56	0.0	0.0	0.04		
108	1	23698.12	312.56	0.0	0.0	0.04		
109	1	23747.21	312.56	0.0	0.0	0.04		
110	1	23794.64	312.57	0.0	0.0	0.04		
111	1	23840.47	312.57	0.0	0.0	0.04		
112	1	23884.75	312.57	0.0	0.0	0.04		
113	1	23927.53	312.57	0.0	0.0	0.04		
114	1	23968.85	312.58	0.0	0.0	0.04		
115	1	24008.77	312.58	0.0	0.0	0.04		
116	1	24047.32	312.58	0.0	0.0	0.04		
117	1	24084.56	312.58	0.0	0.0	0.04		
118	1	24120.52	312.59	0.0	0.0	0.04		
119	1	24155.25	312.59	0.0	0.0	0.04		
120	1	24188.79	312.59	0.0	0.0	0.04		
121	1	24221.16	312.59	0.0	0.0	0.04		
122	1	24252.42	312.59	0.0	0.0	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	24282.59	312.60	0.0	0.0	0.04		
124	1	24311.72	312.60	0.0	0.0	0.04		
125	0	24339.83	312.60	0.0	0.0	0.04		
126	0	24366.96	312.60	0.0	0.0	0.04		
127	0	24393.13	312.60	0.0	0.0	0.04		
128	0	24418.39	312.60	0.0	0.0	0.04		
129	0	24442.76	312.60	0.0	0.0	0.04		
130	0	24466.26	312.61	0.0	0.0	0.04		
131	0	24488.93	312.61	0.0	0.0	0.04		
132	0	24510.80	312.61	0.0	0.0	0.04		
133	0	24531.88	312.61	0.0	0.0	0.04		
134	0	24552.20	312.61	0.0	0.0	0.04		
135	0	24571.80	312.61	0.0	0.0	0.04		
136	0	24590.68	312.61	0.0	0.0	0.04		
137	0	24608.88	312.62	0.0	0.0	0.04		
138	0	24626.41	312.62	0.0	0.0	0.04		
139	0	24643.30	312.62	0.0	0.0	0.04		
140	0	24659.57	312.62	0.0	0.0	0.04		
141	0	24675.24	312.62	0.0	0.0	0.04		
142	0	24690.33	312.62	0.0	0.0	0.04		
143	0	24704.85	312.62	0.0	0.0	0.04		
144	0	24718.83	312.62	0.0	0.0	0.04		
145	0	24732.27	312.62	0.0	0.0	0.04		
146	0	24745.21	312.62	0.0	0.0	0.04		
147	0	24757.65	312.62	0.0	0.0	0.04		
148	0	24769.61	312.62	0.0	0.0	0.04		
149	0	24781.11	312.63	0.0	0.0	0.04		
150	0	24792.16	312.63	0.0	0.0	0.04		
151	0	24802.78	<b>312.63</b>	0.0	0.0	0.04		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.24	ft	
Peak Stage =	312.76	ft	
Rise =	1.76	ft	
Peak Outflow =	0.04	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	316.00 ft
Qp = 9.75 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.8 min	L = 16 ft	Number = 1.00 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Head = 1.90 inches	
Ks = 14713			
b = 1.07			
Z <sub>0</sub> = 311.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 311.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	311.00	0.0	0.0	0.0		
1	0	0.00	311.00	0.0	0.0	0.00		
2	0	0.91	311.00	0.0	0.0	0.00		
3	0	4.55	311.00	0.0	0.0	0.00		
4	0	12.70	311.00	0.0	0.0	0.00		
5	0	27.16	311.00	0.0	0.0	0.00		
6	1	49.63	311.00	0.0	0.0	0.00		
7	1	81.81	311.01	0.0	0.0	0.00		
8	1	125.32	311.01	0.0	0.0	0.00		
9	1	181.70	311.02	0.0	0.0	0.00		
10	1	252.42	311.02	0.0	0.0	0.00		
11	2	338.87	311.03	0.0	0.0	0.00		
12	2	442.32	311.04	0.0	0.0	0.00		
13	2	563.96	311.05	0.0	0.0	0.00		
14	3	704.85	311.06	0.0	0.0	0.00		
15	3	865.93	311.07	0.0	0.0	0.00		
16	3	1048.03	311.09	0.0	0.0	0.00		
17	4	1251.81	311.10	0.0	0.0	0.00		
18	4	1477.85	311.12	0.0	0.0	0.00		
19	5	1726.56	311.14	0.0	0.0	0.00		
20	5	1998.20	311.16	0.0	0.0	0.00		
21	5	2292.91	311.18	0.0	0.0	0.04		
22	6	2608.20	311.20	0.0	0.0	0.04		
23	6	2946.40	311.22	0.0	0.0	0.04		
24	6	3307.19	311.25	0.0	0.0	0.04		
25	7	3690.14	311.28	0.0	0.0	0.04		
26	7	4094.67	311.30	0.0	0.0	0.04		
27	7	4520.08	311.33	0.0	0.0	0.04		
28	8	4965.51	311.36	0.0	0.0	0.04		
29	8	5430.00	311.39	0.0	0.0	0.04		
30	8	5912.46	311.43	0.0	0.0	0.04		
31	9	6411.71	311.46	0.0	0.0	0.04		
32	9	6926.43	311.50	0.0	0.0	0.04		
33	9	7455.23	311.53	0.0	0.0	0.04		
34	9	7996.63	311.57	0.0	0.0	0.04		
35	9	8549.06	311.60	0.0	0.0	0.04		
36	10	9110.89	311.64	0.0	0.0	0.04		
37	10	9680.43	311.68	0.0	0.0	0.04		
38	10	10255.94	311.71	0.0	0.0	0.04		
39	10	10835.64	311.75	0.0	0.0	0.04		
40	10	11417.73	311.79	0.0	0.0	0.04		
41	10	12000.40	311.83	0.0	0.0	0.04		
42	10	12581.82	311.86	0.0	0.0	0.04		
43	10	13160.18	311.90	0.0	0.0	0.04		
44	9	13733.70	311.94	0.0	0.0	0.04		
45	9	14300.59	311.97	0.0	0.0	0.04		
46	9	14859.15	312.01	0.0	0.0	0.04		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	9	15407.71	312.04	0.0	0.0	0.04		
48	9	15944.64	312.08	0.0	0.0	0.04		
49	9	16468.43	312.11	0.0	0.0	0.04		
50	6	16977.61	312.14	0.0	0.0	0.04		
51	6	17317.69	312.16	0.0	0.0	0.04		
52	5	17646.77	312.18	0.0	0.0	0.04		
53	5	17965.19	312.20	0.0	0.0	0.04		
54	5	18273.31	312.22	0.0	0.0	0.04		
55	5	18571.45	312.24	0.0	0.0	0.04		
56	5	18859.94	312.26	0.0	0.0	0.04		
57	5	19139.08	312.28	0.0	0.0	0.04		
58	4	19409.17	312.29	0.0	0.0	0.04		
59	4	19670.51	312.31	0.0	0.0	0.04		
60	4	19923.37	312.33	0.0	0.0	0.04		
61	4	20168.03	312.34	0.0	0.0	0.04		
62	4	20404.75	312.36	0.0	0.0	0.04		
63	4	20633.79	312.37	0.0	0.0	0.04		
64	4	20855.40	312.38	0.0	0.0	0.04		
65	3	21069.80	312.40	0.0	0.0	0.04		
66	3	21277.24	312.41	0.0	0.0	0.04		
67	3	21477.93	312.42	0.0	0.0	0.04		
68	3	21672.10	312.43	0.0	0.0	0.04		
69	3	21859.96	312.45	0.0	0.0	0.04		
70	3	22041.69	312.46	0.0	0.0	0.04		
71	3	22217.52	312.47	0.0	0.0	0.04		
72	3	22387.61	312.48	0.0	0.0	0.04		
73	3	22552.16	312.49	0.0	0.0	0.04		
74	3	22711.35	312.50	0.0	0.0	0.04		
75	3	22865.34	312.51	0.0	0.0	0.04		
76	2	23014.31	312.52	0.0	0.0	0.04		
77	2	23158.41	312.53	0.0	0.0	0.04		
78	2	23297.80	312.53	0.0	0.0	0.04		
79	2	23432.64	312.54	0.0	0.0	0.04		
80	2	23563.07	312.55	0.0	0.0	0.04		
81	2	23689.22	312.56	0.0	0.0	0.04		
82	2	23811.25	312.57	0.0	0.0	0.04		
83	2	23929.28	312.57	0.0	0.0	0.04		
84	2	24043.43	312.58	0.0	0.0	0.04		
85	2	24153.84	312.59	0.0	0.0	0.04		
86	2	24260.63	312.59	0.0	0.0	0.04		
87	2	24363.90	312.60	0.0	0.0	0.04		
88	2	24463.78	312.61	0.0	0.0	0.04		
89	2	24560.37	312.61	0.0	0.0	0.04		
90	2	24653.77	312.62	0.0	0.0	0.04		
91	1	24744.10	312.62	0.0	0.0	0.04		
92	1	24831.45	312.63	0.0	0.0	0.04		
93	1	24915.91	312.63	0.0	0.0	0.04		
94	1	24997.58	312.64	0.0	0.0	0.04		
95	1	25076.55	312.64	0.0	0.0	0.04		
96	1	25152.90	312.65	0.0	0.0	0.04		
97	1	25226.72	312.65	0.0	0.0	0.04		
98	1	25298.09	312.66	0.0	0.0	0.04		
99	1	25367.08	312.66	0.0	0.0	0.04		
100	1	25433.78	312.67	0.0	0.0	0.04		
101	1	25498.26	312.67	0.0	0.0	0.04		
102	1	25560.59	312.67	0.0	0.0	0.04		
103	1	25620.84	312.68	0.0	0.0	0.04		
104	1	25679.07	312.68	0.0	0.0	0.04		
105	1	25735.35	312.68	0.0	0.0	0.04		
106	1	25789.75	312.69	0.0	0.0	0.04		
107	1	25842.32	312.69	0.0	0.0	0.04		
108	1	25893.12	312.69	0.0	0.0	0.04		
109	1	25942.21	312.70	0.0	0.0	0.04		
110	1	25989.64	312.70	0.0	0.0	0.04		
111	1	26035.47	312.70	0.0	0.0	0.04		
112	1	26079.75	312.70	0.0	0.0	0.04		
113	1	26122.53	312.71	0.0	0.0	0.04		
114	1	26163.85	312.71	0.0	0.0	0.04		
115	1	26203.77	312.71	0.0	0.0	0.04		
116	1	26242.32	312.71	0.0	0.0	0.04		
117	1	26279.56	312.72	0.0	0.0	0.04		
118	1	26315.52	312.72	0.0	0.0	0.04		
119	1	26350.25	312.72	0.0	0.0	0.04		
120	1	26383.78	312.72	0.0	0.0	0.04		
121	1	26416.16	312.73	0.0	0.0	0.04		
122	1	26447.42	312.73	0.0	0.0	0.04		

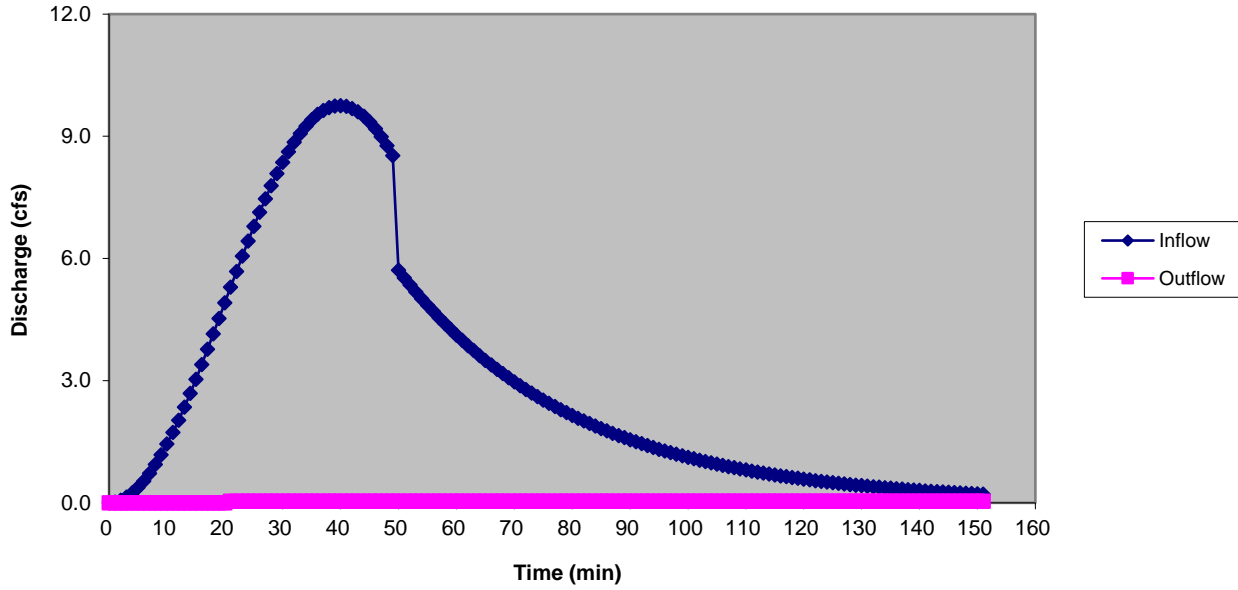
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	26477.59	312.73	0.0	0.0	0.04		
124	1	26506.72	312.73	0.0	0.0	0.04		
125	0	26534.83	312.73	0.0	0.0	0.04		
126	0	26561.96	312.73	0.0	0.0	0.04		
127	0	26588.13	312.74	0.0	0.0	0.04		
128	0	26613.39	312.74	0.0	0.0	0.04		
129	0	26637.76	312.74	0.0	0.0	0.04		
130	0	26661.26	312.74	0.0	0.0	0.04		
131	0	26683.93	312.74	0.0	0.0	0.04		
132	0	26705.80	312.74	0.0	0.0	0.04		
133	0	26726.88	312.74	0.0	0.0	0.04		
134	0	26747.20	312.75	0.0	0.0	0.04		
135	0	26766.80	312.75	0.0	0.0	0.04		
136	0	26785.68	312.75	0.0	0.0	0.04		
137	0	26803.88	312.75	0.0	0.0	0.04		
138	0	26821.41	312.75	0.0	0.0	0.04		
139	0	26838.30	312.75	0.0	0.0	0.04		
140	0	26854.57	312.75	0.0	0.0	0.04		
141	0	26870.24	312.75	0.0	0.0	0.04		
142	0	26885.33	312.75	0.0	0.0	0.04		
143	0	26899.85	312.75	0.0	0.0	0.04		
144	0	26913.82	312.76	0.0	0.0	0.04		
145	0	26927.27	312.76	0.0	0.0	0.04		
146	0	26940.21	312.76	0.0	0.0	0.04		
147	0	26952.65	312.76	0.0	0.0	0.04		
148	0	26964.61	312.76	0.0	0.0	0.04		
149	0	26976.11	312.76	0.0	0.0	0.04		
150	0	26987.16	312.76	0.0	0.0	0.04		
151	0	26997.78	312.76	0.0	0.0	0.04		



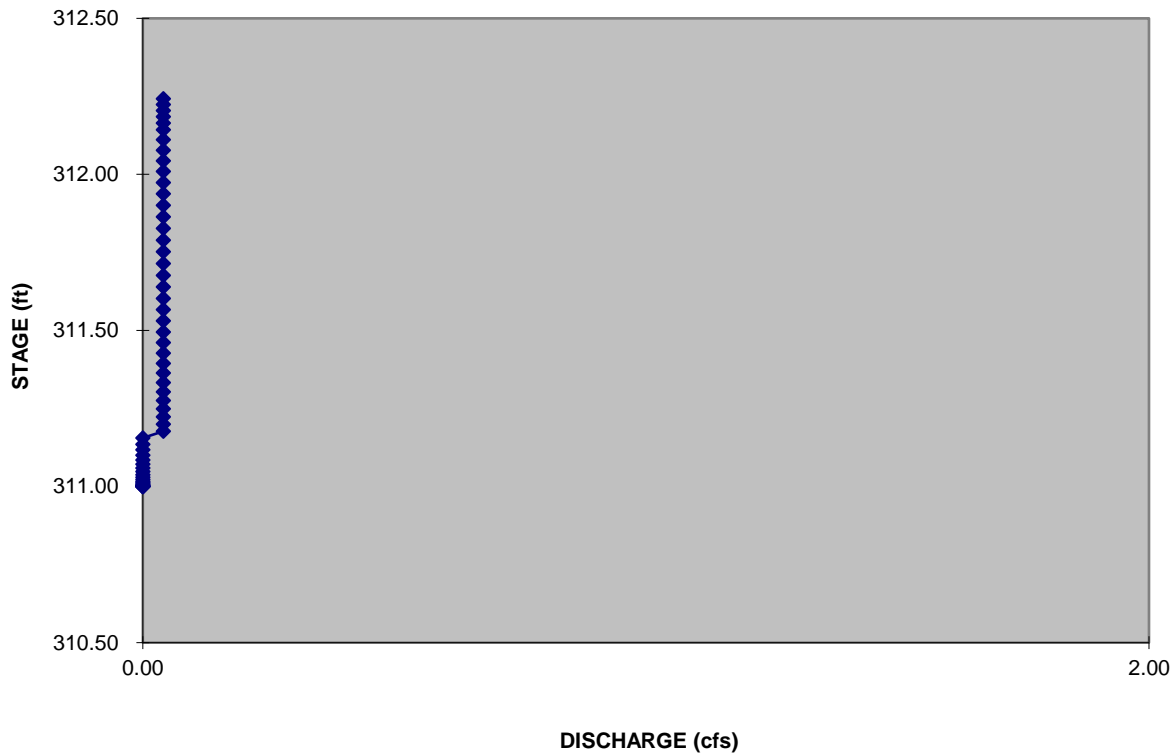
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM-I  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID: SCM-I

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	5.46 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	3.28 ac.	0.25
Undisturbed grassy area	0.00 ac.	2.18 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>5.46 ac.</b>	<b>5.46 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.29</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	1047 feet		
Height of watershed =	19 feet		
Calculated t(c) =	7.7 minutes		
Minimum t(c) =	5.0 minutes		
Time of concentration =	7.7 minutes		

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	5.13 in/hr
10	195	22	6.56 in/hr
25	232	23	7.55 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.13 in/hr	12.6 cfs	8.12 cfs
10-year storm	6.56 in/hr	16.1 cfs	10.38 cfs
25-year storm	7.55 in/hr	18.5 cfs	11.95 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	163
Bottom width =	68
Sediment depth =	2.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	2.0
Side slopes =	3.0 H:1V
Spillway length =	20
Height of berm =	4
Top of trap length =	187
Top of trap width =	92
Bottom elevation =	310

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	19656 cu. ft.	25030 cu. ft.
Sediment surface area =	7010 sq. ft.	14000 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	2
Trap bottom length to width ratio =	2.0L:1W (min)	2.4L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.38 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.62 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 19,656 cubic feet

24 HOURS	Best Option =	1 - 4 inch Skimmers with a 4 inch orifice
2 DAYS	Best Option =	1 - 4 inch Skimmers with a 2.8 inch orifice
3 DAYS	Best Option =	1 - 3 inch Skimmers with a 2.5 inch orifice
4 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.2 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

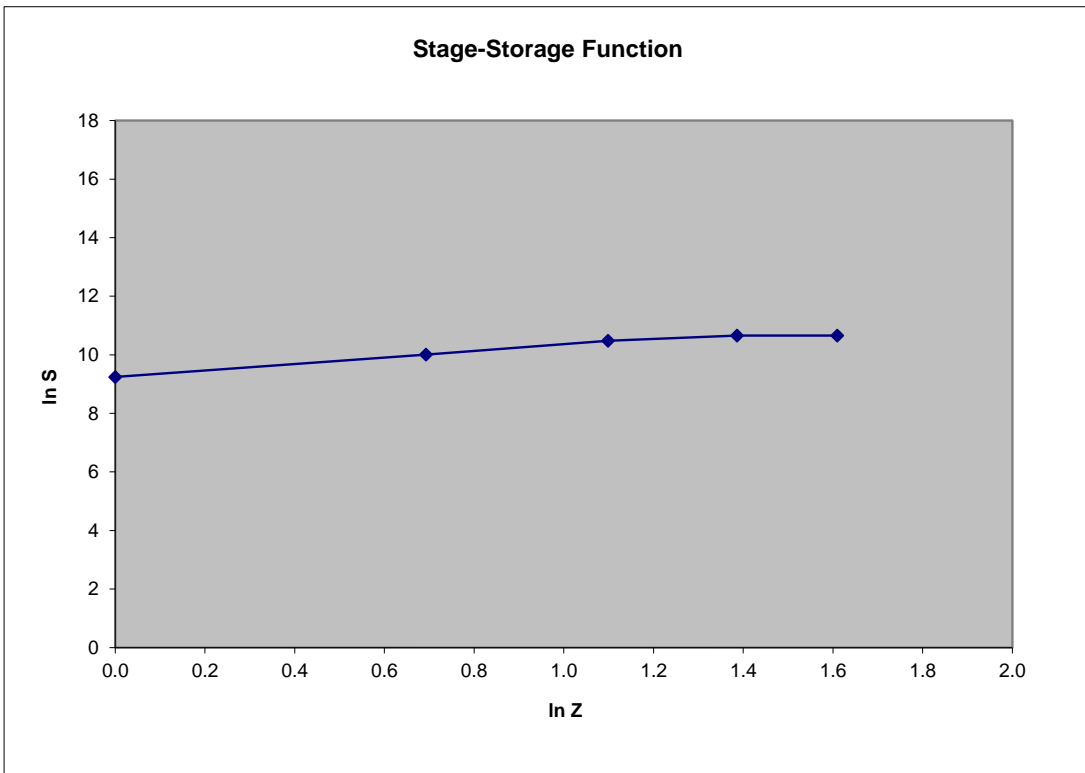
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
310	9626		0	0			
311	11047	10337	10336.5	1	9.2434	0.0000	0.92
312	12540	11793.5	22130	2	10.0047	0.6931	2.09
313	14103	13321.5	35451.5	3	10.4759	1.0986	3.48
314	0	7051.5	42503	4	10.6573	1.3863	4.23
315	0	0	42503	5	10.6573	1.6094	4.23

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 0.93$  and  $K_s = 11140$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT	OUTPUT
Drainage Area, DA = <span style="border: 1px solid gray; padding: 2px;">5.46</span> ac Disturbed Area = <span style="border: 1px solid gray; padding: 2px;">5.46</span> ac Undisturbed Woods = <span style="border: 1px solid gray; padding: 2px;">0.00</span> ac Undisturbed Grass = <span style="border: 1px solid gray; padding: 2px;">0.00</span> ac Hydraulic Length = <span style="border: 1px solid gray; padding: 2px;">1047</span> ft Vertical Fall = <span style="border: 1px solid gray; padding: 2px;">19</span> ft For 1 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">132</span> h = <span style="border: 1px solid gray; padding: 2px;">18</span> For 10 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">195</span> h = <span style="border: 1px solid gray; padding: 2px;">22</span> For 25 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">232</span> h = <span style="border: 1px solid gray; padding: 2px;">23</span>	<b>Use Kirpich Equation:</b> Tc = <span style="border: 1px solid gray; padding: 2px;">7.7 min</span>  <b>Use Malcom Method:</b> i <sub>1</sub> = <span style="border: 1px solid gray; padding: 2px;">5.13 in/hr</span> i <sub>10</sub> = <span style="border: 1px solid gray; padding: 2px;">6.56 in/hr</span> i <sub>25</sub> = <span style="border: 1px solid gray; padding: 2px;">7.55 in/hr</span>  <b>Use Rational Method:</b> Q <sub>2</sub> = <span style="border: 1px solid gray; padding: 2px;">12.6 cfs</span> Q <sub>10</sub> = <span style="border: 1px solid gray; padding: 2px;">16.1 cfs</span> Q <sub>25</sub> = <span style="border: 1px solid gray; padding: 2px;">18.5 cfs</span>
"C" CALCULATION - See Sediment Trap Sizing  <b>"C" = <span style="border-bottom: 3px double orange; padding: 0 20px;">0.45</span></b>	

**SOLUTION - Tp:**

INPUT	OUTPUT
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">2.7</span> in (2 Year Storm) CN = <span style="border: 1px solid gray; padding: 2px;">91</span> (newly graded areas)	S = <span style="border: 1px solid gray; padding: 2px;">0.99</span> in Q* = <span style="border: 1px solid gray; padding: 2px;">1.75</span> in  T <sub>p2</sub> = <span style="border: 1px solid gray; padding: 2px;">33.0</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">3.9</span> in (10 Year Storm)	Q* = <span style="border: 1px solid gray; padding: 2px;">2.92</span> in  T <sub>p10</sub> = <span style="border: 1px solid gray; padding: 2px;">43.1</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">4.6</span> in (25 Year Storm)	Q* = <span style="border: 1px solid gray; padding: 2px;">3.59</span> in  T <sub>p25</sub> = <span style="border: 1px solid gray; padding: 2px;">46.1</span> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	312.21	ft
	Rise =	2.21	ft
Freeboard =		1.79	ft
	Peak Outflow =	6.02	cfs

INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 314.00 ft
Qp =	12.60	cfs	N =
	33.0	min	L =
	1.0	min	Cw =
<b>Stage-Storage Results:</b>		Zcr =	312.00
Ks =	11140		
b =	0.93		
Z <sub>0</sub> =	310.0	ft (inv)	
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	310.00	ft	
			<b>Skimmer Orifice:</b>
			Number =
			1.00
			Ea
			Diameter =
			3.00
			Inches
			Head =
			2.46
			inches

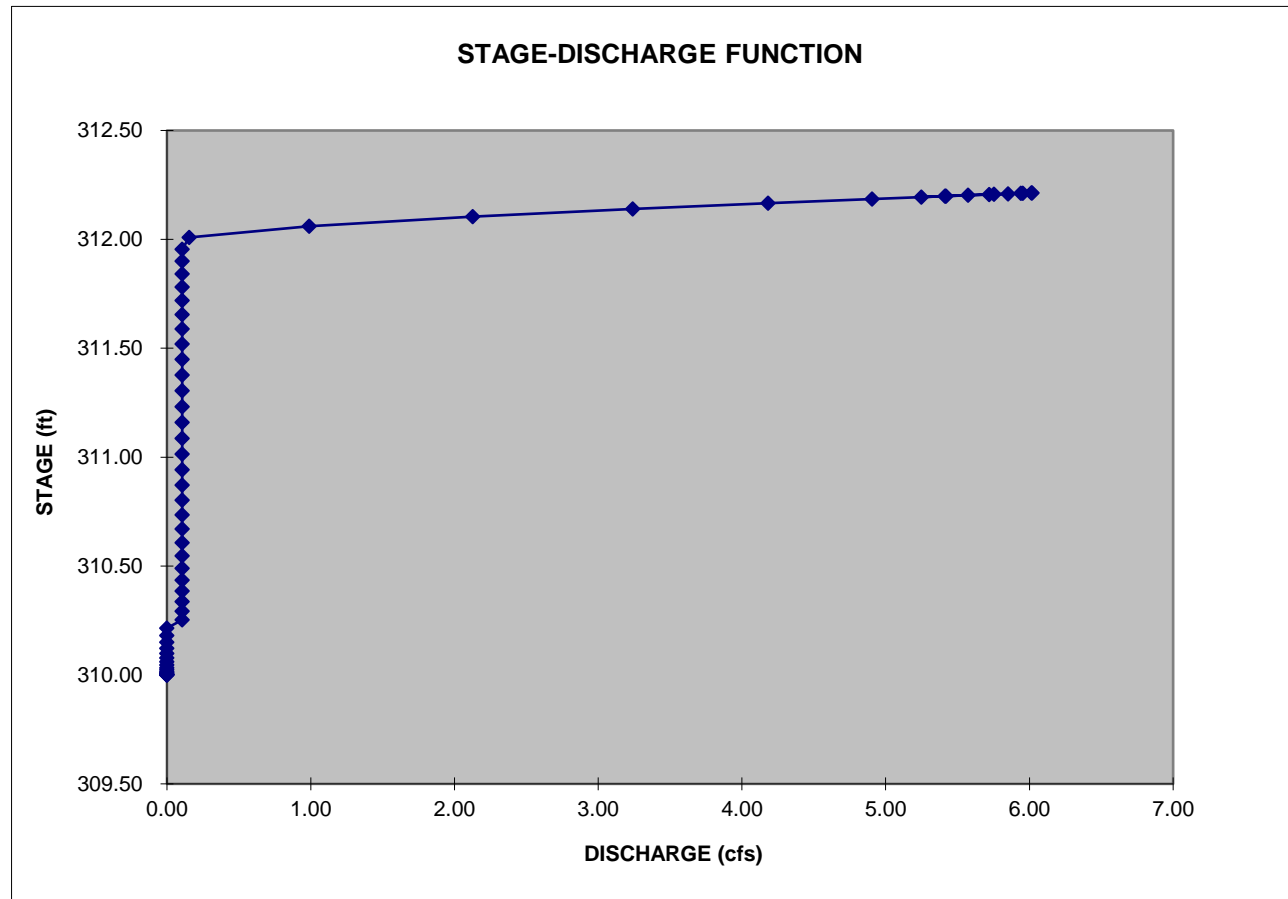
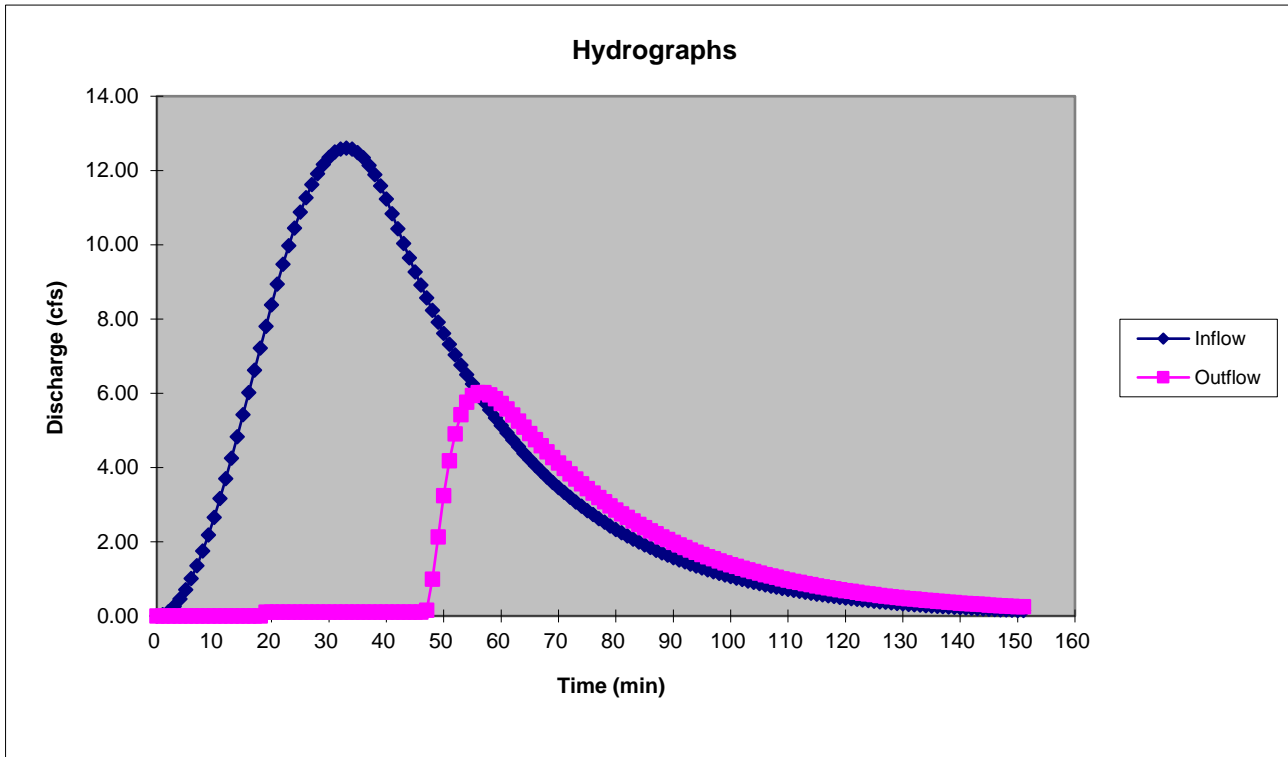
OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	1.72	310.00	0.0	0.0	0.00		
3	0	8.57	310.00	0.0	0.0	0.00		
4	0	23.93	310.00	0.0	0.0	0.00		
5	1	51.08	310.00	0.0	0.0	0.00		
6	1	93.23	310.01	0.0	0.0	0.00		
7	1	153.41	310.01	0.0	0.0	0.00		
8	2	234.52	310.02	0.0	0.0	0.00		
9	2	339.25	310.02	0.0	0.0	0.00		
10	3	470.08	310.03	0.0	0.0	0.00		
11	3	629.27	310.05	0.0	0.0	0.00		
12	4	818.79	310.06	0.0	0.0	0.00		
13	4	1040.36	310.08	0.0	0.0	0.00		
14	5	1295.40	310.10	0.0	0.0	0.00		
15	5	1585.03	310.12	0.0	0.0	0.00		
16	6	1910.06	310.15	0.0	0.0	0.00		
17	7	2270.95	310.18	0.0	0.0	0.00		
18	7	2667.88	310.21	0.0	0.0	0.00		
19	8	3100.66	310.25	0.1	0.0	0.11		
20	8	3562.49	310.29	0.1	0.0	0.11		
21	9	4058.87	310.34	0.1	0.0	0.11		
22	9	4588.65	310.38	0.1	0.0	0.11		
23	10	5150.42	310.44	0.1	0.0	0.11		
24	10	5742.43	310.49	0.1	0.0	0.11		
25	11	6362.69	310.55	0.1	0.0	0.11		
26	11	7008.94	310.61	0.1	0.0	0.11		
27	12	7678.70	310.67	0.1	0.0	0.11		
28	12	8369.24	310.74	0.1	0.0	0.11		
29	12	9077.69	310.80	0.1	0.0	0.11		
30	12	9800.97	310.87	0.1	0.0	0.11		
31	12	10535.90	310.94	0.1	0.0	0.11		
32	13	11279.18	311.01	0.1	0.0	0.11		
33	13	12027.43	311.09	0.1	0.0	0.11		
34	13	12777.24	311.16	0.1	0.0	0.11		
35	12	13525.17	311.23	0.1	0.0	0.11		
36	12	14267.80	311.31	0.1	0.0	0.11		
37	12	15001.77	311.38	0.1	0.0	0.11		
38	12	15723.79	311.45	0.1	0.0	0.11		
39	12	16430.68	311.52	0.1	0.0	0.11		
40	11	17119.38	311.59	0.1	0.0	0.11		





# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.62	ft	
Peak Stage =	312.38	ft	
Rise =	2.38	ft	
Peak Outflow =	14.16	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 314.00 ft	
Qp = 16.11 cfs	N = 1		
Tp = 43.1 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 312.00 ft	<b>Skimmer Orifice:</b>	
Ks = 11140		Number = 1	Ea
b = 0.93		Diameter = 3.00	Inches
Z <sub>0</sub> = 310.0 ft (inv)		Head = 2.46	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

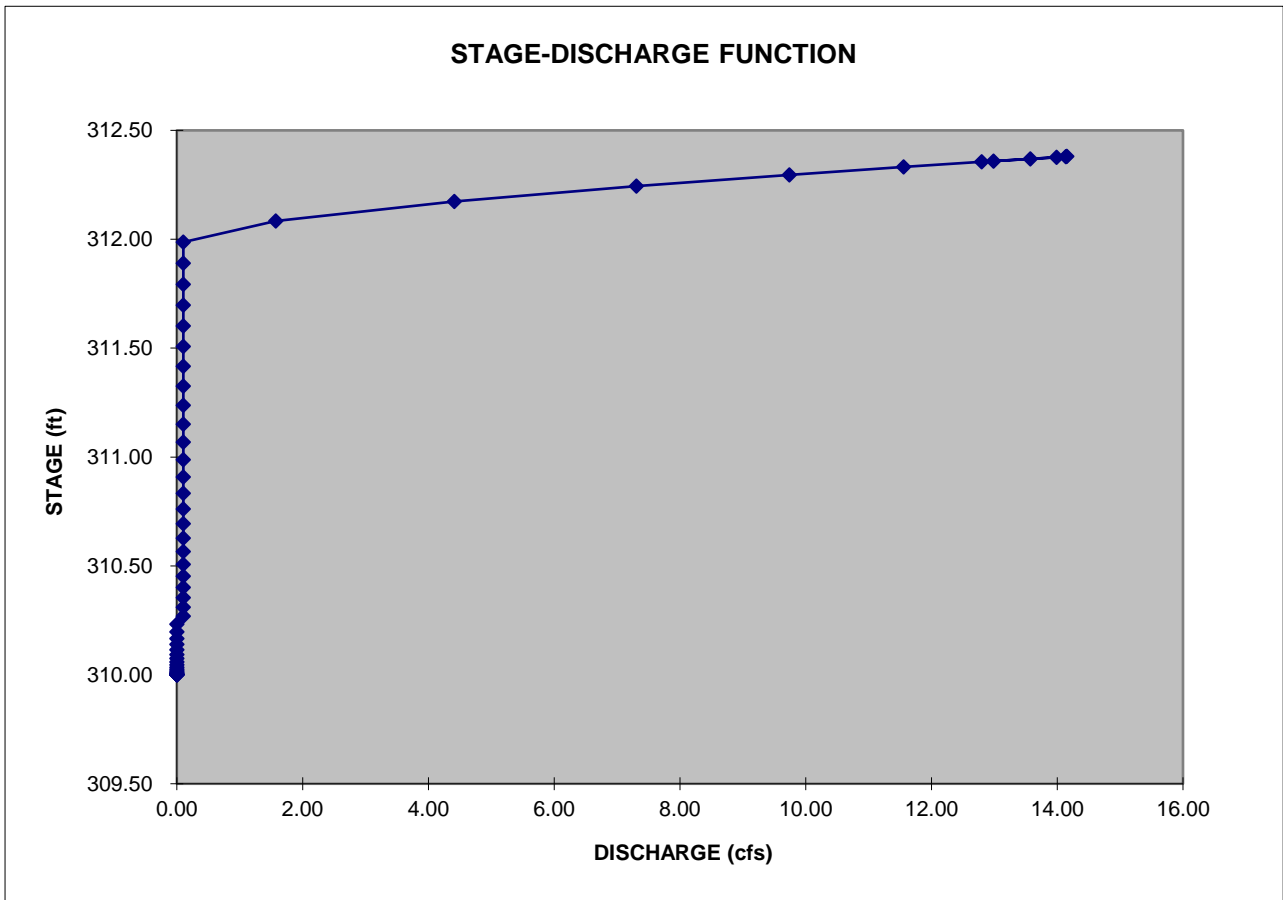
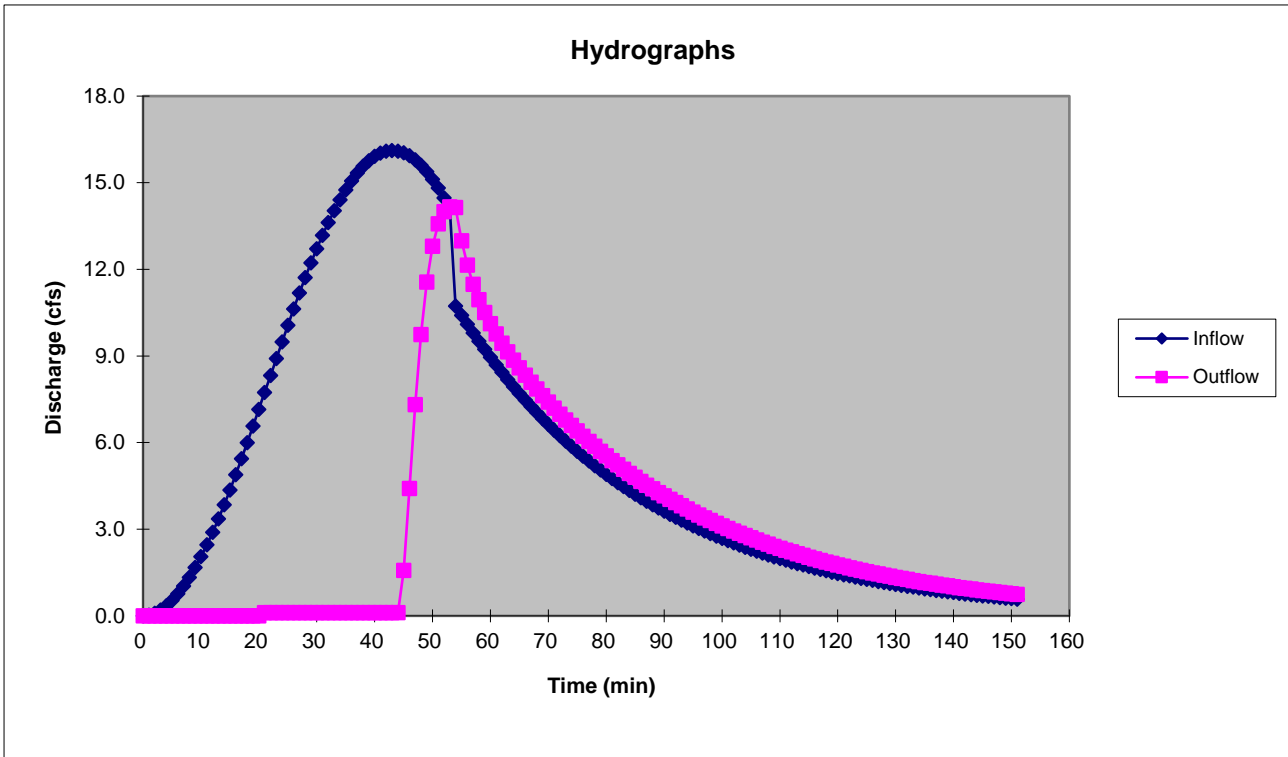
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	1.28	310.00	0.0	0.0	0.00		
3	0	6.41	310.00	0.0	0.0	0.00		
4	0	17.93	310.00	0.0	0.0	0.00		
5	1	38.33	310.00	0.0	0.0	0.00		
6	1	70.09	310.00	0.0	0.0	0.00		
7	1	115.60	310.01	0.0	0.0	0.00		
8	1	177.19	310.01	0.0	0.0	0.00		
9	2	257.09	310.02	0.0	0.0	0.00		
10	2	357.46	310.02	0.0	0.0	0.00		
11	2	480.32	310.03	0.0	0.0	0.00		
12	3	627.59	310.05	0.0	0.0	0.00		
13	3	801.06	310.06	0.0	0.0	0.00		
14	4	1002.37	310.07	0.0	0.0	0.00		
15	4	1233.02	310.09	0.0	0.0	0.00		
16	5	1494.36	310.12	0.0	0.0	0.00		
17	5	1787.55	310.14	0.0	0.0	0.00		
18	6	2113.63	310.17	0.0	0.0	0.00		
19	7	2473.41	310.20	0.0	0.0	0.00		
20	7	2867.55	310.23	0.0	0.0	0.00		
21	8	3296.54	310.27	0.1	0.0	0.11		
22	8	3754.35	310.31	0.1	0.0	0.11		
23	9	4247.38	310.35	0.1	0.0	0.11		
24	9	4775.56	310.40	0.1	0.0	0.11		
25	10	5338.62	310.45	0.1	0.0	0.11		
26	11	5936.10	310.51	0.1	0.0	0.11		
27	11	6567.35	310.57	0.1	0.0	0.11		
28	12	7231.56	310.63	0.1	0.0	0.11		
29	12	7927.73	310.69	0.1	0.0	0.11		
30	13	8654.71	310.76	0.1	0.0	0.11		
31	13	9411.16	310.83	0.1	0.0	0.11		
32	14	10195.60	310.91	0.1	0.0	0.11		
33	14	11006.39	310.99	0.1	0.0	0.11		
34	14	11841.77	311.07	0.1	0.0	0.11		
35	15	12699.82	311.15	0.1	0.0	0.11		
36	15	13578.53	311.24	0.1	0.0	0.11		
37	15	14475.77	311.33	0.1	0.0	0.11		
38	16	15389.29	311.42	0.1	0.0	0.11		
39	16	16316.78	311.51	0.1	0.0	0.11		
40	16	17255.85	311.60	0.1	0.0	0.11		
41	16	18204.05	311.70	0.1	0.0	0.11		
42	16	19158.86	311.79	0.1	0.0	0.11		
43	16	20117.76	311.89	0.1	0.0	0.11		
44	16	21078.18	311.99	0.1	0.0	0.11		
45	16	22037.56	312.08	1.6	1.5	0.11		
46	16	22905.28	312.17	4.4	4.3	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	16	23596.46	312.24	7.3	7.2	0.11		
48	16	24105.15	312.30	9.7	9.6	0.11		
49	15	24456.72	312.33	11.6	11.5	0.11		
50	15	24685.74	312.36	12.8	12.7	0.11		
51	15	24824.52	312.37	13.6	13.5	0.11		
52	14	24898.85	312.38	14.0	13.9	0.11		
53	14	24927.73	312.38	14.2	14.1	0.11		
54	11	24924.44	312.38	14.1	14.0	0.11		
55	10	24719.79	312.36	13.0	12.9	0.11		
56	10	24564.99	312.34	12.1	12.0	0.11		
57	10	24442.46	312.33	11.5	11.4	0.11		
58	10	24341.31	312.32	11.0	10.8	0.11		
59	9	24254.64	312.31	10.5	10.4	0.11		
60	9	24177.97	312.30	10.1	10.0	0.11		
61	9	24108.37	312.30	9.8	9.7	0.11		
62	8	24043.89	312.29	9.4	9.3	0.11		
63	8	23983.24	312.28	9.1	9.0	0.11		
64	8	23925.51	312.28	8.9	8.8	0.11		
65	8	23870.11	312.27	8.6	8.5	0.11		
66	7	23816.61	312.27	8.3	8.2	0.11		
67	7	23764.73	312.26	8.1	8.0	0.11		
68	7	23714.25	312.26	7.9	7.7	0.11		
69	7	23665.03	312.25	7.6	7.5	0.11		
70	7	23616.95	312.25	7.4	7.3	0.11		
71	6	23569.94	312.24	7.2	7.1	0.11		
72	6	23523.94	312.24	7.0	6.9	0.11		
73	6	23478.90	312.23	6.8	6.7	0.11		
74	6	23434.77	312.23	6.6	6.5	0.11		
75	6	23391.53	312.22	6.4	6.3	0.11		
76	6	23349.15	312.22	6.2	6.1	0.11		
77	5	23307.60	312.21	6.0	5.9	0.11		
78	5	23266.87	312.21	5.9	5.8	0.11		
79	5	23226.93	312.21	5.7	5.6	0.11		
80	5	23187.76	312.20	5.5	5.4	0.11		
81	5	23149.35	312.20	5.4	5.3	0.11		
82	5	23111.68	312.19	5.2	5.1	0.11		
83	4	23074.74	312.19	5.1	5.0	0.11		
84	4	23038.51	312.19	4.9	4.8	0.11		
85	4	23002.98	312.18	4.8	4.7	0.11		
86	4	22968.13	312.18	4.7	4.5	0.11		
87	4	22933.95	312.18	4.5	4.4	0.11		
88	4	22900.42	312.17	4.4	4.3	0.11		
89	4	22867.54	312.17	4.3	4.2	0.11		
90	4	22835.29	312.17	4.1	4.0	0.11		
91	4	22803.65	312.16	4.0	3.9	0.11		
92	3	22772.63	312.16	3.9	3.8	0.11		
93	3	22742.19	312.16	3.8	3.7	0.11		
94	3	22712.34	312.15	3.7	3.6	0.11		
95	3	22683.05	312.15	3.6	3.5	0.11		
96	3	22654.33	312.15	3.5	3.4	0.11		
97	3	22626.15	312.14	3.4	3.3	0.11		
98	3	22598.52	312.14	3.3	3.2	0.11		
99	3	22571.40	312.14	3.2	3.1	0.11		
100	3	22544.81	312.14	3.1	3.0	0.11		
101	3	22518.72	312.13	3.0	2.9	0.11		
102	3	22493.13	312.13	2.9	2.8	0.11		
103	2	22468.03	312.13	2.9	2.8	0.11		
104	2	22443.40	312.13	2.8	2.7	0.11		
105	2	22419.24	312.12	2.7	2.6	0.11		
106	2	22395.54	312.12	2.6	2.5	0.11		
107	2	22372.29	312.12	2.5	2.4	0.11		
108	2	22349.48	312.12	2.5	2.4	0.11		
109	2	22327.10	312.11	2.4	2.3	0.11		
110	2	22305.15	312.11	2.3	2.2	0.11		
111	2	22283.61	312.11	2.3	2.2	0.11		
112	2	22262.47	312.11	2.2	2.1	0.11		
113	2	22241.74	312.11	2.1	2.0	0.11		
114	2	22221.40	312.10	2.1	2.0	0.11		
115	2	22201.44	312.10	2.0	1.9	0.11		
116	2	22181.86	312.10	2.0	1.9	0.11		
117	2	22162.65	312.10	1.9	1.8	0.11		
118	2	22143.79	312.10	1.9	1.8	0.11		
119	2	22125.29	312.09	1.8	1.7	0.11		
120	1	22107.14	312.09	1.8	1.7	0.11		
121	1	22089.33	312.09	1.7	1.6	0.11		
122	1	22071.86	312.09	1.7	1.6	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	22054.71	312.09	1.6	1.5	0.11		
124	1	22037.88	312.08	1.6	1.5	0.11		
125	1	22021.36	312.08	1.5	1.4	0.11		
126	1	22005.16	312.08	1.5	1.4	0.11		
127	1	21989.25	312.08	1.4	1.3	0.11		
128	1	21973.64	312.08	1.4	1.3	0.11		
129	1	21958.32	312.08	1.4	1.3	0.11		
130	1	21943.29	312.07	1.3	1.2	0.11		
131	1	21928.53	312.07	1.3	1.2	0.11		
132	1	21914.05	312.07	1.3	1.2	0.11		
133	1	21899.83	312.07	1.2	1.1	0.11		
134	1	21885.88	312.07	1.2	1.1	0.11		
135	1	21872.19	312.07	1.2	1.1	0.11		
136	1	21858.74	312.07	1.1	1.0	0.11		
137	1	21845.55	312.06	1.1	1.0	0.11		
138	1	21832.59	312.06	1.1	1.0	0.11		
139	1	21819.88	312.06	1.0	0.9	0.11		
140	1	21807.39	312.06	1.0	0.9	0.11		
141	1	21795.14	312.06	1.0	0.9	0.11		
142	1	21783.11	312.06	1.0	0.8	0.11		
143	1	21771.29	312.06	0.9	0.8	0.11		
144	1	21759.69	312.06	0.9	0.8	0.11		
145	1	21748.31	312.05	0.9	0.8	0.11		
146	1	21737.12	312.05	0.9	0.7	0.11		
147	1	21726.14	312.05	0.8	0.7	0.11		
148	1	21715.36	312.05	0.8	0.7	0.11		
149	1	21704.77	312.05	0.8	0.7	0.11		
150	1	21694.37	312.05	0.8	0.7	0.11		
151	1	21684.16	312.05	0.7	0.6	0.11		

# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.57	ft	
Peak Stage =	312.43	ft	
Rise =	2.43	ft	
Peak Outflow =	17.30	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 314.00 ft	
Qp = 18.55 cfs	N = 1		
Tp = 43.1 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 312.00 ft	<b>Skimmer Orifice:</b>	
Ks = 11140		Number = 1.00	Ea
b = 0.93		Diameter = 3.00	Inches
Z <sub>0</sub> = 310.0 ft (inv)		Head = 2.46	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	1.48	310.00	0.0	0.0	0.00		
3	0	7.38	310.00	0.0	0.0	0.00		
4	0	20.63	310.00	0.0	0.0	0.00		
5	1	44.12	310.00	0.0	0.0	0.00		
6	1	80.68	310.00	0.0	0.0	0.00		
7	1	133.06	310.01	0.0	0.0	0.00		
8	2	203.95	310.01	0.0	0.0	0.00		
9	2	295.92	310.02	0.0	0.0	0.00		
10	2	411.45	310.03	0.0	0.0	0.00		
11	3	552.86	310.04	0.0	0.0	0.00		
12	3	722.38	310.05	0.0	0.0	0.00		
13	4	922.05	310.07	0.0	0.0	0.00		
14	4	1153.76	310.09	0.0	0.0	0.00		
15	5	1419.25	310.11	0.0	0.0	0.00		
16	6	1720.06	310.13	0.0	0.0	0.00		
17	6	2057.54	310.16	0.0	0.0	0.00		
18	7	2432.86	310.19	0.0	0.0	0.00		
19	8	2846.98	310.23	0.0	0.0	0.00		
20	8	3300.66	310.27	0.1	0.0	0.11		
21	9	3788.12	310.31	0.1	0.0	0.11		
22	10	4316.03	310.36	0.1	0.0	0.11		
23	10	4884.48	310.41	0.1	0.0	0.11		
24	11	5493.39	310.47	0.1	0.0	0.11		
25	12	6142.44	310.53	0.1	0.0	0.11		
26	12	6831.11	310.59	0.1	0.0	0.11		
27	13	7558.66	310.66	0.1	0.0	0.11		
28	13	8324.14	310.73	0.1	0.0	0.11		
29	14	9126.41	310.81	0.1	0.0	0.11		
30	15	9964.14	310.89	0.1	0.0	0.11		
31	15	10835.79	310.97	0.1	0.0	0.11		
32	16	11739.66	311.06	0.1	0.0	0.11		
33	16	12673.87	311.15	0.1	0.0	0.11		
34	17	13636.37	311.24	0.1	0.0	0.11		
35	17	14624.97	311.34	0.1	0.0	0.11		
36	17	15637.35	311.44	0.1	0.0	0.11		
37	18	16671.05	311.54	0.1	0.0	0.11		
38	18	17723.50	311.65	0.1	0.0	0.11		
39	18	18792.03	311.76	0.1	0.0	0.11		
40	18	19873.89	311.86	0.1	0.0	0.11		
41	18	20966.24	311.98	0.1	0.0	0.11		
42	19	22066.22	312.09	1.6	1.5	0.11		
43	19	23078.24	312.19	5.1	5.0	0.11		
44	19	23885.58	312.27	8.7	8.6	0.11		
45	18	24477.32	312.33	11.7	11.6	0.11		
46	18	24884.54	312.38	13.9	13.8	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	18	25150.22	312.40	15.4	15.3	0.11		
48	18	25313.94	312.42	16.4	16.3	0.11		
49	18	25406.53	312.43	17.0	16.9	0.11		
50	17	25450.03	312.43	17.2	17.1	0.11		
51	17	25459.39	312.43	17.3	17.2	0.11		
52	17	25444.47	312.43	17.2	17.1	0.11		
53	16	25411.69	312.43	17.0	16.9	0.11		
54	11	25365.19	312.42	16.7	16.6	0.11		
55	10	25005.32	312.39	14.6	14.5	0.11		
56	10	24753.74	312.36	13.2	13.1	0.11		
57	10	24569.08	312.34	12.2	12.1	0.11		
58	10	24427.22	312.33	11.4	11.3	0.11		
59	9	24313.45	312.32	10.8	10.7	0.11		
60	9	24218.53	312.31	10.3	10.2	0.11		
61	9	24136.53	312.30	9.9	9.8	0.11		
62	8	24063.55	312.29	9.5	9.4	0.11		
63	8	23997.03	312.28	9.2	9.1	0.11		
64	8	23935.24	312.28	8.9	8.8	0.11		
65	8	23877.00	312.27	8.6	8.5	0.11		
66	7	23821.52	312.27	8.4	8.3	0.11		
67	7	23768.23	312.26	8.1	8.0	0.11		
68	7	23716.76	312.26	7.9	7.8	0.11		
69	7	23666.84	312.25	7.6	7.5	0.11		
70	7	23618.26	312.25	7.4	7.3	0.11		
71	6	23570.90	312.24	7.2	7.1	0.11		
72	6	23524.64	312.24	7.0	6.9	0.11		
73	6	23479.41	312.23	6.8	6.7	0.11		
74	6	23435.15	312.23	6.6	6.5	0.11		
75	6	23391.81	312.22	6.4	6.3	0.11		
76	6	23349.35	312.22	6.2	6.1	0.11		
77	5	23307.75	312.21	6.0	5.9	0.11		
78	5	23266.98	312.21	5.9	5.8	0.11		
79	5	23227.01	312.21	5.7	5.6	0.11		
80	5	23187.82	312.20	5.5	5.4	0.11		
81	5	23149.40	312.20	5.4	5.3	0.11		
82	5	23111.72	312.19	5.2	5.1	0.11		
83	4	23074.77	312.19	5.1	5.0	0.11		
84	4	23038.53	312.19	4.9	4.8	0.11		
85	4	23003.00	312.18	4.8	4.7	0.11		
86	4	22968.14	312.18	4.7	4.5	0.11		
87	4	22933.96	312.18	4.5	4.4	0.11		
88	4	22900.43	312.17	4.4	4.3	0.11		
89	4	22867.55	312.17	4.3	4.2	0.11		
90	4	22835.29	312.17	4.1	4.0	0.11		
91	4	22803.66	312.16	4.0	3.9	0.11		
92	3	22772.63	312.16	3.9	3.8	0.11		
93	3	22742.19	312.16	3.8	3.7	0.11		
94	3	22712.34	312.15	3.7	3.6	0.11		
95	3	22683.05	312.15	3.6	3.5	0.11		
96	3	22654.33	312.15	3.5	3.4	0.11		
97	3	22626.15	312.14	3.4	3.3	0.11		
98	3	22598.52	312.14	3.3	3.2	0.11		
99	3	22571.41	312.14	3.2	3.1	0.11		
100	3	22544.81	312.14	3.1	3.0	0.11		
101	3	22518.72	312.13	3.0	2.9	0.11		
102	3	22493.13	312.13	2.9	2.8	0.11		
103	2	22468.03	312.13	2.9	2.8	0.11		
104	2	22443.40	312.13	2.8	2.7	0.11		
105	2	22419.24	312.12	2.7	2.6	0.11		
106	2	22395.54	312.12	2.6	2.5	0.11		
107	2	22372.29	312.12	2.5	2.4	0.11		
108	2	22349.48	312.12	2.5	2.4	0.11		
109	2	22327.10	312.11	2.4	2.3	0.11		
110	2	22305.15	312.11	2.3	2.2	0.11		
111	2	22283.61	312.11	2.3	2.2	0.11		
112	2	22262.47	312.11	2.2	2.1	0.11		
113	2	22241.74	312.11	2.1	2.0	0.11		
114	2	22221.40	312.10	2.1	2.0	0.11		
115	2	22201.44	312.10	2.0	1.9	0.11		
116	2	22181.86	312.10	2.0	1.9	0.11		
117	2	22162.65	312.10	1.9	1.8	0.11		
118	2	22143.79	312.10	1.9	1.8	0.11		
119	2	22125.29	312.09	1.8	1.7	0.11		
120	1	22107.14	312.09	1.8	1.7	0.11		
121	1	22089.33	312.09	1.7	1.6	0.11		
122	1	22071.86	312.09	1.7	1.6	0.11		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	22054.71	312.09	1.6	1.5	0.11		
124	1	22037.88	312.08	1.6	1.5	0.11		
125	1	22021.36	312.08	1.5	1.4	0.11		
126	1	22005.16	312.08	1.5	1.4	0.11		
127	1	21989.25	312.08	1.4	1.3	0.11		
128	1	21973.64	312.08	1.4	1.3	0.11		
129	1	21958.32	312.08	1.4	1.3	0.11		
130	1	21943.29	312.07	1.3	1.2	0.11		
131	1	21928.53	312.07	1.3	1.2	0.11		
132	1	21914.05	312.07	1.3	1.2	0.11		
133	1	21899.83	312.07	1.2	1.1	0.11		
134	1	21885.88	312.07	1.2	1.1	0.11		
135	1	21872.19	312.07	1.2	1.1	0.11		
136	1	21858.74	312.07	1.1	1.0	0.11		
137	1	21845.55	312.06	1.1	1.0	0.11		
138	1	21832.59	312.06	1.1	1.0	0.11		
139	1	21819.88	312.06	1.0	0.9	0.11		
140	1	21807.39	312.06	1.0	0.9	0.11		
141	1	21795.14	312.06	1.0	0.9	0.11		
142	1	21783.11	312.06	1.0	0.8	0.11		
143	1	21771.29	312.06	0.9	0.8	0.11		
144	1	21759.69	312.06	0.9	0.8	0.11		
145	1	21748.31	312.05	0.9	0.8	0.11		
146	1	21737.12	312.05	0.9	0.7	0.11		
147	1	21726.14	312.05	0.8	0.7	0.11		
148	1	21715.36	312.05	0.8	0.7	0.11		
149	1	21704.77	312.05	0.8	0.7	0.11		
150	1	21694.37	312.05	0.8	0.7	0.11		
151	1	21684.16	312.05	0.7	0.6	0.11		

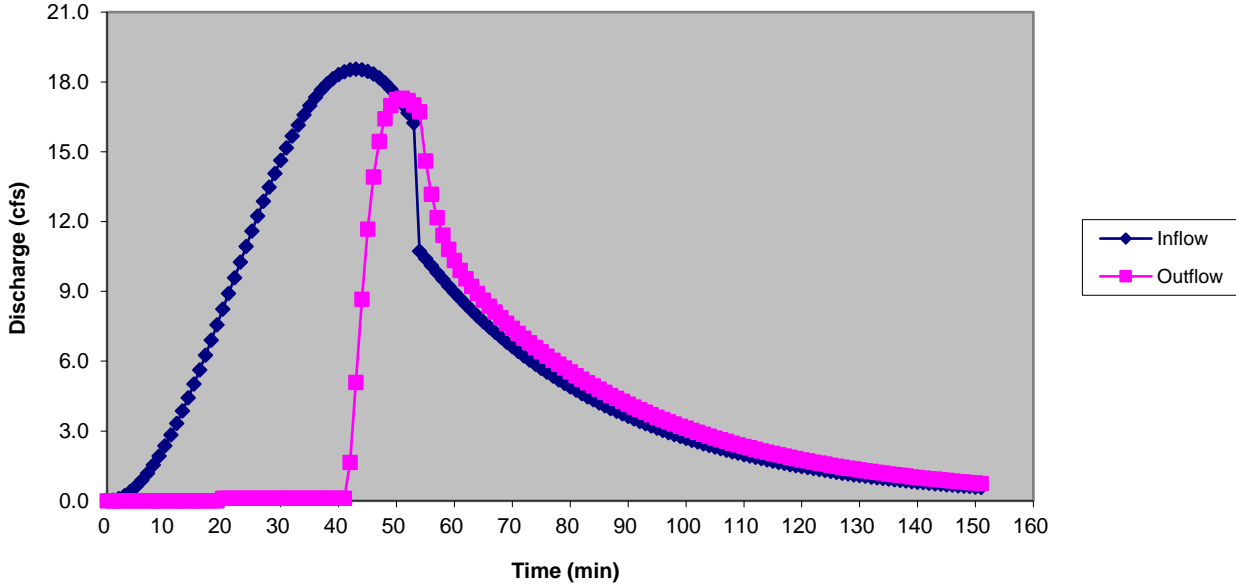




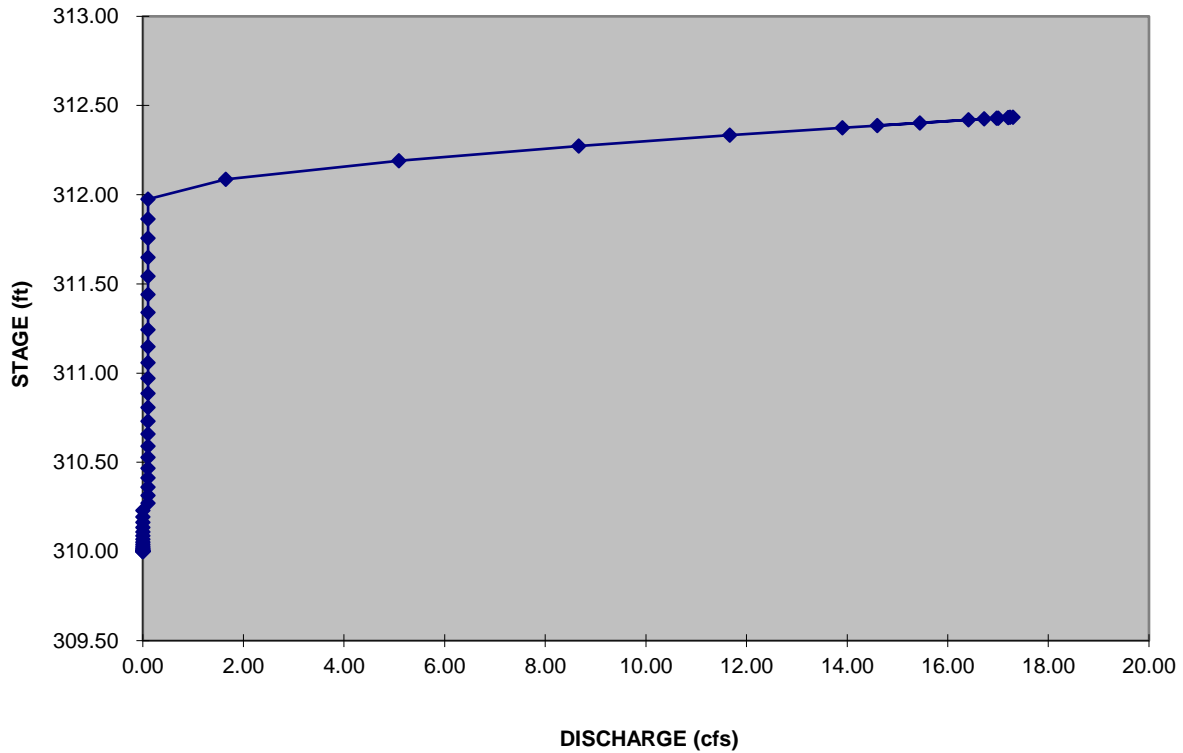
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**SCM-J  
SKIMMER BASIN WITH RISER  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON  
DRH-22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm  
Routing for the 25-year storm  
Graphed Data for the 25-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH-22004  
 Sediment Trap ID SCM-J

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	3.11 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	3.11 ac.	0.25
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.35
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>3.11 ac.</b>	<b>3.11 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.25</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	756	feet	
Height of watershed =	6	feet	
Calculated t(c) =	8.3	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	8.3	minutes	

K Values		
Overland on grassed surfaces:		2.0
Overland on paved surfaces:		0.4
Channel in natural channels:		1.0
Channel in mixed urban setting:		1.1
Channel in paved pipes or channels:		0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	g	h	Intensity
2	132	18	5.02 in/hr
10	195	22	6.44 in/hr
25	232	23	7.42 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.02 in/hr	7.0 cfs	3.91 cfs
10-year storm	6.44 in/hr	9.0 cfs	5.01 cfs
25-year storm	7.42 in/hr	10.4 cfs	5.77 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	3,600	cubic feet per disturbed acre
Required sediment trap surface area	435	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	161
Bottom width =	86
Sediment depth =	4.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Permanent Riser =	4.0
Side slopes =	3.0 H:1V
Spillway length =	16
Height of berm =	6
Top of trap length =	197
Top of trap width =	122
Bottom elevation =	313

### Sediment Trap Data

	Required	Provided	
Sediment storage volume =	11196 cu. ft.	67980 cu. ft.	
Sediment surface area =	3920 sq. ft.	20350 sq. ft.	
Sediment storage depth =	3.5 ft. (max.)	4	Decrease storage depth
Trap bottom length to width ratio =	2.0L:1W (min)	1.9L:1W	Ratio too low
Spillway length =	10.0 ft. (min)	16	
10-Year flow depth over spillway =	0.50 ft. (max)	-1.81 ft	
Freeboard at 10-Year discharge =	1.00 ft. (min)	3.81 ft	

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Required for Sediment Basin = 11,196 cubic feet

24 HOURS	Best Option =	1 - 4 inch Skimmers with a 3 inch orifice
2 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.4 inch orifice
3 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 1.9 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.8 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

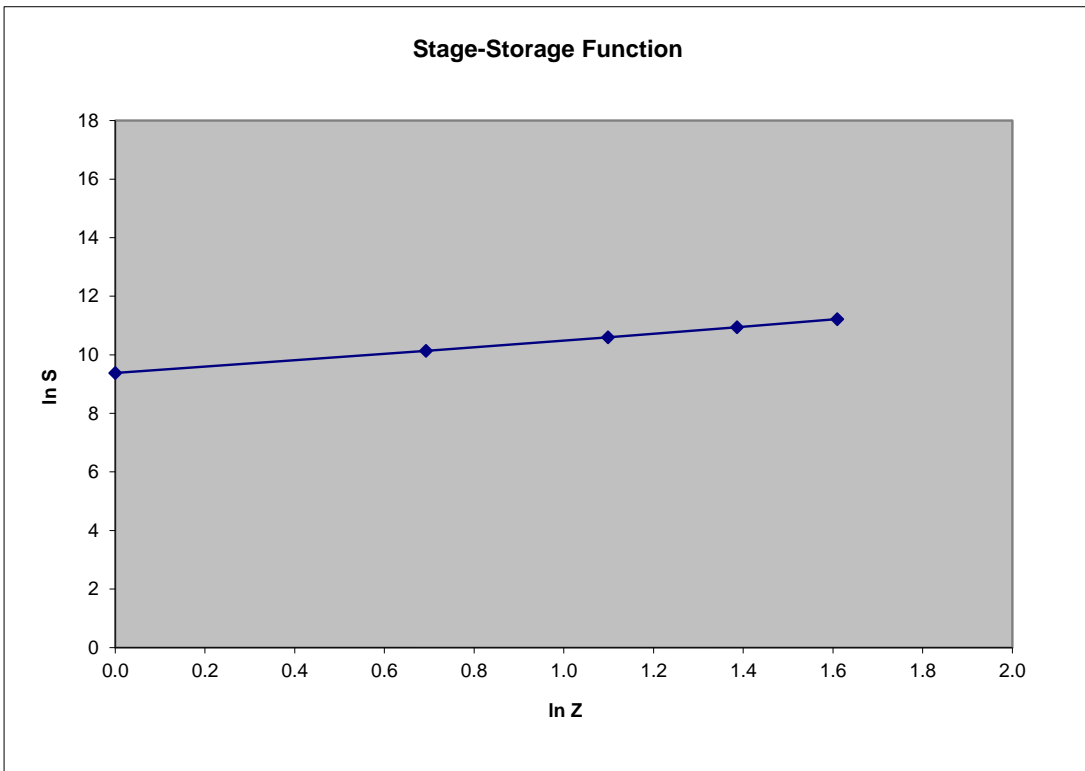
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
313	11119		0	0			
314	12557	11838	11838	1	9.3791	0.0000	1.02
315	14061	13309	25147	2	10.1325	0.6931	1.97
316	15632	14846.5	39993.5	3	10.5965	1.0986	2.95
317	17269	16450.5	56444	4	10.9410	1.3863	3.99
318	18972	18120.5	74564.5	5	11.2194	1.6094	5.10

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.14$  and  $K_s = 11636$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	<b>3.11</b> ac	<b>Use Kirpich Equation:</b> Tc = <b>8.3 min</b>
Disturbed Area =	<b>3.11</b> ac	
Undisturbed Woods =	<b>0.00</b> ac	<b>Use Malcom Method:</b> i <sub>1</sub> = <b>5.02 in/hr</b> i <sub>10</sub> = <b>6.44 in/hr</b> i <sub>25</sub> = <b>7.42 in/hr</b>
Undisturbed Grass =	<b>0.00</b> ac	
Hydraulic Length =	<b>756</b> ft	
Vertical Fall =	<b>6</b> ft	<b>Use Rational Method:</b> Q <sub>2</sub> = <b>7.0 cfs</b> Q <sub>10</sub> = <b>9.0 cfs</b> Q <sub>25</sub> = <b>10.4 cfs</b>
For 1 yr Storm, g =	<b>132</b>	
h =	<b>18</b>	
For 10 yr Storm, g =	<b>195</b>	
h =	<b>22</b>	
For 25 yr Storm, g =	<b>232</b>	
h =	<b>23</b>	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u><u>0.45</u></u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	<b>2.7</b> in (2 Year Storm)	S = <b>0.99</b> in
CN =	<b>91</b> (newly graded areas)	Q* = <b>1.75</b> in
		T <sub>p2</sub> = <b>33.6</b> minutes
P <sub>n6</sub> =	<b>3.9</b> in (10 Year Storm)	Q* = <b>2.92</b> in
		T <sub>p10</sub> = <b>43.9</b> minutes
P <sub>n6</sub> =	<b>4.6</b> in (25 Year Storm)	Q* = <b>3.59</b> in
		T <sub>p25</sub> = <b>46.9</b> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	314.55	ft
	Rise =	1.55	ft
Freeboard =	4.45	ft	Peak Outflow =
			0.06
			cfs

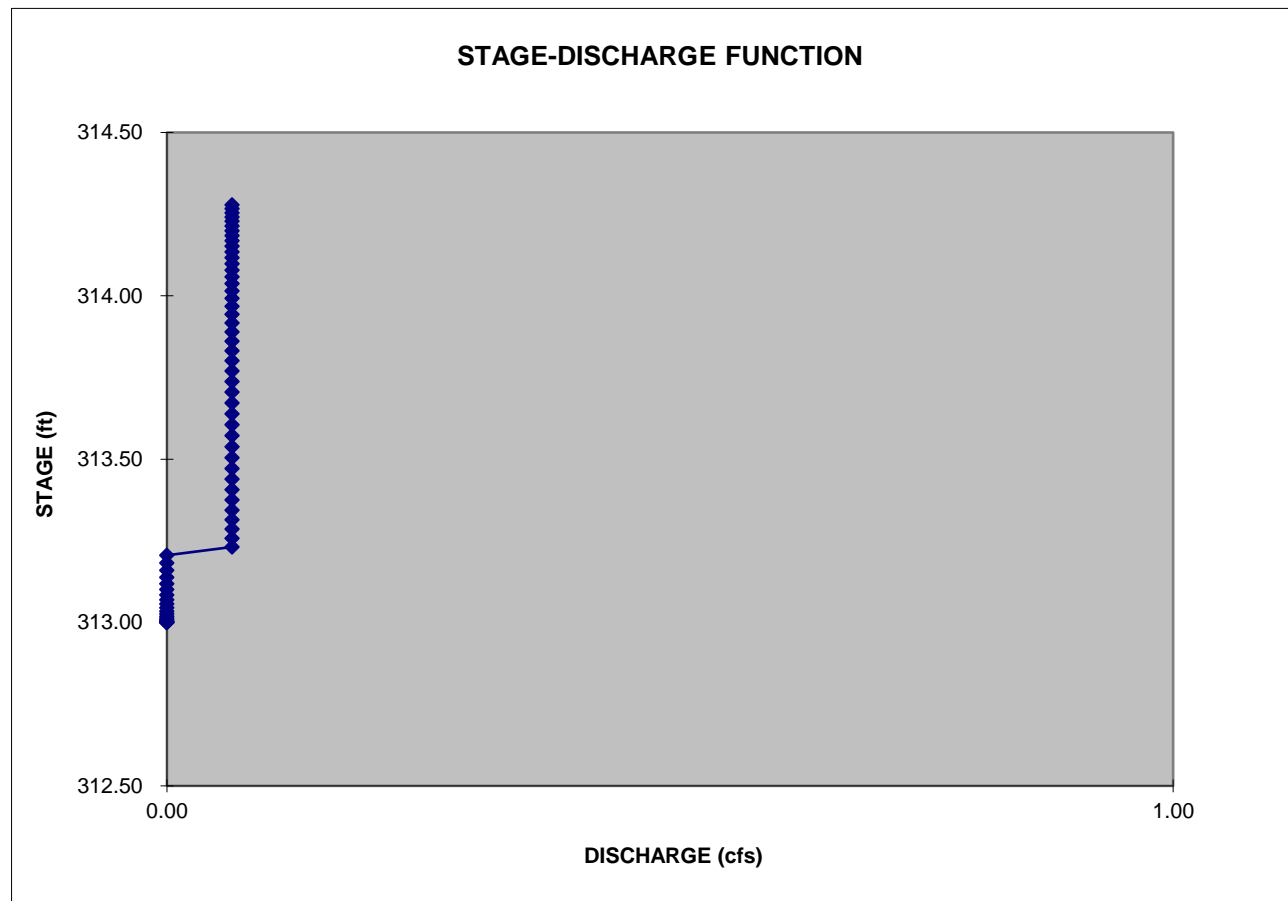
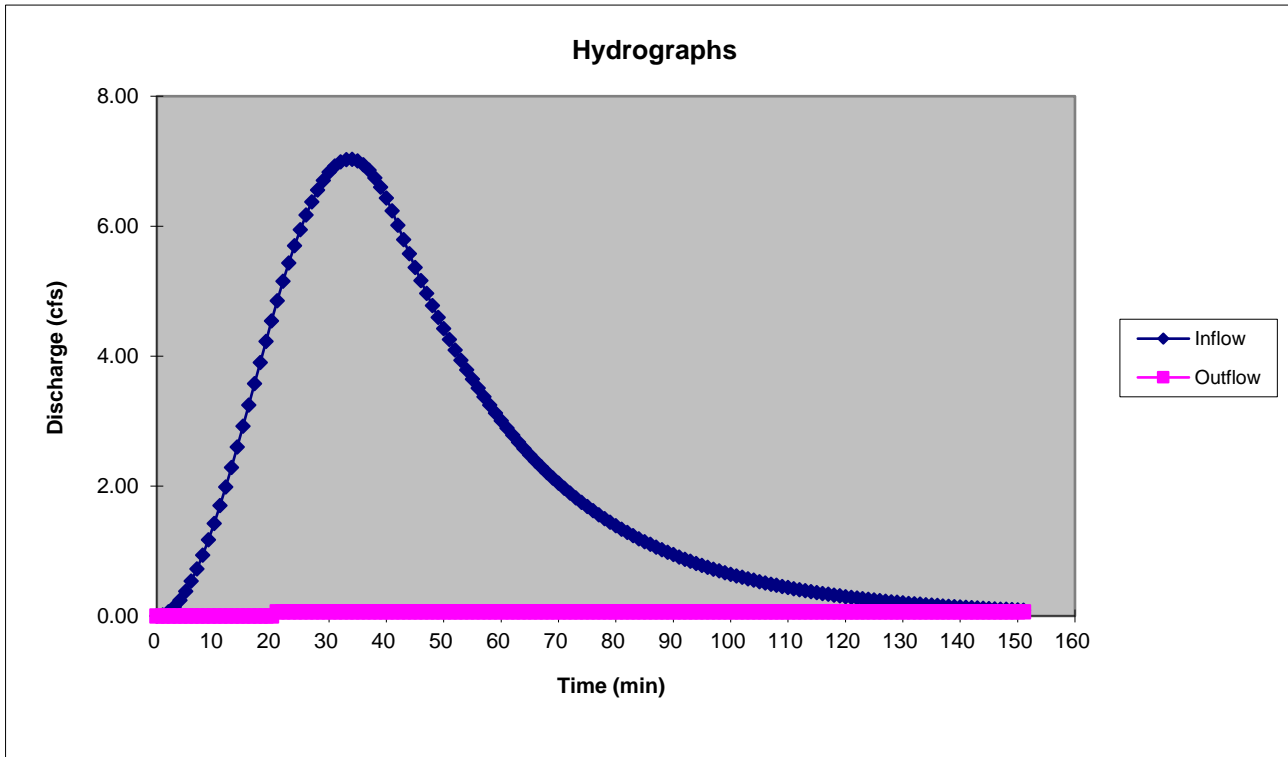
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 319.00 ft
Qp =	7.03	cfs	N =
	33.6	min	L =
	1.0	min	Cw =
<b>Stage-Storage Results:</b>		Zcr =	317.00
Ks =	11636		
b =	1.14		
Z <sub>0</sub> =	313.0	ft (inv)	<b>Skimmer Orifice:</b>
<b>Initial Water Level:</b>			Number =
Z <sub>i</sub> =	313.00	ft	1.00
			Ea
			Diameter =
			2.50
			Inches
			Head =
			1.93
			inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	313.00	0.0	0.0	0.0		
1	0	0.00	313.00	0.0	0.0	0.0		
2	0	0.92	313.00	0.0	0.0	0.0		
3	0	4.59	313.00	0.0	0.0	0.0		
4	0	12.81	313.00	0.0	0.0	0.0		
5	0	27.35	313.00	0.0	0.0	0.0		
6	1	49.91	313.01	0.0	0.0	0.0		
7	1	82.15	313.01	0.0	0.0	0.0		
8	1	125.62	313.02	0.0	0.0	0.0		
9	1	181.77	313.03	0.0	0.0	0.0		
10	1	251.96	313.03	0.0	0.0	0.0		
11	2	337.41	313.04	0.0	0.0	0.0		
12	2	439.21	313.06	0.0	0.0	0.0		
13	2	558.31	313.07	0.0	0.0	0.0		
14	3	695.52	313.08	0.0	0.0	0.0		
15	3	851.47	313.10	0.0	0.0	0.0		
16	3	1026.65	313.12	0.0	0.0	0.0		
17	4	1221.36	313.14	0.0	0.0	0.0		
18	4	1435.75	313.16	0.0	0.0	0.0		
19	4	1669.78	313.18	0.0	0.0	0.0		
20	5	1923.26	313.21	0.0	0.0	0.0		
21	5	2195.81	313.23	0.1	0.0	0.06		
22	5	2483.01	313.26	0.1	0.0	0.06		
23	5	2788.04	313.29	0.1	0.0	0.06		
24	6	3110.07	313.31	0.1	0.0	0.06		
25	6	3448.07	313.34	0.1	0.0	0.06		
26	6	3800.91	313.37	0.1	0.0	0.06		
27	6	4167.33	313.41	0.1	0.0	0.06		
28	7	4545.93	313.44	0.1	0.0	0.06		
29	7	4935.21	313.47	0.1	0.0	0.06		
30	7	5333.59	313.50	0.1	0.0	0.06		
31	7	5739.41	313.54	0.1	0.0	0.06		
32	7	6150.93	313.57	0.1	0.0	0.06		
33	7	6566.36	313.61	0.1	0.0	0.06		
34	7	6983.90	313.64	0.1	0.0	0.06		
35	7	7401.70	313.67	0.1	0.0	0.06		
36	7	7817.94	313.71	0.1	0.0	0.06		
37	7	8230.79	313.74	0.1	0.0	0.06		
38	7	8638.45	313.77	0.1	0.0	0.06		
39	7	9039.19	313.80	0.1	0.0	0.06		
40	6	9431.30	313.83	0.1	0.0	0.06		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.81	ft	
Peak Stage =	315.19	ft	
Rise =	2.19	ft	
Peak Outflow =	0.06	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	319.00 ft
Qp = 9.01 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 43.9 min	L = 16 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.50 Inches	
<b>Stage-Storage Results:</b>	Zcr = 317.00 ft	Head = 1.93 inches	
Ks = 11636			
b = 1.14			
Z <sub>0</sub> = 313.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 313.00 ft			

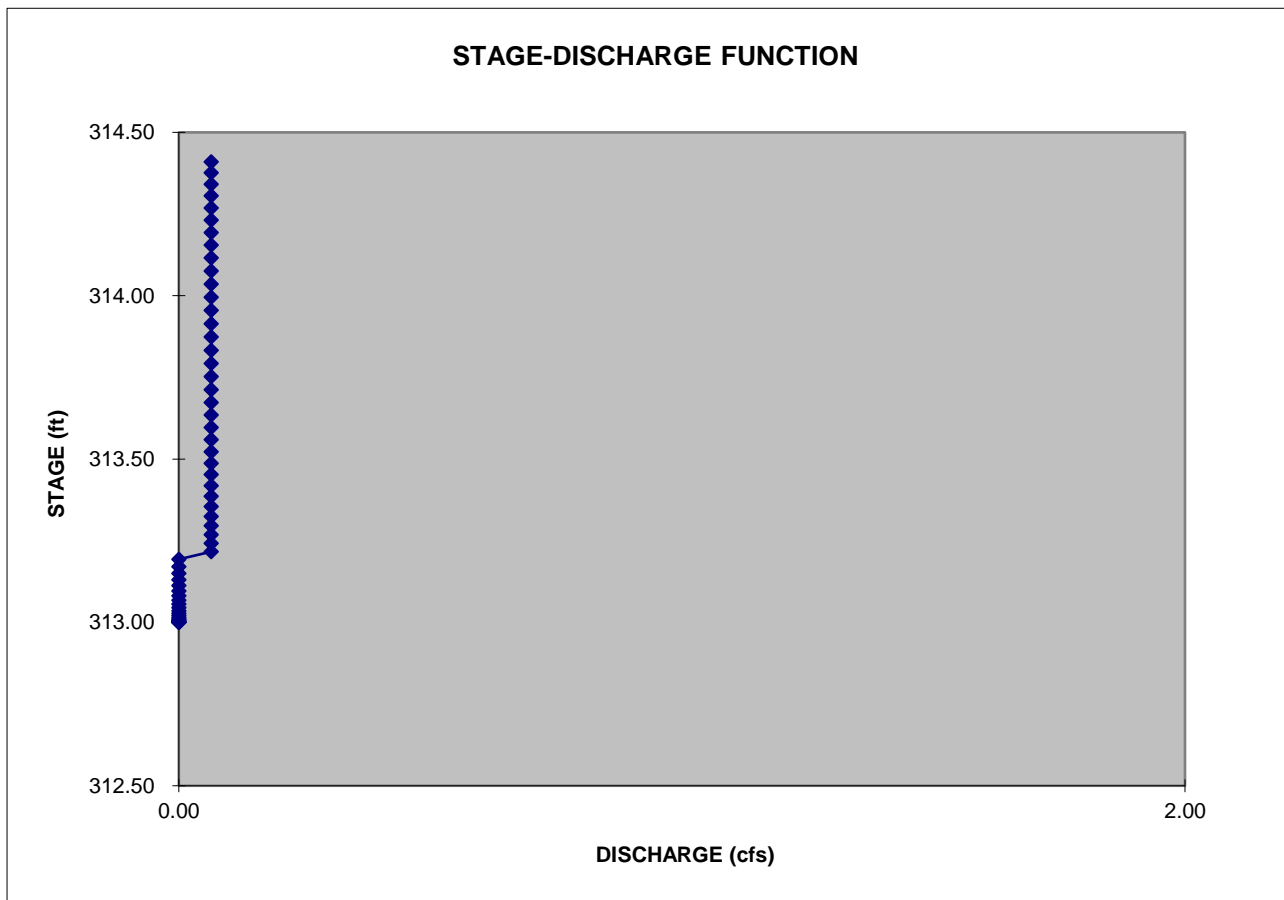
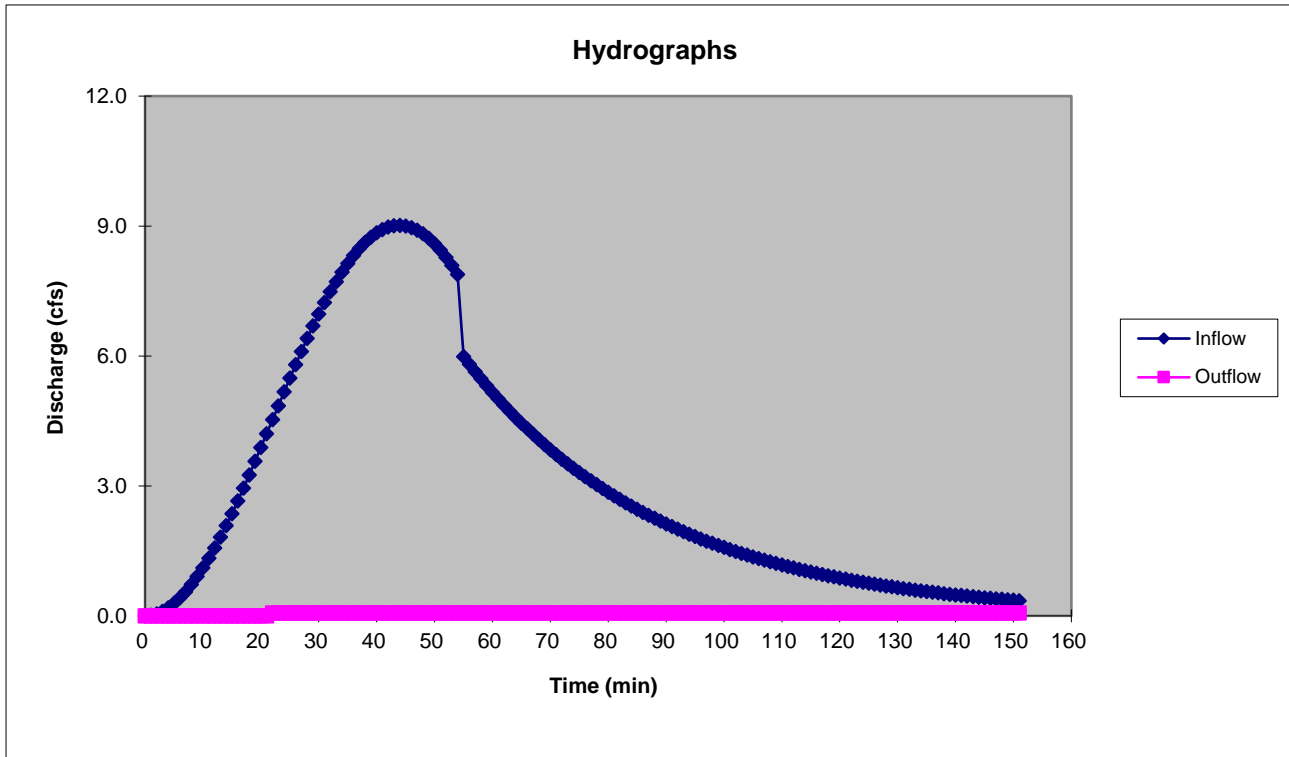
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	313.00	0.0	0.0	0.0		
1	0	0.00	313.00	0.0	0.0	0.00		
2	0	0.69	313.00	0.0	0.0	0.00		
3	0	3.46	313.00	0.0	0.0	0.00		
4	0	9.67	313.00	0.0	0.0	0.00		
5	0	20.69	313.00	0.0	0.0	0.00		
6	0	37.83	313.01	0.0	0.0	0.00		
7	1	62.40	313.01	0.0	0.0	0.00		
8	1	95.66	313.01	0.0	0.0	0.00		
9	1	138.81	313.02	0.0	0.0	0.00		
10	1	193.04	313.03	0.0	0.0	0.00		
11	1	259.44	313.04	0.0	0.0	0.00		
12	2	339.05	313.05	0.0	0.0	0.00		
13	2	432.87	313.06	0.0	0.0	0.00		
14	2	541.78	313.07	0.0	0.0	0.00		
15	2	666.63	313.08	0.0	0.0	0.00		
16	3	808.15	313.10	0.0	0.0	0.00		
17	3	967.00	313.11	0.0	0.0	0.00		
18	3	1143.76	313.13	0.0	0.0	0.00		
19	4	1338.91	313.15	0.0	0.0	0.00		
20	4	1552.83	313.17	0.0	0.0	0.00		
21	4	1785.82	313.19	0.0	0.0	0.00		
22	5	2038.05	313.22	0.1	0.0	0.06		
23	5	2305.75	313.24	0.1	0.0	0.06		
24	5	2592.78	313.27	0.1	0.0	0.06		
25	5	2899.06	313.30	0.1	0.0	0.06		
26	6	3224.36	313.32	0.1	0.0	0.06		
27	6	3568.39	313.35	0.1	0.0	0.06		
28	6	3930.76	313.39	0.1	0.0	0.06		
29	7	4310.97	313.42	0.1	0.0	0.06		
30	7	4708.44	313.45	0.1	0.0	0.06		
31	7	5122.49	313.49	0.1	0.0	0.06		
32	7	5552.38	313.52	0.1	0.0	0.06		
33	8	5997.27	313.56	0.1	0.0	0.06		
34	8	6456.23	313.60	0.1	0.0	0.06		
35	8	6928.29	313.63	0.1	0.0	0.06		
36	8	7412.40	313.67	0.1	0.0	0.06		
37	8	7907.43	313.71	0.1	0.0	0.06		
38	9	8412.21	313.75	0.1	0.0	0.06		
39	9	8925.53	313.79	0.1	0.0	0.06		
40	9	9446.13	313.83	0.1	0.0	0.06		
41	9	9972.69	313.87	0.1	0.0	0.06		
42	9	10503.88	313.91	0.1	0.0	0.06		
43	9	11038.36	313.95	0.1	0.0	0.06		
44	9	11574.74	314.00	0.1	0.0	0.06		
45	9	12111.64	314.04	0.1	0.0	0.06		
46	9	12647.69	314.08	0.1	0.0	0.06		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	9	13181.49	314.12	0.1	0.0	0.06		
48	9	13711.68	314.15	0.1	0.0	0.06		
49	9	14236.90	314.19	0.1	0.0	0.06		
50	9	14755.84	314.23	0.1	0.0	0.06		
51	8	15267.19	314.27	0.1	0.0	0.06		
52	8	15769.71	314.31	0.1	0.0	0.06		
53	8	16262.18	314.34	0.1	0.0	0.06		
54	8	16743.44	314.38	0.1	0.0	0.06		
55	6	17212.40	314.41	0.1	0.0	0.06		
56	6	17567.37	314.44	0.1	0.0	0.06		
57	6	17911.85	314.46	0.1	0.0	0.06		
58	5	18246.17	314.48	0.1	0.0	0.06		
59	5	18570.61	314.51	0.1	0.0	0.06		
60	5	18885.47	314.53	0.1	0.0	0.06		
61	5	19191.03	314.55	0.1	0.0	0.06		
62	5	19487.55	314.57	0.1	0.0	0.06		
63	5	19775.30	314.59	0.1	0.0	0.06		
64	5	20054.53	314.61	0.1	0.0	0.06		
65	4	20325.50	314.63	0.1	0.0	0.06		
66	4	20588.45	314.65	0.1	0.0	0.06		
67	4	20843.61	314.67	0.1	0.0	0.06		
68	4	21091.20	314.68	0.1	0.0	0.06		
69	4	21331.46	314.70	0.1	0.0	0.06		
70	4	21564.58	314.72	0.1	0.0	0.06		
71	4	21790.79	314.73	0.1	0.0	0.06		
72	4	22010.28	314.75	0.1	0.0	0.06		
73	4	22223.25	314.76	0.1	0.0	0.06		
74	3	22429.89	314.78	0.1	0.0	0.06		
75	3	22630.38	314.79	0.1	0.0	0.06		
76	3	22824.91	314.81	0.1	0.0	0.06		
77	3	23013.64	314.82	0.1	0.0	0.06		
78	3	23196.75	314.83	0.1	0.0	0.06		
79	3	23374.40	314.84	0.1	0.0	0.06		
80	3	23546.75	314.86	0.1	0.0	0.06		
81	3	23713.96	314.87	0.1	0.0	0.06		
82	3	23876.17	314.88	0.1	0.0	0.06		
83	3	24033.53	314.89	0.1	0.0	0.06		
84	3	24186.19	314.90	0.1	0.0	0.06		
85	2	24334.28	314.91	0.1	0.0	0.06		
86	2	24477.92	314.92	0.1	0.0	0.06		
87	2	24617.27	314.93	0.1	0.0	0.06		
88	2	24752.43	314.94	0.1	0.0	0.06		
89	2	24883.53	314.95	0.1	0.0	0.06		
90	2	25010.69	314.96	0.1	0.0	0.06		
91	2	25134.02	314.96	0.1	0.0	0.06		
92	2	25253.65	314.97	0.1	0.0	0.06		
93	2	25369.66	314.98	0.1	0.0	0.06		
94	2	25482.18	314.99	0.1	0.0	0.06		
95	2	25591.29	315.00	0.1	0.0	0.06		
96	2	25697.11	315.00	0.1	0.0	0.06		
97	2	25799.73	315.01	0.1	0.0	0.06		
98	2	25899.23	315.02	0.1	0.0	0.06		
99	2	25995.72	315.02	0.1	0.0	0.06		
100	2	26089.28	315.03	0.1	0.0	0.06		
101	2	26179.99	315.04	0.1	0.0	0.06		
102	1	26267.94	315.04	0.1	0.0	0.06		
103	1	26353.21	315.05	0.1	0.0	0.06		
104	1	26435.88	315.05	0.1	0.0	0.06		
105	1	26516.02	315.06	0.1	0.0	0.06		
106	1	26593.70	315.06	0.1	0.0	0.06		
107	1	26669.01	315.07	0.1	0.0	0.06		
108	1	26742.00	315.07	0.1	0.0	0.06		
109	1	26812.75	315.08	0.1	0.0	0.06		
110	1	26881.32	315.08	0.1	0.0	0.06		
111	1	26947.78	315.09	0.1	0.0	0.06		
112	1	27012.18	315.09	0.1	0.0	0.06		
113	1	27074.59	315.10	0.1	0.0	0.06		
114	1	27135.06	315.10	0.1	0.0	0.06		
115	1	27193.66	315.11	0.1	0.0	0.06		
116	1	27250.43	315.11	0.1	0.0	0.06		
117	1	27305.43	315.11	0.1	0.0	0.06		
118	1	27358.71	315.12	0.1	0.0	0.06		
119	1	27410.32	315.12	0.1	0.0	0.06		
120	1	27460.31	315.12	0.1	0.0	0.06		
121	1	27508.73	315.13	0.1	0.0	0.06		
122	1	27555.62	315.13	0.1	0.0	0.06		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	27601.03	315.13	0.1	0.0	0.06		
124	1	27645.00	315.14	0.1	0.0	0.06		
125	1	27687.57	315.14	0.1	0.0	0.06		
126	1	27728.78	315.14	0.1	0.0	0.06		
127	1	27768.68	315.14	0.1	0.0	0.06		
128	1	27807.31	315.15	0.1	0.0	0.06		
129	1	27844.69	315.15	0.1	0.0	0.06		
130	1	27880.86	315.15	0.1	0.0	0.06		
131	1	27915.87	315.15	0.1	0.0	0.06		
132	1	27949.74	315.16	0.1	0.0	0.06		
133	1	27982.51	315.16	0.1	0.0	0.06		
134	1	28014.20	315.16	0.1	0.0	0.06		
135	1	28044.86	315.16	0.1	0.0	0.06		
136	1	28074.51	315.16	0.1	0.0	0.06		
137	1	28103.18	315.17	0.1	0.0	0.06		
138	1	28130.91	315.17	0.1	0.0	0.06		
139	0	28157.70	315.17	0.1	0.0	0.06		
140	0	28183.61	315.17	0.1	0.0	0.06		
141	0	28208.64	315.17	0.1	0.0	0.06		
142	0	28232.83	315.18	0.1	0.0	0.06		
143	0	28256.20	315.18	0.1	0.0	0.06		
144	0	28278.77	315.18	0.1	0.0	0.06		
145	0	28300.57	315.18	0.1	0.0	0.06		
146	0	28321.62	315.18	0.1	0.0	0.06		
147	0	28341.94	315.18	0.1	0.0	0.06		
148	0	28361.56	315.18	0.1	0.0	0.06		
149	0	28380.49	315.19	0.1	0.0	0.06		
150	0	28398.75	315.19	0.1	0.0	0.06		
151	0	28416.37	<b>315.19</b>	0.1	0.0	0.06		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.64	ft	
Peak Stage =	315.36	ft	
Rise =	2.36	ft	
Peak Outflow =	0.06	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	319.00 ft
Qp = 10.38 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 43.9 min	L = 16 ft	Number = 1.00 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.50 Inches	
<b>Stage-Storage Results:</b>	Zcr = 317.00 ft	Head = 1.93 inches	
Ks = 11636			
b = 1.14			
Z <sub>0</sub> = 313.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 313.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	313.00	0.0	0.0	0.0		
1	0	0.00	313.00	0.0	0.0	0.0		
2	0	0.80	313.00	0.0	0.0	0.0		
3	0	3.99	313.00	0.0	0.0	0.0		
4	0	11.14	313.00	0.0	0.0	0.0		
5	0	23.83	313.00	0.0	0.0	0.0		
6	0	43.57	313.01	0.0	0.0	0.0		
7	1	71.87	313.01	0.0	0.0	0.0		
8	1	110.17	313.02	0.0	0.0	0.0		
9	1	159.87	313.02	0.0	0.0	0.0		
10	1	222.32	313.03	0.0	0.0	0.0		
11	2	298.79	313.04	0.0	0.0	0.0		
12	2	390.49	313.05	0.0	0.0	0.0		
13	2	498.54	313.06	0.0	0.0	0.0		
14	2	623.97	313.08	0.0	0.0	0.0		
15	3	767.76	313.09	0.0	0.0	0.0		
16	3	930.75	313.11	0.0	0.0	0.0		
17	3	1113.70	313.13	0.0	0.0	0.0		
18	4	1317.28	313.15	0.0	0.0	0.0		
19	4	1542.03	313.17	0.0	0.0	0.0		
20	4	1788.41	313.19	0.0	0.0	0.0		
21	5	2056.73	313.22	0.1	0.0	0.06		
22	5	2343.35	313.25	0.1	0.0	0.06		
23	6	2652.24	313.27	0.1	0.0	0.06		
24	6	2983.41	313.30	0.1	0.0	0.06		
25	6	3336.74	313.33	0.1	0.0	0.06		
26	7	3711.98	313.37	0.1	0.0	0.06		
27	7	4108.80	313.40	0.1	0.0	0.06		
28	7	4526.73	313.44	0.1	0.0	0.06		
29	8	4965.20	313.47	0.1	0.0	0.06		
30	8	5423.56	313.51	0.1	0.0	0.06		
31	8	5901.02	313.55	0.1	0.0	0.06		
32	9	6396.72	313.59	0.1	0.0	0.06		
33	9	6909.68	313.63	0.1	0.0	0.06		
34	9	7438.87	313.68	0.1	0.0	0.06		
35	9	7983.13	313.72	0.1	0.0	0.06		
36	10	8541.27	313.76	0.1	0.0	0.06		
37	10	9111.98	313.81	0.1	0.0	0.06		
38	10	9693.94	313.85	0.1	0.0	0.06		
39	10	10285.72	313.90	0.1	0.0	0.06		
40	10	10885.88	313.94	0.1	0.0	0.06		
41	10	11492.91	313.99	0.1	0.0	0.06		
42	10	12105.28	314.04	0.1	0.0	0.06		
43	10	12721.43	314.08	0.1	0.0	0.06		
44	10	13339.77	314.13	0.1	0.0	0.06		
45	10	13958.72	314.17	0.1	0.0	0.06		
46	10	14576.68	314.22	0.1	0.0	0.06		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	10	15192.05	314.26	0.1	0.0	0.06		
48	10	15803.26	314.31	0.1	0.0	0.06		
49	10	16408.75	314.35	0.1	0.0	0.06		
50	10	17007.00	314.39	0.1	0.0	0.06		
51	10	17596.52	314.44	0.1	0.0	0.06		
52	10	18175.86	314.48	0.1	0.0	0.06		
53	9	18743.63	314.52	0.1	0.0	0.06		
54	9	19298.50	314.56	0.1	0.0	0.06		
55	6	19839.19	314.60	0.1	0.0	0.06		
56	6	20194.15	314.62	0.1	0.0	0.06		
57	6	20538.64	314.65	0.1	0.0	0.06		
58	5	20872.96	314.67	0.1	0.0	0.06		
59	5	21197.40	314.69	0.1	0.0	0.06		
60	5	21512.26	314.71	0.1	0.0	0.06		
61	5	21817.81	314.74	0.1	0.0	0.06		
62	5	22114.33	314.76	0.1	0.0	0.06		
63	5	22402.08	314.78	0.1	0.0	0.06		
64	5	22681.32	314.80	0.1	0.0	0.06		
65	4	22952.29	314.81	0.1	0.0	0.06		
66	4	23215.24	314.83	0.1	0.0	0.06		
67	4	23470.40	314.85	0.1	0.0	0.06		
68	4	23717.99	314.87	0.1	0.0	0.06		
69	4	23958.24	314.88	0.1	0.0	0.06		
70	4	24191.37	314.90	0.1	0.0	0.06		
71	4	24417.58	314.92	0.1	0.0	0.06		
72	4	24637.07	314.93	0.1	0.0	0.06		
73	4	24850.04	314.95	0.1	0.0	0.06		
74	3	25056.67	314.96	0.1	0.0	0.06		
75	3	25257.17	314.97	0.1	0.0	0.06		
76	3	25451.69	314.99	0.1	0.0	0.06		
77	3	25640.43	315.00	0.1	0.0	0.06		
78	3	25823.54	315.01	0.1	0.0	0.06		
79	3	26001.19	315.02	0.1	0.0	0.06		
80	3	26173.54	315.04	0.1	0.0	0.06		
81	3	26340.75	315.05	0.1	0.0	0.06		
82	3	26502.96	315.06	0.1	0.0	0.06		
83	3	26660.32	315.07	0.1	0.0	0.06		
84	3	26812.98	315.08	0.1	0.0	0.06		
85	2	26961.06	315.09	0.1	0.0	0.06		
86	2	27104.71	315.10	0.1	0.0	0.06		
87	2	27244.05	315.11	0.1	0.0	0.06		
88	2	27379.21	315.12	0.1	0.0	0.06		
89	2	27510.32	315.13	0.1	0.0	0.06		
90	2	27637.48	315.14	0.1	0.0	0.06		
91	2	27760.81	315.14	0.1	0.0	0.06		
92	2	27880.43	315.15	0.1	0.0	0.06		
93	2	27996.45	315.16	0.1	0.0	0.06		
94	2	28108.96	315.17	0.1	0.0	0.06		
95	2	28218.08	315.17	0.1	0.0	0.06		
96	2	28323.90	315.18	0.1	0.0	0.06		
97	2	28426.51	315.19	0.1	0.0	0.06		
98	2	28526.02	315.20	0.1	0.0	0.06		
99	2	28622.51	315.20	0.1	0.0	0.06		
100	2	28716.06	315.21	0.1	0.0	0.06		
101	2	28806.78	315.21	0.1	0.0	0.06		
102	1	28894.73	315.22	0.1	0.0	0.06		
103	1	28980.00	315.23	0.1	0.0	0.06		
104	1	29062.66	315.23	0.1	0.0	0.06		
105	1	29142.80	315.24	0.1	0.0	0.06		
106	1	29220.49	315.24	0.1	0.0	0.06		
107	1	29295.80	315.25	0.1	0.0	0.06		
108	1	29368.79	315.25	0.1	0.0	0.06		
109	1	29439.54	315.26	0.1	0.0	0.06		
110	1	29508.11	315.26	0.1	0.0	0.06		
111	1	29574.57	315.27	0.1	0.0	0.06		
112	1	29638.97	315.27	0.1	0.0	0.06		
113	1	29701.38	315.27	0.1	0.0	0.06		
114	1	29761.85	315.28	0.1	0.0	0.06		
115	1	29820.44	315.28	0.1	0.0	0.06		
116	1	29877.21	315.29	0.1	0.0	0.06		
117	1	29932.21	315.29	0.1	0.0	0.06		
118	1	29985.49	315.29	0.1	0.0	0.06		
119	1	30037.10	315.30	0.1	0.0	0.06		
120	1	30087.10	315.30	0.1	0.0	0.06		
121	1	30135.51	315.30	0.1	0.0	0.06		
122	1	30182.40	315.31	0.1	0.0	0.06		

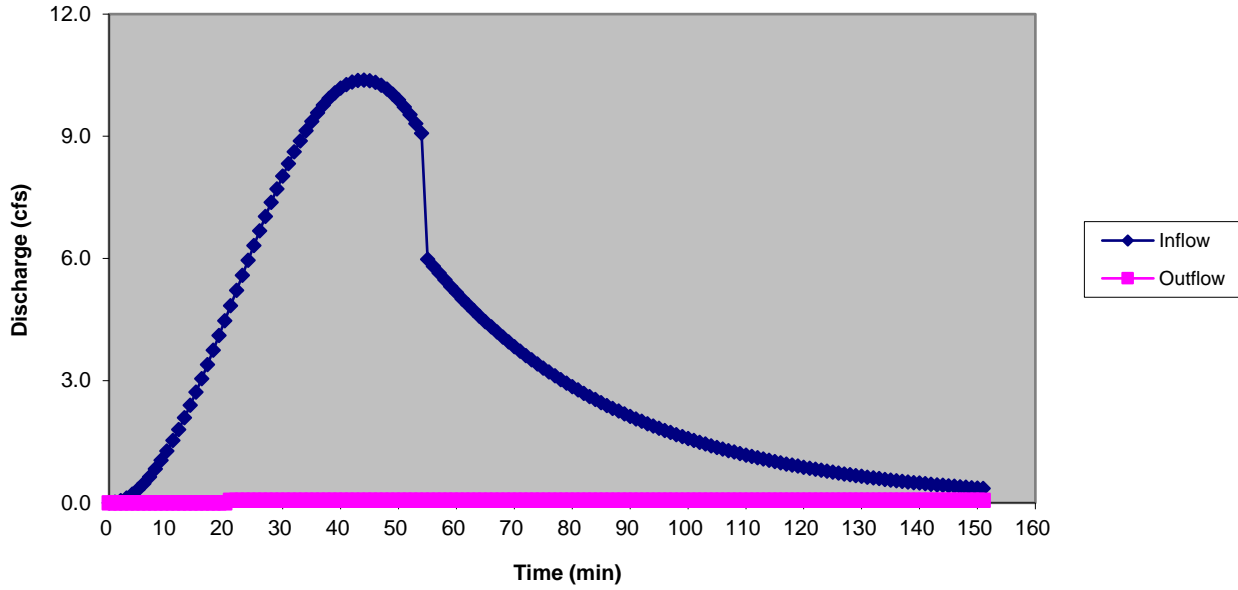
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	30227.81	315.31	0.1	0.0	0.06		
124	1	30271.78	315.31	0.1	0.0	0.06		
125	1	30314.36	315.32	0.1	0.0	0.06		
126	1	30355.57	315.32	0.1	0.0	0.06		
127	1	30395.47	315.32	0.1	0.0	0.06		
128	1	30434.09	315.32	0.1	0.0	0.06		
129	1	30471.47	315.33	0.1	0.0	0.06		
130	1	30507.65	315.33	0.1	0.0	0.06		
131	1	30542.65	315.33	0.1	0.0	0.06		
132	1	30576.52	315.33	0.1	0.0	0.06		
133	1	30609.29	315.34	0.1	0.0	0.06		
134	1	30640.99	315.34	0.1	0.0	0.06		
135	1	30671.65	315.34	0.1	0.0	0.06		
136	1	30701.30	315.34	0.1	0.0	0.06		
137	1	30729.97	315.34	0.1	0.0	0.06		
138	1	30757.69	315.35	0.1	0.0	0.06		
139	0	30784.49	315.35	0.1	0.0	0.06		
140	0	30810.39	315.35	0.1	0.0	0.06		
141	0	30835.43	315.35	0.1	0.0	0.06		
142	0	30859.61	315.35	0.1	0.0	0.06		
143	0	30882.98	315.35	0.1	0.0	0.06		
144	0	30905.56	315.36	0.1	0.0	0.06		
145	0	30927.36	315.36	0.1	0.0	0.06		
146	0	30948.41	315.36	0.1	0.0	0.06		
147	0	30968.73	315.36	0.1	0.0	0.06		
148	0	30988.35	315.36	0.1	0.0	0.06		
149	0	31007.28	315.36	0.1	0.0	0.06		
150	0	31025.54	315.36	0.1	0.0	0.06		
151	0	31043.16	315.36	0.1	0.0	0.06		



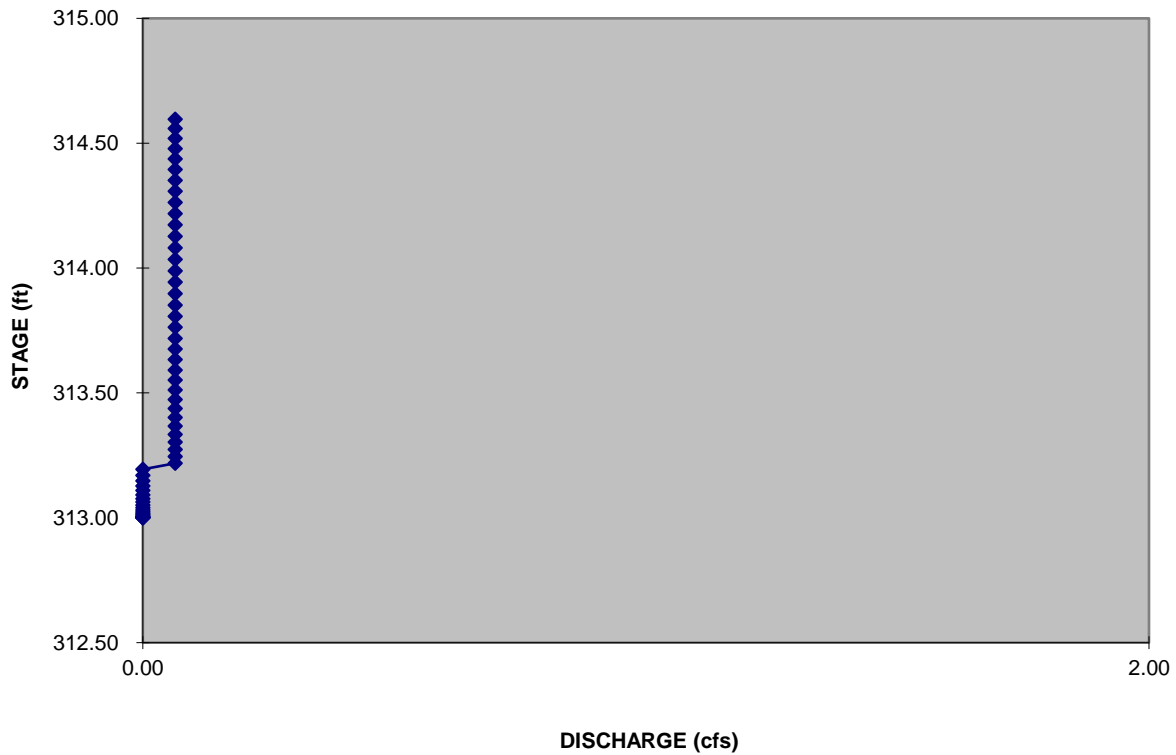
# McADAMS

## GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-1  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm



# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID ST-1

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	1.76 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	1.76 ac.	0.35
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>1.76 ac.</b>	<b>1.76 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	309	feet	
Height of watershed =	6	feet	
Calculated t(c) =	2.9	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	5.0	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.74 in/hr
10	195	22	7.22 in/hr
25	232	23	8.29 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.74 in/hr	4.5 cfs	3.54 cfs
10-year storm	7.22 in/hr	5.7 cfs	4.45 cfs
25-year storm	8.29 in/hr	6.6 cfs	5.10 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	80
Bottom width =	40
Sediment depth =	2.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	2.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	10
Height of berm =	4
Top of trap length =	104
Top of trap width =	64
Bottom elevation =	310

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	3168 cu. ft.	7930 cu. ft.
Sediment surface area =	1860 sq. ft.	4780 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	2
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	10
10-Year flow depth over spillway =	0.50 ft. (max)	-1.12 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	3.12 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 3,168 cubic feet

24 HOURS	Best Option =	1 - 2 inch Skimmers with a 1.9 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.3 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.1 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 0.8 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
310	18395		0	0			
311	19981	19188	19188	1	9.8620	0.0000	1.01
312	21475	20728	39916	2	10.5945	0.6931	1.97
313	22972	22223.5	62139.5	3	11.0371	1.0986	2.95
314	26299	24635.5	86775	4	11.3711	1.3863	3.99
315	27427	26863	113638	5	11.6408	1.6094	5.10

**STAGE-STORAGE FUNCTION:**

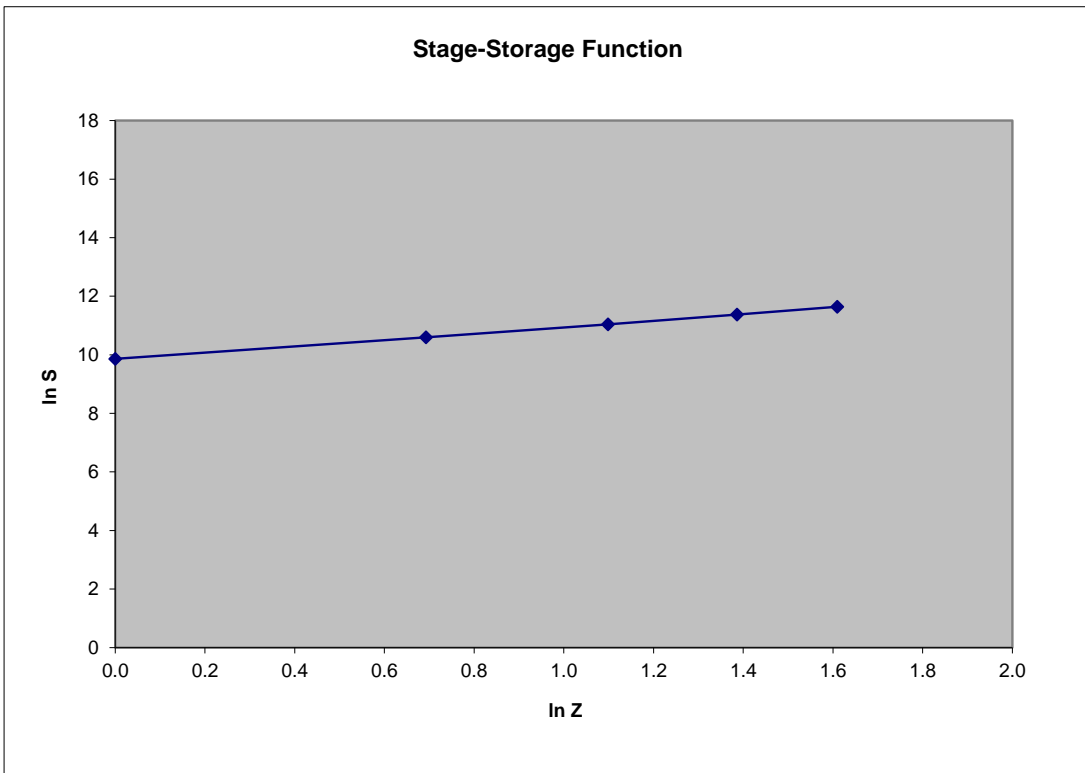
$$S = K_s * Z^b$$

where:

$b = 1.10$

and

$K_s = 18897$





# McADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	1.76 ac	<b>Use Kirpich Equation:</b> Tc = 5.0 min
Disturbed Area =	1.76 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.74 in/hr i <sub>10</sub> = 7.22 in/hr i <sub>25</sub> = 8.29 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	309 ft	
Vertical Fall =	6 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 4.5 cfs Q <sub>10</sub> = 5.7 cfs Q <sub>25</sub> = 6.6 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 29.4 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 39.1 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 42.0 minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	310.61	ft
	Rise =	0.61	ft
Freeboard =		3.39	ft
	Peak Outflow =	0.03	cfs

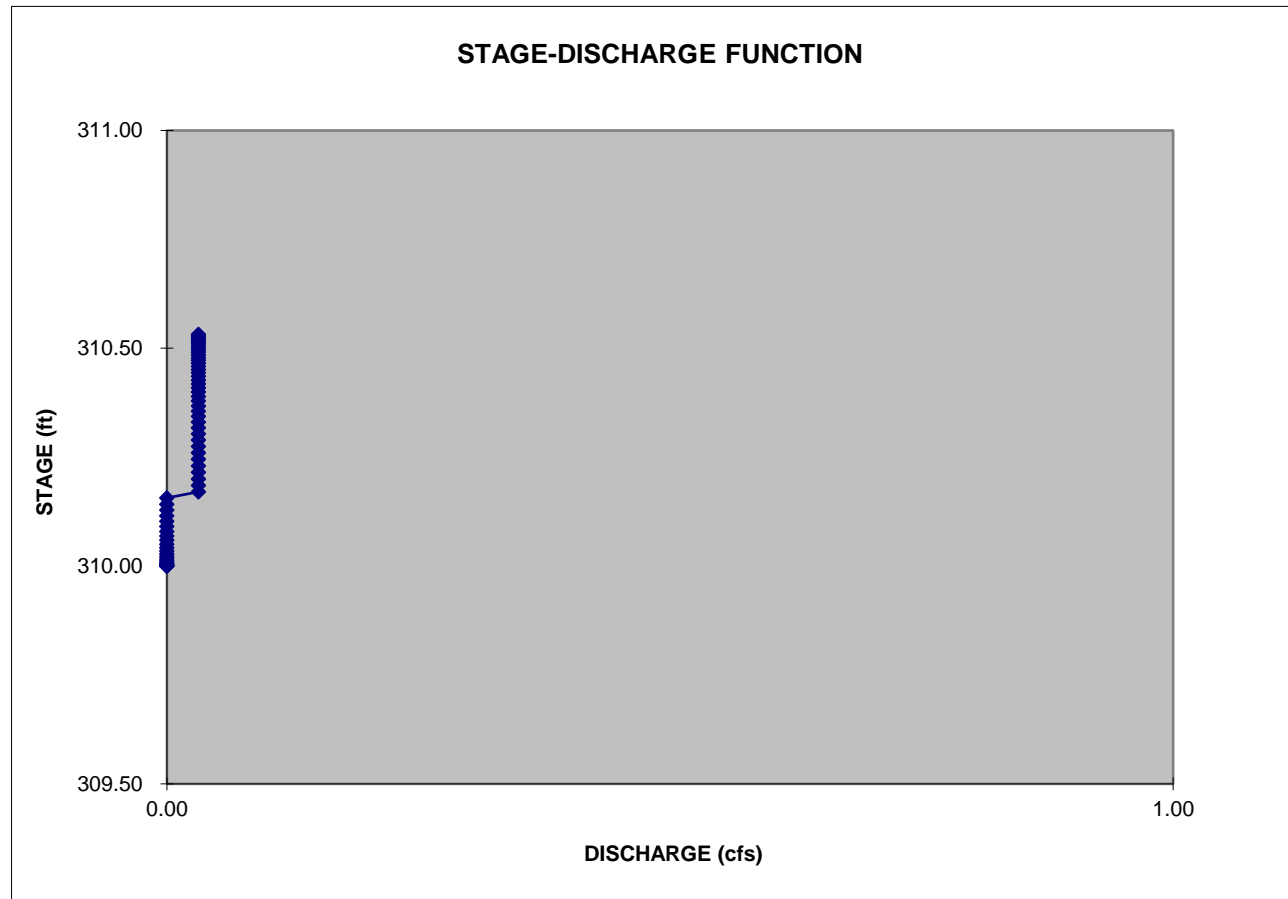
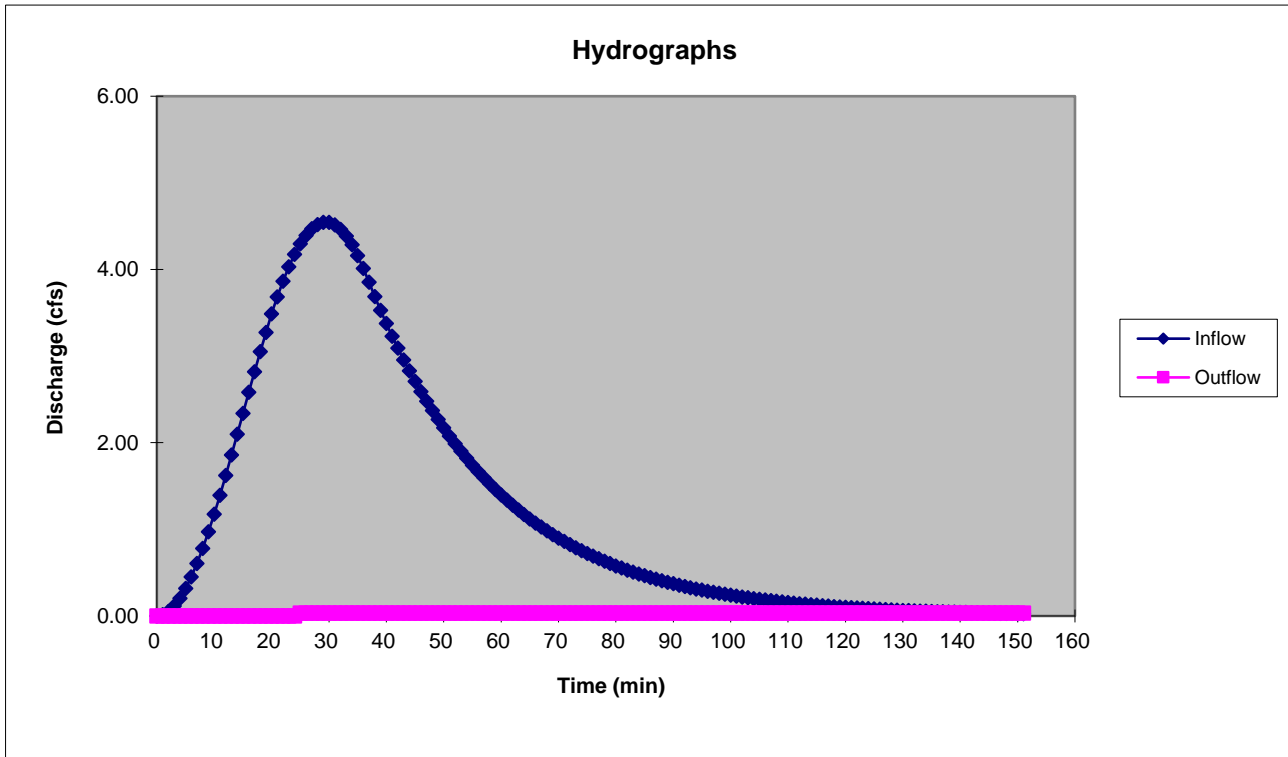
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 314.00 ft
Qp =	4.55	cfs	N =
	29.4	min	L =
	1.0	min	Cw =
<b>Stage-Storage Results:</b>		Zcr =	312.00
Ks =	18897		
b =	1.10		
Z <sub>0</sub> =	310.0	ft (inv)	
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	310.00	ft	
			<b>Skimmer Orifice:</b>
			Number =
			1.00
			Ea
			Diameter =
			2.00
			Inches
			Head =
			1.09
			inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.0		
2	0	0.78	310.00	0.0	0.0	0.0		
3	0	3.87	310.00	0.0	0.0	0.0		
4	0	10.79	310.00	0.0	0.0	0.0		
5	0	23.02	310.00	0.0	0.0	0.0		
6	0	41.96	310.00	0.0	0.0	0.0		
7	1	68.95	310.01	0.0	0.0	0.0		
8	1	105.23	310.01	0.0	0.0	0.0		
9	1	151.95	310.01	0.0	0.0	0.0		
10	1	210.12	310.02	0.0	0.0	0.0		
11	1	280.62	310.02	0.0	0.0	0.0		
12	2	364.21	310.03	0.0	0.0	0.0		
13	2	461.49	310.03	0.0	0.0	0.0		
14	2	572.90	310.04	0.0	0.0	0.0		
15	2	698.73	310.05	0.0	0.0	0.0		
16	3	839.09	310.06	0.0	0.0	0.0		
17	3	993.95	310.07	0.0	0.0	0.0		
18	3	1163.08	310.08	0.0	0.0	0.0		
19	3	1346.12	310.09	0.0	0.0	0.0		
20	3	1542.54	310.10	0.0	0.0	0.0		
21	4	1751.65	310.12	0.0	0.0	0.0		
22	4	1972.62	310.13	0.0	0.0	0.0		
23	4	2204.51	310.14	0.0	0.0	0.0		
24	4	2446.21	310.16	0.0	0.0	0.0		
25	4	2696.53	310.17	0.0	0.0	0.03		
26	4	2952.30	310.19	0.0	0.0	0.03		
27	4	3214.02	310.20	0.0	0.0	0.03		
28	5	3480.25	310.22	0.0	0.0	0.03		
29	5	3749.47	310.23	0.0	0.0	0.03		
30	5	4020.17	310.25	0.0	0.0	0.03		
31	5	4290.78	310.26	0.0	0.0	0.03		
32	4	4559.77	310.27	0.0	0.0	0.03		
33	4	4825.61	310.29	0.0	0.0	0.03		
34	4	5086.80	310.30	0.0	0.0	0.03		
35	4	5341.90	310.32	0.0	0.0	0.03		
36	4	5589.54	310.33	0.0	0.0	0.03		
37	4	5828.44	310.34	0.0	0.0	0.03		
38	4	6057.71	310.36	0.0	0.0	0.03		
39	4	6277.00	310.37	0.0	0.0	0.03		
40	3	6486.74	310.38	0.0	0.0	0.03		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.12	ft	
Peak Stage =	310.88	ft	
Rise =	0.88	ft	
Peak Outflow =	0.03	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 314.00 ft	
Qp = 5.72 cfs	N = 1		
Tp = 39.1 min	L = 10 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 312.00 ft	<b>Skimmer Orifice:</b>	
Ks = 18897		Number = 1	Ea
b = 1.10		Diameter = 2.00	Inches
Z <sub>0</sub> = 310.0 ft (inv)		Head = 1.09	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	0.55	310.00	0.0	0.0	0.00		
3	0	2.76	310.00	0.0	0.0	0.00		
4	0	7.71	310.00	0.0	0.0	0.00		
5	0	16.49	310.00	0.0	0.0	0.00		
6	0	30.13	310.00	0.0	0.0	0.00		
7	0	49.66	310.00	0.0	0.0	0.00		
8	1	76.06	310.01	0.0	0.0	0.00		
9	1	110.25	310.01	0.0	0.0	0.00		
10	1	153.14	310.01	0.0	0.0	0.00		
11	1	205.54	310.02	0.0	0.0	0.00		
12	1	268.23	310.02	0.0	0.0	0.00		
13	1	341.90	310.03	0.0	0.0	0.00		
14	2	427.19	310.03	0.0	0.0	0.00		
15	2	524.65	310.04	0.0	0.0	0.00		
16	2	634.77	310.05	0.0	0.0	0.00		
17	2	757.93	310.05	0.0	0.0	0.00		
18	3	894.45	310.06	0.0	0.0	0.00		
19	3	1044.56	310.07	0.0	0.0	0.00		
20	3	1208.39	310.08	0.0	0.0	0.00		
21	3	1385.99	310.09	0.0	0.0	0.00		
22	3	1577.33	310.10	0.0	0.0	0.00		
23	4	1782.27	310.12	0.0	0.0	0.00		
24	4	2000.61	310.13	0.0	0.0	0.00		
25	4	2232.04	310.14	0.0	0.0	0.00		
26	4	2476.18	310.16	0.0	0.0	0.00		
27	4	2732.56	310.17	0.0	0.0	0.03		
28	5	2998.76	310.19	0.0	0.0	0.03		
29	5	3276.03	310.20	0.0	0.0	0.03		
30	5	3563.68	310.22	0.0	0.0	0.03		
31	5	3860.94	310.24	0.0	0.0	0.03		
32	5	4167.01	310.25	0.0	0.0	0.03		
33	5	4480.99	310.27	0.0	0.0	0.03		
34	5	4801.96	310.29	0.0	0.0	0.03		
35	6	5128.95	310.31	0.0	0.0	0.03		
36	6	5460.93	310.32	0.0	0.0	0.03		
37	6	5796.88	310.34	0.0	0.0	0.03		
38	6	6135.71	310.36	0.0	0.0	0.03		
39	6	6476.33	310.38	0.0	0.0	0.03		
40	6	6817.65	310.40	0.0	0.0	0.03		
41	6	7158.56	310.41	0.0	0.0	0.03		
42	6	7497.95	310.43	0.0	0.0	0.03		
43	6	7834.74	310.45	0.0	0.0	0.03		
44	6	8167.85	310.47	0.0	0.0	0.03		
45	5	8496.21	310.48	0.0	0.0	0.03		
46	5	8818.82	310.50	0.0	0.0	0.03		

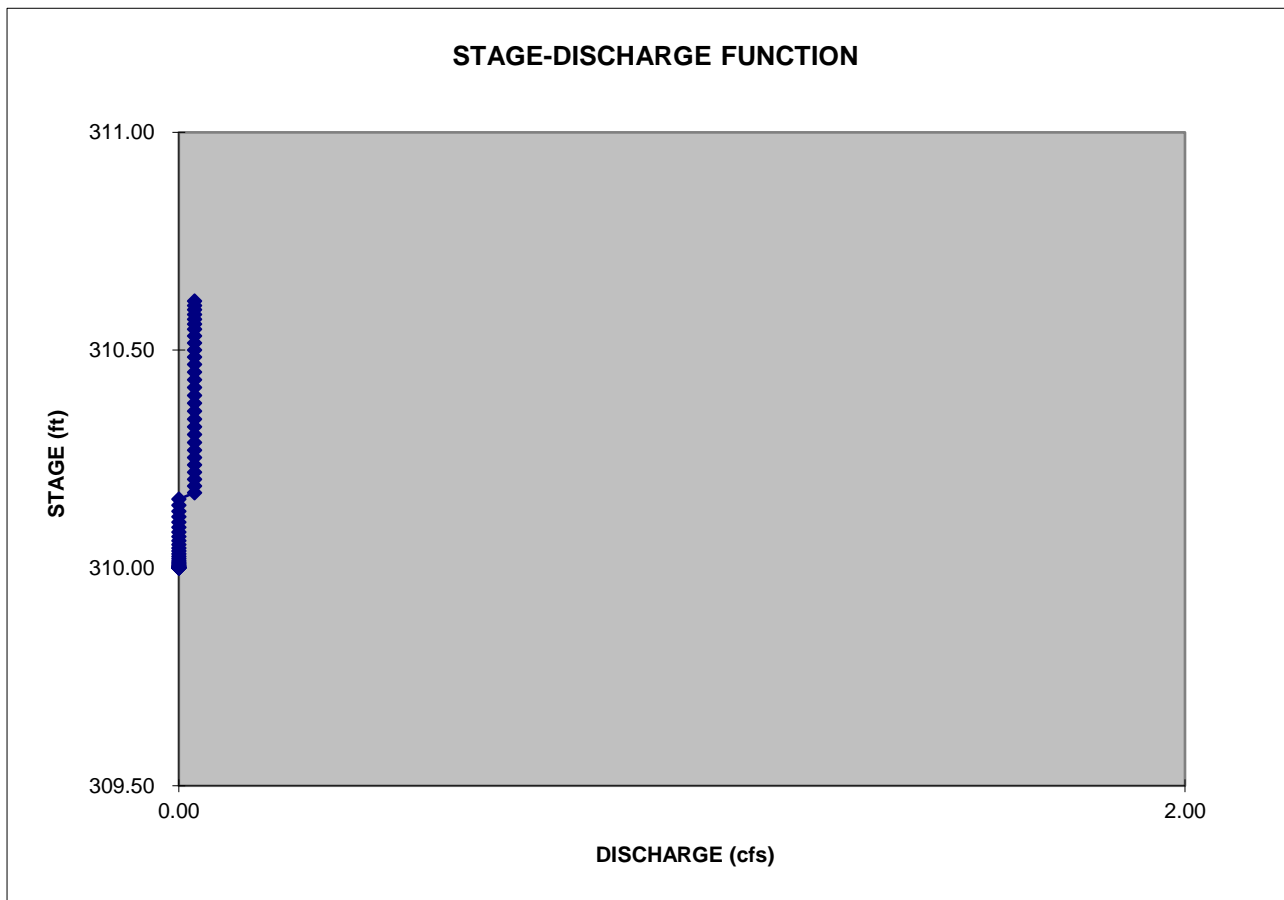
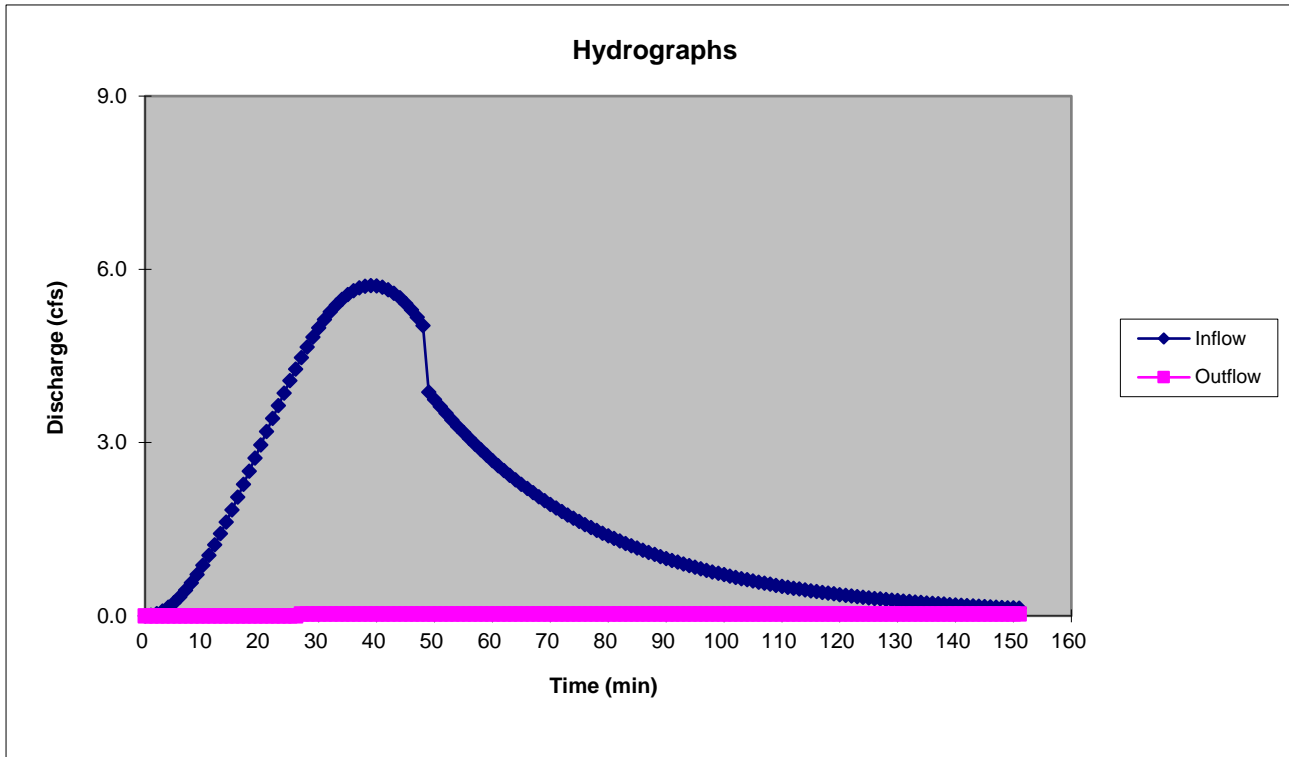
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	5	9134.69	310.52	0.0	0.0	0.03		
48	5	9442.87	310.53	0.0	0.0	0.03		
49	4	9742.48	310.55	0.0	0.0	0.03		
50	4	9972.98	310.56	0.0	0.0	0.03		
51	4	10195.90	310.57	0.0	0.0	0.03		
52	4	10411.46	310.58	0.0	0.0	0.03		
53	3	10619.92	310.59	0.0	0.0	0.03		
54	3	10821.51	310.60	0.0	0.0	0.03		
55	3	11016.45	310.61	0.0	0.0	0.03		
56	3	11204.95	310.62	0.0	0.0	0.03		
57	3	11387.24	310.63	0.0	0.0	0.03		
58	3	11563.51	310.64	0.0	0.0	0.03		
59	3	11733.95	310.65	0.0	0.0	0.03		
60	3	11898.77	310.66	0.0	0.0	0.03		
61	3	12058.14	310.66	0.0	0.0	0.03		
62	3	12212.24	310.67	0.0	0.0	0.03		
63	2	12361.24	310.68	0.0	0.0	0.03		
64	2	12505.31	310.69	0.0	0.0	0.03		
65	2	12644.62	310.69	0.0	0.0	0.03		
66	2	12779.31	310.70	0.0	0.0	0.03		
67	2	12909.53	310.71	0.0	0.0	0.03		
68	2	13035.44	310.71	0.0	0.0	0.03		
69	2	13157.18	310.72	0.0	0.0	0.03		
70	2	13274.87	310.73	0.0	0.0	0.03		
71	2	13388.66	310.73	0.0	0.0	0.03		
72	2	13498.67	310.74	0.0	0.0	0.03		
73	2	13605.02	310.74	0.0	0.0	0.03		
74	2	13707.84	310.75	0.0	0.0	0.03		
75	2	13807.24	310.75	0.0	0.0	0.03		
76	2	13903.32	310.76	0.0	0.0	0.03		
77	2	13996.21	310.76	0.0	0.0	0.03		
78	1	14085.99	310.77	0.0	0.0	0.03		
79	1	14172.79	310.77	0.0	0.0	0.03		
80	1	14256.68	310.77	0.0	0.0	0.03		
81	1	14337.78	310.78	0.0	0.0	0.03		
82	1	14416.16	310.78	0.0	0.0	0.03		
83	1	14491.92	310.79	0.0	0.0	0.03		
84	1	14565.14	310.79	0.0	0.0	0.03		
85	1	14635.91	310.79	0.0	0.0	0.03		
86	1	14704.30	310.80	0.0	0.0	0.03		
87	1	14770.40	310.80	0.0	0.0	0.03		
88	1	14834.28	310.80	0.0	0.0	0.03		
89	1	14896.01	310.81	0.0	0.0	0.03		
90	1	14955.66	310.81	0.0	0.0	0.03		
91	1	15013.30	310.81	0.0	0.0	0.03		
92	1	15069.00	310.81	0.0	0.0	0.03		
93	1	15122.81	310.82	0.0	0.0	0.03		
94	1	15174.81	310.82	0.0	0.0	0.03		
95	1	15225.04	310.82	0.0	0.0	0.03		
96	1	15273.57	310.82	0.0	0.0	0.03		
97	1	15320.46	310.83	0.0	0.0	0.03		
98	1	15365.75	310.83	0.0	0.0	0.03		
99	1	15409.50	310.83	0.0	0.0	0.03		
100	1	15451.76	310.83	0.0	0.0	0.03		
101	1	15492.57	310.83	0.0	0.0	0.03		
102	1	15532.00	310.84	0.0	0.0	0.03		
103	1	15570.07	310.84	0.0	0.0	0.03		
104	1	15606.84	310.84	0.0	0.0	0.03		
105	1	15642.34	310.84	0.0	0.0	0.03		
106	1	15676.63	310.84	0.0	0.0	0.03		
107	1	15709.73	310.85	0.0	0.0	0.03		
108	1	15741.69	310.85	0.0	0.0	0.03		
109	1	15772.54	310.85	0.0	0.0	0.03		
110	1	15802.32	310.85	0.0	0.0	0.03		
111	0	15831.07	310.85	0.0	0.0	0.03		
112	0	15858.82	310.85	0.0	0.0	0.03		
113	0	15885.60	310.85	0.0	0.0	0.03		
114	0	15911.45	310.86	0.0	0.0	0.03		
115	0	15936.39	310.86	0.0	0.0	0.03		
116	0	15960.45	310.86	0.0	0.0	0.03		
117	0	15983.67	310.86	0.0	0.0	0.03		
118	0	16006.06	310.86	0.0	0.0	0.03		
119	0	16027.67	310.86	0.0	0.0	0.03		
120	0	16048.50	310.86	0.0	0.0	0.03		
121	0	16068.59	310.86	0.0	0.0	0.03		
122	0	16087.97	310.86	0.0	0.0	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	16106.65	310.86	0.0	0.0	0.03		
124	0	16124.66	310.87	0.0	0.0	0.03		
125	0	16142.02	310.87	0.0	0.0	0.03		
126	0	16158.75	310.87	0.0	0.0	0.03		
127	0	16174.88	310.87	0.0	0.0	0.03		
128	0	16190.41	310.87	0.0	0.0	0.03		
129	0	16205.38	310.87	0.0	0.0	0.03		
130	0	16219.80	310.87	0.0	0.0	0.03		
131	0	16233.68	310.87	0.0	0.0	0.03		
132	0	16247.05	310.87	0.0	0.0	0.03		
133	0	16259.92	310.87	0.0	0.0	0.03		
134	0	16272.31	310.87	0.0	0.0	0.03		
135	0	16284.24	310.87	0.0	0.0	0.03		
136	0	16295.71	310.87	0.0	0.0	0.03		
137	0	16306.74	310.87	0.0	0.0	0.03		
138	0	16317.36	310.88	0.0	0.0	0.03		
139	0	16327.57	310.88	0.0	0.0	0.03		
140	0	16337.38	310.88	0.0	0.0	0.03		
141	0	16346.81	310.88	0.0	0.0	0.03		
142	0	16355.87	310.88	0.0	0.0	0.03		
143	0	16364.57	310.88	0.0	0.0	0.03		
144	0	16372.93	310.88	0.0	0.0	0.03		
145	0	16380.96	310.88	0.0	0.0	0.03		
146	0	16388.66	310.88	0.0	0.0	0.03		
147	0	16396.05	310.88	0.0	0.0	0.03		
148	0	16403.13	310.88	0.0	0.0	0.03		
149	0	16409.92	310.88	0.0	0.0	0.03		
150	0	16416.43	310.88	0.0	0.0	0.03		
151	0	16422.67	<b>310.88</b>	0.0	0.0	0.03		



# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	3.05	ft	
Peak Stage =	310.95	ft	
Rise =	0.95	ft	
Peak Outflow =	0.03	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 314.00 ft	
Qp = 6.56 cfs	N = 1		
Tp = 39.1 min	L = 10 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 312.00 ft	Number = 1.00	Ea
Ks = 18897		Diameter = 2.00	Inches
b = 1.10		Head = 1.09	inches
Z <sub>0</sub> = 310.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 310.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	310.00	0.0	0.0	0.0		
1	0	0.00	310.00	0.0	0.0	0.00		
2	0	0.63	310.00	0.0	0.0	0.00		
3	0	3.17	310.00	0.0	0.0	0.00		
4	0	8.85	310.00	0.0	0.0	0.00		
5	0	18.92	310.00	0.0	0.0	0.00		
6	0	34.57	310.00	0.0	0.0	0.00		
7	1	56.97	310.01	0.0	0.0	0.00		
8	1	87.26	310.01	0.0	0.0	0.00		
9	1	126.49	310.01	0.0	0.0	0.00		
10	1	175.69	310.01	0.0	0.0	0.00		
11	1	235.81	310.02	0.0	0.0	0.00		
12	1	307.73	310.02	0.0	0.0	0.00		
13	2	392.25	310.03	0.0	0.0	0.00		
14	2	490.10	310.04	0.0	0.0	0.00		
15	2	601.91	310.04	0.0	0.0	0.00		
16	2	728.24	310.05	0.0	0.0	0.00		
17	3	869.54	310.06	0.0	0.0	0.00		
18	3	1026.16	310.07	0.0	0.0	0.00		
19	3	1198.37	310.08	0.0	0.0	0.00		
20	3	1386.32	310.09	0.0	0.0	0.00		
21	4	1590.08	310.11	0.0	0.0	0.00		
22	4	1809.59	310.12	0.0	0.0	0.00		
23	4	2044.72	310.13	0.0	0.0	0.00		
24	4	2295.21	310.15	0.0	0.0	0.00		
25	5	2560.72	310.16	0.0	0.0	0.00		
26	5	2840.81	310.18	0.0	0.0	0.03		
27	5	3133.06	310.20	0.0	0.0	0.03		
28	5	3438.74	310.21	0.0	0.0	0.03		
29	6	3757.11	310.23	0.0	0.0	0.03		
30	6	4087.39	310.25	0.0	0.0	0.03		
31	6	4428.71	310.27	0.0	0.0	0.03		
32	6	4780.12	310.29	0.0	0.0	0.03		
33	6	5140.61	310.31	0.0	0.0	0.03		
34	6	5509.12	310.33	0.0	0.0	0.03		
35	6	5884.53	310.35	0.0	0.0	0.03		
36	6	6265.68	310.37	0.0	0.0	0.03		
37	7	6651.37	310.39	0.0	0.0	0.03		
38	7	7040.37	310.41	0.0	0.0	0.03		
39	7	7431.42	310.43	0.0	0.0	0.03		
40	7	7823.28	310.45	0.0	0.0	0.03		
41	7	8214.66	310.47	0.0	0.0	0.03		
42	6	8604.31	310.49	0.0	0.0	0.03		
43	6	8990.97	310.51	0.0	0.0	0.03		
44	6	9373.40	310.53	0.0	0.0	0.03		
45	6	9750.39	310.55	0.0	0.0	0.03		
46	6	10120.79	310.57	0.0	0.0	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	6	10483.44	310.59	0.0	0.0	0.03		
48	6	10837.28	310.60	0.0	0.0	0.03		
49	4	11181.28	310.62	0.0	0.0	0.03		
50	4	11411.78	310.63	0.0	0.0	0.03		
51	4	11634.69	310.64	0.0	0.0	0.03		
52	4	11850.26	310.65	0.0	0.0	0.03		
53	3	12058.72	310.67	0.0	0.0	0.03		
54	3	12260.31	310.68	0.0	0.0	0.03		
55	3	12455.25	310.68	0.0	0.0	0.03		
56	3	12643.75	310.69	0.0	0.0	0.03		
57	3	12826.04	310.70	0.0	0.0	0.03		
58	3	13002.31	310.71	0.0	0.0	0.03		
59	3	13172.75	310.72	0.0	0.0	0.03		
60	3	13337.57	310.73	0.0	0.0	0.03		
61	3	13496.94	310.74	0.0	0.0	0.03		
62	3	13651.04	310.74	0.0	0.0	0.03		
63	2	13800.04	310.75	0.0	0.0	0.03		
64	2	13944.11	310.76	0.0	0.0	0.03		
65	2	14083.41	310.77	0.0	0.0	0.03		
66	2	14218.10	310.77	0.0	0.0	0.03		
67	2	14348.33	310.78	0.0	0.0	0.03		
68	2	14474.24	310.78	0.0	0.0	0.03		
69	2	14595.98	310.79	0.0	0.0	0.03		
70	2	14713.67	310.80	0.0	0.0	0.03		
71	2	14827.46	310.80	0.0	0.0	0.03		
72	2	14937.47	310.81	0.0	0.0	0.03		
73	2	15043.82	310.81	0.0	0.0	0.03		
74	2	15146.64	310.82	0.0	0.0	0.03		
75	2	15246.03	310.82	0.0	0.0	0.03		
76	2	15342.12	310.83	0.0	0.0	0.03		
77	2	15435.00	310.83	0.0	0.0	0.03		
78	1	15524.79	310.84	0.0	0.0	0.03		
79	1	15611.59	310.84	0.0	0.0	0.03		
80	1	15695.48	310.84	0.0	0.0	0.03		
81	1	15776.58	310.85	0.0	0.0	0.03		
82	1	15854.96	310.85	0.0	0.0	0.03		
83	1	15930.72	310.86	0.0	0.0	0.03		
84	1	16003.94	310.86	0.0	0.0	0.03		
85	1	16074.71	310.86	0.0	0.0	0.03		
86	1	16143.10	310.87	0.0	0.0	0.03		
87	1	16209.20	310.87	0.0	0.0	0.03		
88	1	16273.08	310.87	0.0	0.0	0.03		
89	1	16334.81	310.88	0.0	0.0	0.03		
90	1	16394.46	310.88	0.0	0.0	0.03		
91	1	16452.10	310.88	0.0	0.0	0.03		
92	1	16507.79	310.88	0.0	0.0	0.03		
93	1	16561.61	310.89	0.0	0.0	0.03		
94	1	16613.60	310.89	0.0	0.0	0.03		
95	1	16663.84	310.89	0.0	0.0	0.03		
96	1	16712.37	310.89	0.0	0.0	0.03		
97	1	16759.25	310.90	0.0	0.0	0.03		
98	1	16804.55	310.90	0.0	0.0	0.03		
99	1	16848.30	310.90	0.0	0.0	0.03		
100	1	16890.56	310.90	0.0	0.0	0.03		
101	1	16931.37	310.91	0.0	0.0	0.03		
102	1	16970.79	310.91	0.0	0.0	0.03		
103	1	17008.87	310.91	0.0	0.0	0.03		
104	1	17045.64	310.91	0.0	0.0	0.03		
105	1	17081.14	310.91	0.0	0.0	0.03		
106	1	17115.42	310.91	0.0	0.0	0.03		
107	1	17148.53	310.92	0.0	0.0	0.03		
108	1	17180.48	310.92	0.0	0.0	0.03		
109	1	17211.34	310.92	0.0	0.0	0.03		
110	1	17241.12	310.92	0.0	0.0	0.03		
111	0	17269.87	310.92	0.0	0.0	0.03		
112	0	17297.62	310.92	0.0	0.0	0.03		
113	0	17324.40	310.92	0.0	0.0	0.03		
114	0	17350.25	310.93	0.0	0.0	0.03		
115	0	17375.19	310.93	0.0	0.0	0.03		
116	0	17399.25	310.93	0.0	0.0	0.03		
117	0	17422.47	310.93	0.0	0.0	0.03		
118	0	17444.86	310.93	0.0	0.0	0.03		
119	0	17466.46	310.93	0.0	0.0	0.03		
120	0	17487.30	310.93	0.0	0.0	0.03		
121	0	17507.39	310.93	0.0	0.0	0.03		
122	0	17526.77	310.93	0.0	0.0	0.03		

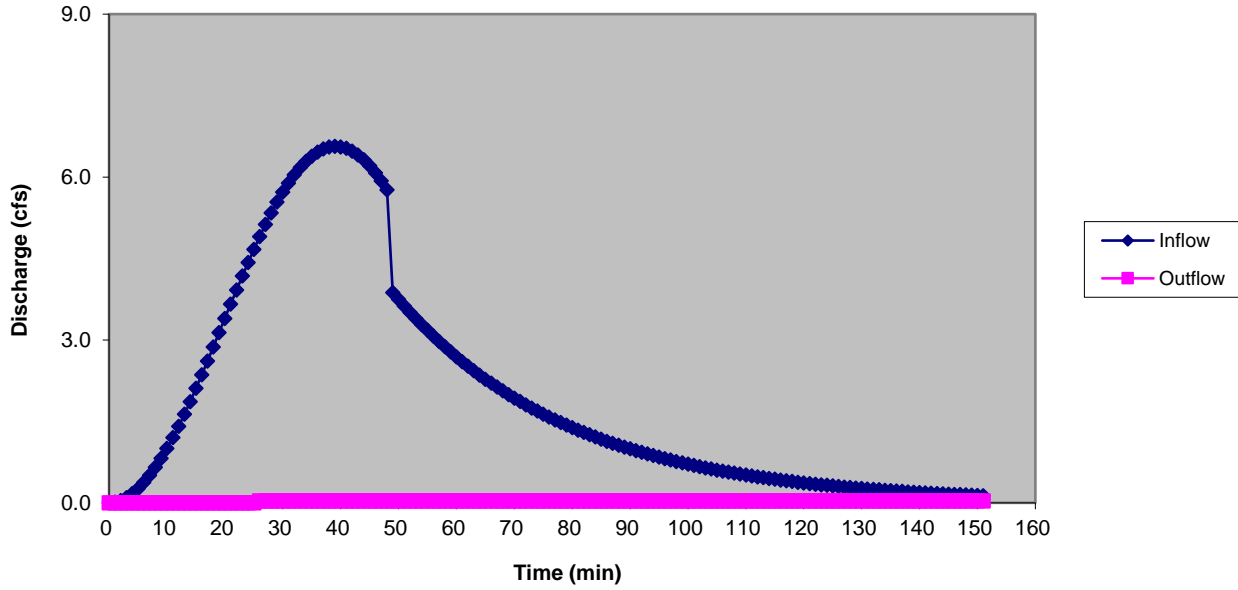
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	17545.45	310.93	0.0	0.0	0.03		
124	0	17563.46	310.94	0.0	0.0	0.03		
125	0	17580.82	310.94	0.0	0.0	0.03		
126	0	17597.55	310.94	0.0	0.0	0.03		
127	0	17613.68	310.94	0.0	0.0	0.03		
128	0	17629.21	310.94	0.0	0.0	0.03		
129	0	17644.18	310.94	0.0	0.0	0.03		
130	0	17658.59	310.94	0.0	0.0	0.03		
131	0	17672.48	310.94	0.0	0.0	0.03		
132	0	17685.85	310.94	0.0	0.0	0.03		
133	0	17698.72	310.94	0.0	0.0	0.03		
134	0	17711.11	310.94	0.0	0.0	0.03		
135	0	17723.03	310.94	0.0	0.0	0.03		
136	0	17734.51	310.94	0.0	0.0	0.03		
137	0	17745.54	310.94	0.0	0.0	0.03		
138	0	17756.16	310.95	0.0	0.0	0.03		
139	0	17766.37	310.95	0.0	0.0	0.03		
140	0	17776.18	310.95	0.0	0.0	0.03		
141	0	17785.61	310.95	0.0	0.0	0.03		
142	0	17794.67	310.95	0.0	0.0	0.03		
143	0	17803.37	310.95	0.0	0.0	0.03		
144	0	17811.73	310.95	0.0	0.0	0.03		
145	0	17819.76	310.95	0.0	0.0	0.03		
146	0	17827.46	310.95	0.0	0.0	0.03		
147	0	17834.84	310.95	0.0	0.0	0.03		
148	0	17841.93	310.95	0.0	0.0	0.03		
149	0	17848.72	310.95	0.0	0.0	0.03		
150	0	17855.23	310.95	0.0	0.0	0.03		
151	0	17861.47	<b>310.95</b>	0.0	0.0	0.03		



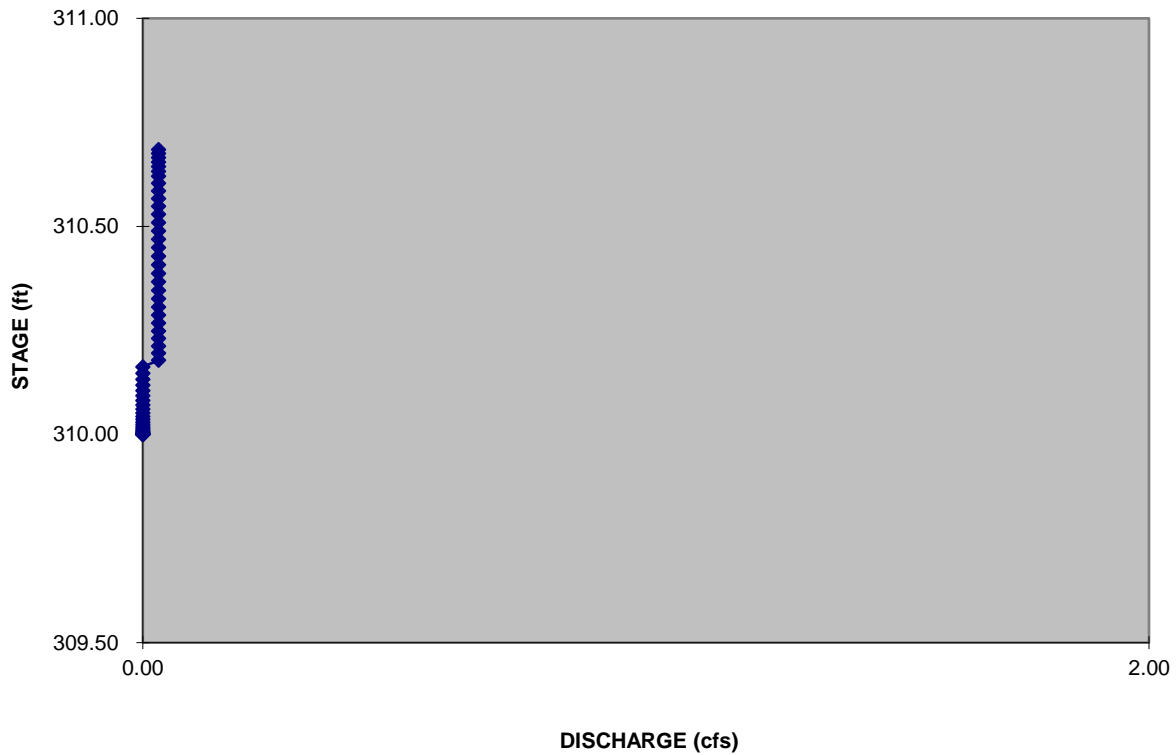
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-2  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID: ST-2

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	3.67 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	3.67 ac.	0.35
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>3.67 ac.</b>	<b>3.67 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	662 feet		
Height of watershed =	11 feet		
Calculated t(c) =	5.6 minutes		
Minimum t(c) =	5.0 minutes		
Time of concentration =	5.6 minutes		

K Values		
Overland on grassed surfaces:		2.0
Overland on paved surfaces:		0.4
Channel in natural channels:		1.0
Channel in mixed urban setting:		1.1
Channel in paved pipes or channels:		0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.59 in/hr
10	195	22	7.06 in/hr
25	232	23	8.11 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.59 in/hr	9.2 cfs	7.18 cfs
10-year storm	7.06 in/hr	11.7 cfs	9.07 cfs
25-year storm	8.11 in/hr	13.4 cfs	10.41 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	80
Bottom width =	40
Sediment depth =	2.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	2.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	4
Top of trap length =	104
Top of trap width =	64
Bottom elevation =	308

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	6606 cu. ft.	7930 cu. ft.
Sediment surface area =	3790 sq. ft.	4780 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	2
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.33 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.67 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 6,606 cubic feet

24 HOURS	Best Option =	1 - 3 inch Skimmers with a 2.5 inch orifice
2 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 1.9 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.6 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.2 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

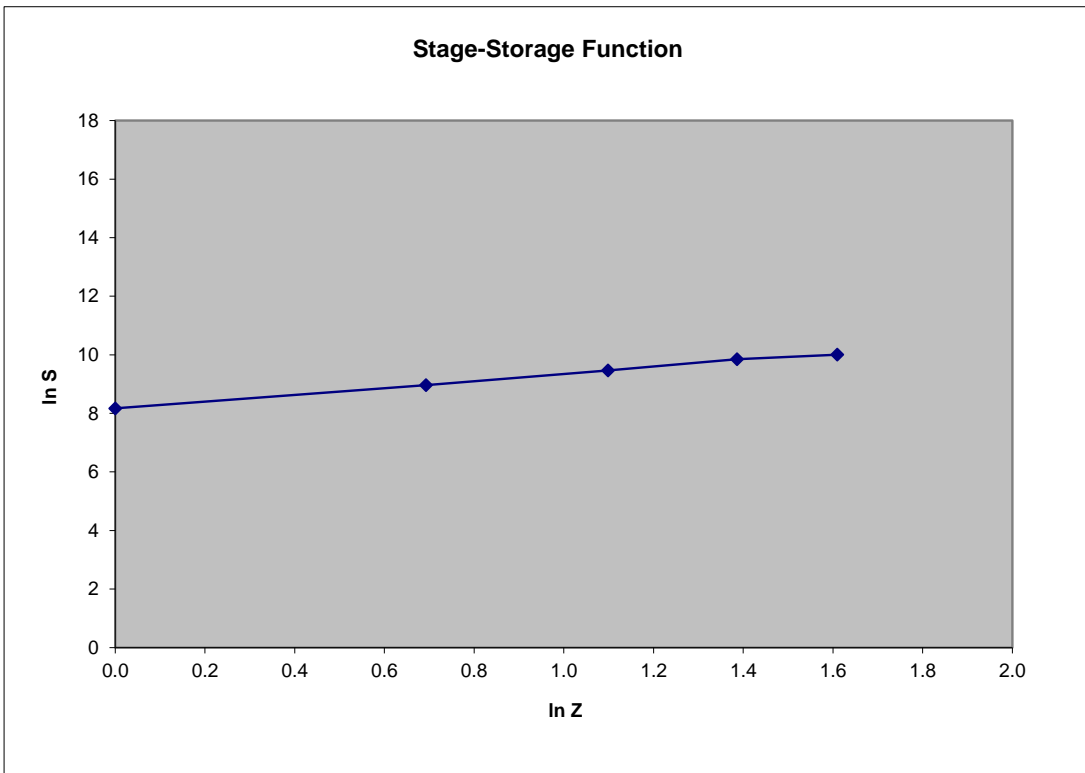
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
308	3179		0	0			
309	3901	3540	3540	1	8.1719	0.0000	1.00
310	4680	4290.5	7830.5	2	8.9658	0.6931	1.97
311	5516	5098	12928.5	3	9.4672	1.0986	3.03
312	6408	5962	18890.5	4	9.8464	1.3863	4.19
313	0	3204	22094.5	5	10.0031	1.6094	4.80

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.17$  and  $K_s = 3547$







# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	3.67 ac	<b>Use Kirpich Equation:</b> Tc = 5.6 min
Disturbed Area =	3.67 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.59 in/hr i <sub>10</sub> = 7.06 in/hr i <sub>25</sub> = 8.11 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	662 ft	
Vertical Fall =	11 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 9.2 cfs Q <sub>10</sub> = 11.7 cfs Q <sub>25</sub> = 13.4 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 30.2 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 40.0 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 42.9 minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	310.27	ft
	Rise =	2.27	ft
Freeboard =		1.73	ft
	Peak Outflow =	8.67	cfs

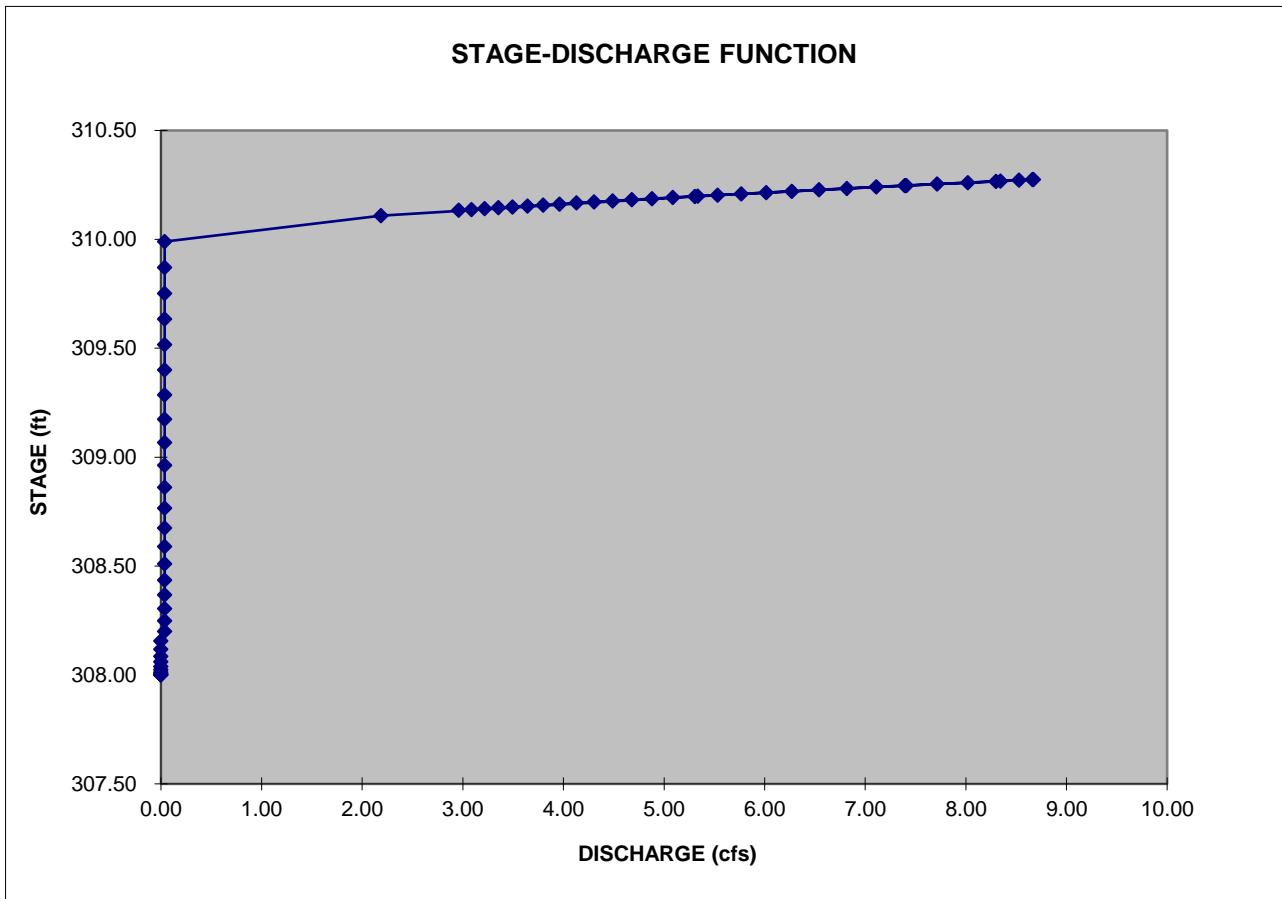
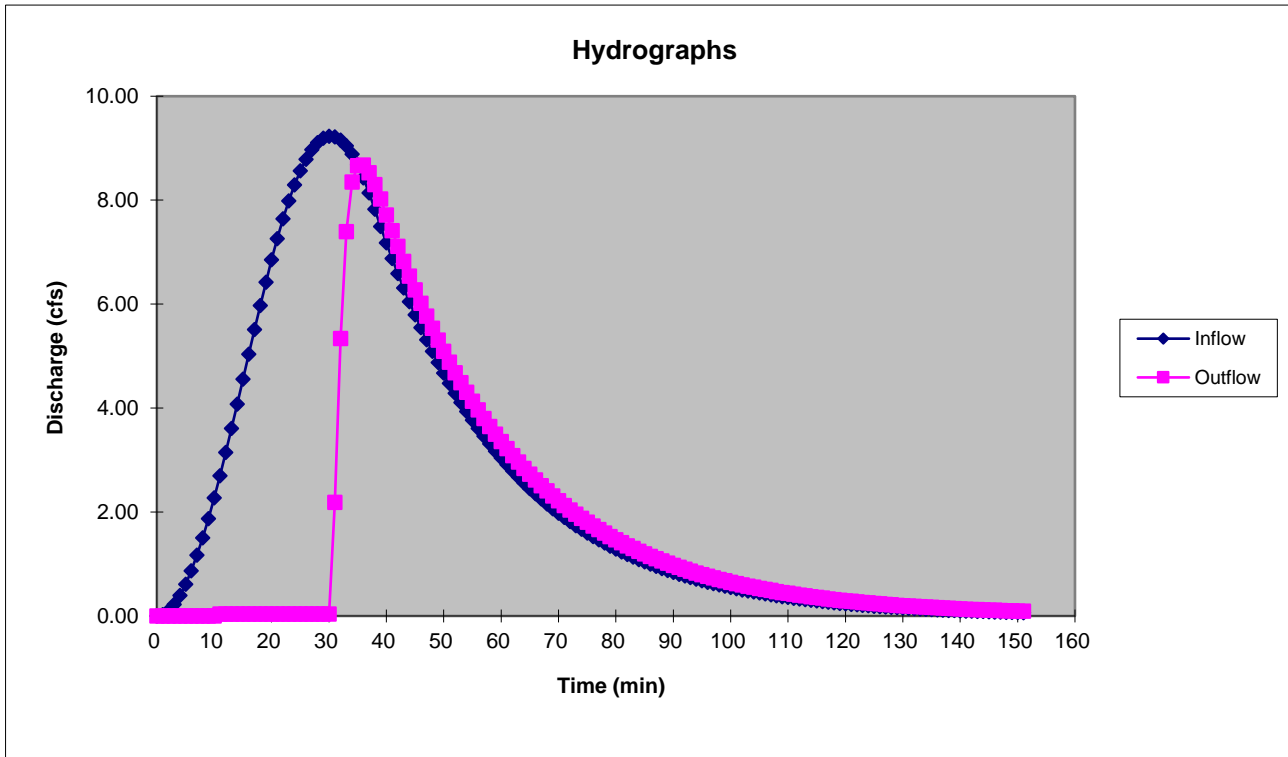
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 312.00 ft
Qp =	9.23 cfs	N =	1
Tp =	30.2 min	L =	20 ft
dT =	1.0 min	Cw =	3.00
<b>Stage-Storage Results:</b>		Zcr =	310.00 ft
Ks =	3547	<b>Skimmer Orifice:</b>	
b =	1.17	Number =	1.00 Ea
Z <sub>0</sub> =	308.0 ft (inv)	Diameter =	2.00 Inches
<b>Initial Water Level:</b>		Head =	1.58 inches
Z <sub>i</sub> =	308.00 ft		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	308.00	0.0	0.0	0.0		
1	0	0.00	308.00	0.0	0.0	0.00		
2	0	1.49	308.00	0.0	0.0	0.00		
3	0	7.44	308.01	0.0	0.0	0.00		
4	0	20.78	308.01	0.0	0.0	0.00		
5	1	44.33	308.02	0.0	0.0	0.00		
6	1	80.83	308.04	0.0	0.0	0.00		
7	1	132.87	308.06	0.0	0.0	0.00		
8	2	202.87	308.09	0.0	0.0	0.00		
9	2	293.07	308.12	0.0	0.0	0.00		
10	2	405.47	308.16	0.0	0.0	0.00		
11	3	541.85	308.20	0.0	0.0	0.04		
12	3	701.47	308.25	0.0	0.0	0.04		
13	4	887.82	308.31	0.0	0.0	0.04		
14	4	1101.86	308.37	0.0	0.0	0.04		
15	5	1344.22	308.44	0.0	0.0	0.04		
16	5	1615.27	308.51	0.0	0.0	0.04		
17	6	1915.04	308.59	0.0	0.0	0.04		
18	6	2243.26	308.68	0.0	0.0	0.04		
19	6	2599.34	308.77	0.0	0.0	0.04		
20	7	2982.42	308.86	0.0	0.0	0.04		
21	7	3391.33	308.96	0.0	0.0	0.04		
22	8	3824.61	309.07	0.0	0.0	0.04		
23	8	4280.55	309.17	0.0	0.0	0.04		
24	8	4757.21	309.29	0.0	0.0	0.04		
25	9	5252.40	309.40	0.0	0.0	0.04		
26	9	5763.74	309.52	0.0	0.0	0.04		
27	9	6288.69	309.63	0.0	0.0	0.04		
28	9	6824.55	309.75	0.0	0.0	0.04		
29	9	7368.49	309.87	0.0	0.0	0.04		
30	9	7917.62	309.99	0.0	0.0	0.04		
31	9	8468.98	310.11	2.2	2.1	0.04		
32	9	8890.68	310.20	5.3	5.3	0.04		
33	9	9119.61	310.25	7.4	7.4	0.04		
34	9	9218.57	310.27	8.3	8.3	0.04		
35	9	9250.83	310.27	8.7	8.6	0.04		
36	8	9251.68	310.27	8.7	8.6	0.04		
37	8	9237.11	310.27	8.5	8.5	0.04		
38	8	9213.83	310.27	8.3	8.3	0.04		
39	7	9185.26	310.26	8.0	8.0	0.04		
40	7	9153.66	310.25	7.7	7.7	0.04		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.67	ft	
Peak Stage =	310.33	ft	
Rise =	2.33	ft	
Peak Outflow =	11.64	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 312.00 ft	
Qp = 11.66 cfs	N = 1		
Tp = 40.0 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 310.00 ft	<b>Skimmer Orifice:</b>	
Ks = 3547		Number = 1	Ea
b = 1.17		Diameter = 2.00	Inches
Z <sub>0</sub> = 308.0 ft (inv)		Head = 1.58	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 308.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	308.00	0.0	0.0	0.0		
1	0	0.00	308.00	0.0	0.0	0.00		
2	0	1.08	308.00	0.0	0.0	0.00		
3	0	5.38	308.00	0.0	0.0	0.00		
4	0	15.03	308.01	0.0	0.0	0.00		
5	0	32.12	308.02	0.0	0.0	0.00		
6	1	58.70	308.03	0.0	0.0	0.00		
7	1	96.76	308.05	0.0	0.0	0.00		
8	1	148.23	308.07	0.0	0.0	0.00		
9	1	214.93	308.09	0.0	0.0	0.00		
10	2	298.60	308.12	0.0	0.0	0.00		
11	2	400.90	308.15	0.0	0.0	0.00		
12	2	523.33	308.19	0.0	0.0	0.04		
13	3	665.05	308.24	0.0	0.0	0.04		
14	3	829.58	308.29	0.0	0.0	0.04		
15	4	1018.04	308.34	0.0	0.0	0.04		
16	4	1231.41	308.40	0.0	0.0	0.04		
17	4	1470.51	308.47	0.0	0.0	0.04		
18	5	1736.02	308.54	0.0	0.0	0.04		
19	5	2028.44	308.62	0.0	0.0	0.04		
20	6	2348.11	308.70	0.0	0.0	0.04		
21	6	2695.20	308.79	0.0	0.0	0.04		
22	7	3069.71	308.88	0.0	0.0	0.04		
23	7	3471.47	308.98	0.0	0.0	0.04		
24	8	3900.16	309.08	0.0	0.0	0.04		
25	8	4355.27	309.19	0.0	0.0	0.04		
26	8	4836.14	309.30	0.0	0.0	0.04		
27	9	5341.94	309.42	0.0	0.0	0.04		
28	9	5871.72	309.54	0.0	0.0	0.04		
29	10	6424.33	309.66	0.0	0.0	0.04		
30	10	6998.52	309.79	0.0	0.0	0.04		
31	10	7592.90	309.92	0.0	0.0	0.04		
32	11	8205.94	310.05	0.8	0.7	0.04		
33	11	8792.86	310.18	4.5	4.5	0.04		
34	11	9168.85	310.26	7.9	7.8	0.04		
35	11	9358.25	310.30	9.8	9.7	0.04		
36	11	9445.78	310.32	10.7	10.6	0.04		
37	11	9487.88	310.32	11.1	11.1	0.04		
38	12	9510.47	310.33	11.4	11.3	0.04		
39	12	9523.85	310.33	11.5	11.5	0.04		
40	12	9531.79	310.33	11.6	11.6	0.04		
41	12	9535.69	310.33	11.6	11.6	0.04		
42	12	9536.02	310.33	11.6	11.6	0.04		
43	12	9532.98	310.33	11.6	11.6	0.04		
44	11	9526.65	310.33	11.5	11.5	0.04		
45	11	9517.08	310.33	11.4	11.4	0.04		
46	11	9504.32	310.33	11.3	11.3	0.04		

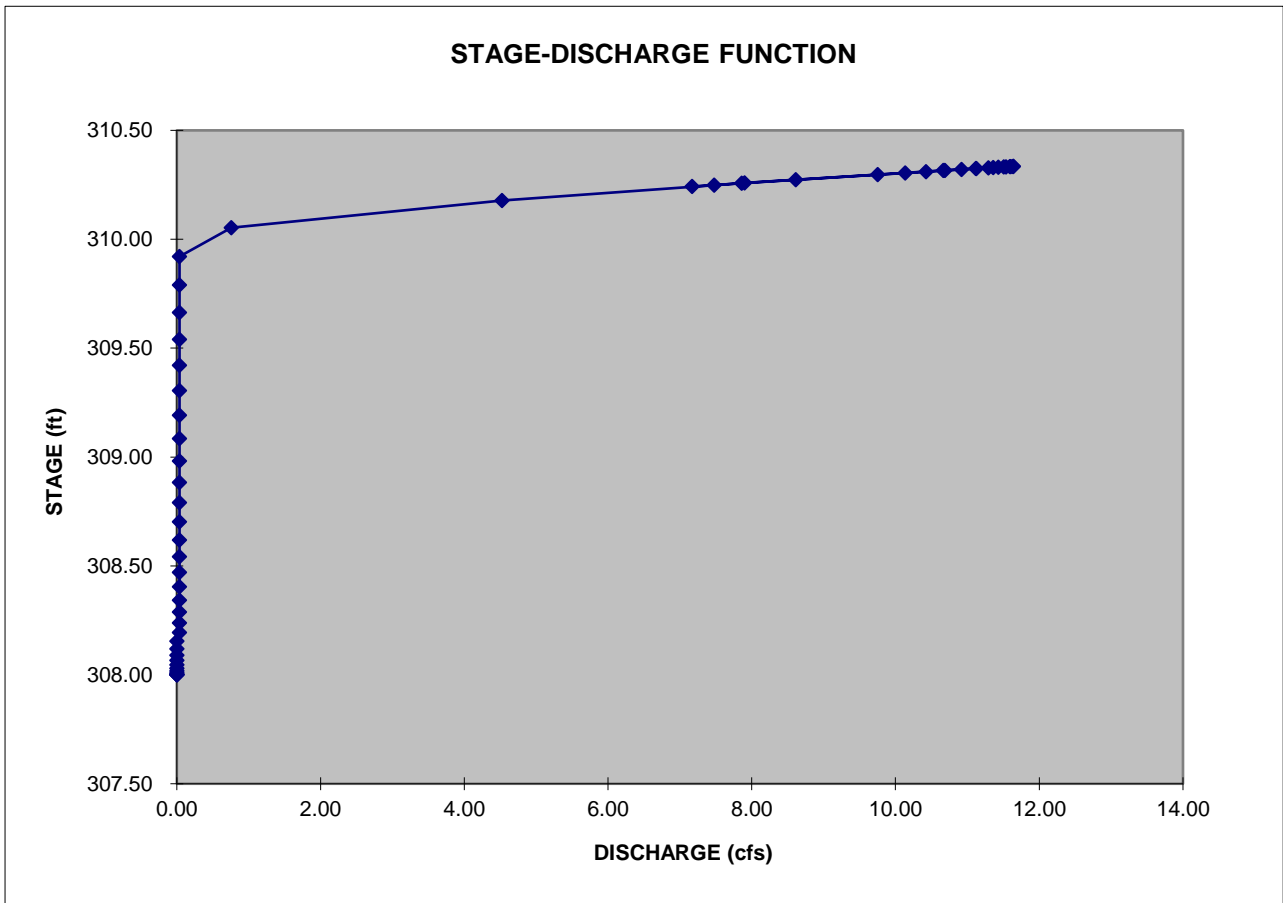
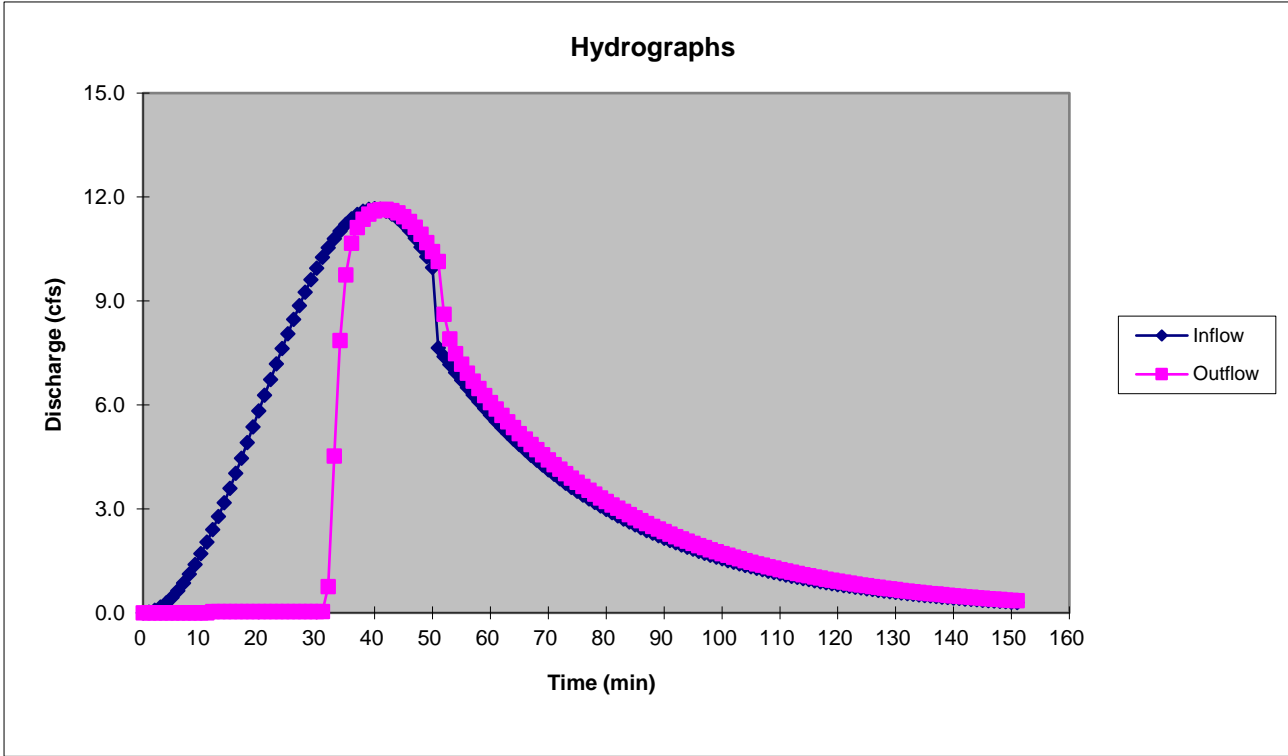
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	11	9488.44	310.32	11.1	11.1	0.04		
48	11	9469.50	310.32	10.9	10.9	0.04		
49	10	9447.57	310.32	10.7	10.6	0.04		
50	10	9422.75	310.31	10.4	10.4	0.04		
51	8	9395.12	310.30	10.1	10.1	0.04		
52	7	9245.72	310.27	8.6	8.6	0.04		
53	7	9172.96	310.26	7.9	7.9	0.04		
54	7	9128.72	310.25	7.5	7.4	0.04		
55	7	9096.12	310.24	7.2	7.1	0.04		
56	6	9068.64	310.24	6.9	6.9	0.04		
57	6	9043.61	310.23	6.7	6.6	0.04		
58	6	9019.92	310.23	6.5	6.4	0.04		
59	6	8997.09	310.22	6.3	6.2	0.04		
60	6	8974.91	310.22	6.1	6.0	0.04		
61	6	8953.28	310.21	5.9	5.8	0.04		
62	5	8932.14	310.21	5.7	5.7	0.04		
63	5	8911.47	310.20	5.5	5.5	0.04		
64	5	8891.25	310.20	5.3	5.3	0.04		
65	5	8871.46	310.19	5.2	5.1	0.04		
66	5	8852.10	310.19	5.0	5.0	0.04		
67	5	8833.16	310.19	4.9	4.8	0.04		
68	4	8814.63	310.18	4.7	4.7	0.04		
69	4	8796.49	310.18	4.6	4.5	0.04		
70	4	8778.73	310.17	4.4	4.4	0.04		
71	4	8761.36	310.17	4.3	4.2	0.04		
72	4	8744.36	310.17	4.1	4.1	0.04		
73	4	8727.73	310.16	4.0	4.0	0.04		
74	4	8711.45	310.16	3.9	3.8	0.04		
75	4	8695.51	310.16	3.8	3.7	0.04		
76	3	8679.92	310.15	3.6	3.6	0.04		
77	3	8664.66	310.15	3.5	3.5	0.04		
78	3	8649.72	310.15	3.4	3.4	0.04		
79	3	8635.10	310.14	3.3	3.3	0.04		
80	3	8620.79	310.14	3.2	3.2	0.04		
81	3	8606.79	310.14	3.1	3.1	0.04		
82	3	8593.09	310.14	3.0	3.0	0.04		
83	3	8579.67	310.13	2.9	2.9	0.04		
84	3	8566.55	310.13	2.8	2.8	0.04		
85	3	8553.70	310.13	2.7	2.7	0.04		
86	2	8541.12	310.12	2.7	2.6	0.04		
87	2	8528.81	310.12	2.6	2.5	0.04		
88	2	8516.76	310.12	2.5	2.5	0.04		
89	2	8504.97	310.12	2.4	2.4	0.04		
90	2	8493.42	310.11	2.3	2.3	0.04		
91	2	8482.13	310.11	2.3	2.2	0.04		
92	2	8471.07	310.11	2.2	2.2	0.04		
93	2	8460.24	310.11	2.1	2.1	0.04		
94	2	8449.64	310.10	2.1	2.0	0.04		
95	2	8439.27	310.10	2.0	2.0	0.04		
96	2	8429.11	310.10	1.9	1.9	0.04		
97	2	8419.17	310.10	1.9	1.8	0.04		
98	2	8409.44	310.10	1.8	1.8	0.04		
99	2	8399.91	310.09	1.8	1.7	0.04		
100	2	8390.59	310.09	1.7	1.7	0.04		
101	2	8381.46	310.09	1.7	1.6	0.04		
102	1	8372.52	310.09	1.6	1.6	0.04		
103	1	8363.77	310.09	1.6	1.5	0.04		
104	1	8355.21	310.08	1.5	1.5	0.04		
105	1	8346.82	310.08	1.5	1.4	0.04		
106	1	8338.61	310.08	1.4	1.4	0.04		
107	1	8330.57	310.08	1.4	1.3	0.04		
108	1	8322.70	310.08	1.3	1.3	0.04		
109	1	8315.00	310.08	1.3	1.3	0.04		
110	1	8307.46	310.07	1.2	1.2	0.04		
111	1	8300.07	310.07	1.2	1.2	0.04		
112	1	8292.84	310.07	1.2	1.1	0.04		
113	1	8285.76	310.07	1.1	1.1	0.04		
114	1	8278.83	310.07	1.1	1.1	0.04		
115	1	8272.04	310.07	1.1	1.0	0.04		
116	1	8265.39	310.07	1.0	1.0	0.04		
117	1	8258.88	310.06	1.0	1.0	0.04		
118	1	8252.51	310.06	1.0	0.9	0.04		
119	1	8246.27	310.06	0.9	0.9	0.04		
120	1	8240.15	310.06	0.9	0.9	0.04		
121	1	8234.17	310.06	0.9	0.8	0.04		
122	1	8228.31	310.06	0.9	0.8	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	8222.57	310.06	0.8	0.8	0.04		
124	1	8216.95	310.05	0.8	0.8	0.04		
125	1	8211.44	310.05	0.8	0.7	0.04		
126	1	8206.05	310.05	0.8	0.7	0.04		
127	1	8200.77	310.05	0.7	0.7	0.04		
128	1	8195.60	310.05	0.7	0.7	0.04		
129	1	8190.53	310.05	0.7	0.7	0.04		
130	1	8185.57	310.05	0.7	0.6	0.04		
131	1	8180.71	310.05	0.6	0.6	0.04		
132	1	8175.95	310.05	0.6	0.6	0.04		
133	1	8171.29	310.04	0.6	0.6	0.04		
134	1	8166.73	310.04	0.6	0.6	0.04		
135	0	8162.25	310.04	0.6	0.5	0.04		
136	0	8157.87	310.04	0.6	0.5	0.04		
137	0	8153.58	310.04	0.5	0.5	0.04		
138	0	8149.37	310.04	0.5	0.5	0.04		
139	0	8145.25	310.04	0.5	0.5	0.04		
140	0	8141.22	310.04	0.5	0.5	0.04		
141	0	8137.26	310.04	0.5	0.4	0.04		
142	0	8133.39	310.04	0.5	0.4	0.04		
143	0	8129.59	310.04	0.4	0.4	0.04		
144	0	8125.87	310.04	0.4	0.4	0.04		
145	0	8122.22	310.03	0.4	0.4	0.04		
146	0	8118.65	310.03	0.4	0.4	0.04		
147	0	8115.15	310.03	0.4	0.4	0.04		
148	0	8111.72	310.03	0.4	0.3	0.04		
149	0	8108.36	310.03	0.4	0.3	0.04		
150	0	8105.07	310.03	0.4	0.3	0.04		
151	0	8101.84	310.03	0.3	0.3	0.04		

# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.63	ft	
Peak Stage =	310.37	ft	
Rise =	2.37	ft	
Peak Outflow =	13.37	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 312.00 ft	
Qp = 13.39 cfs	N = 1		
Tp = 40.0 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 310.00 ft	<b>Skimmer Orifice:</b>	
Ks = 3547		Number = 1.00	Ea
b = 1.17		Diameter = 2.00	Inches
Z <sub>0</sub> = 308.0 ft (inv)		Head = 1.58	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 308.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	308.00	0.0	0.0	0.0		
1	0	0.00	308.00	0.0	0.0	0.00		
2	0	1.24	308.00	0.0	0.0	0.00		
3	0	6.17	308.00	0.0	0.0	0.00		
4	0	17.25	308.01	0.0	0.0	0.00		
5	1	36.88	308.02	0.0	0.0	0.00		
6	1	67.40	308.03	0.0	0.0	0.00		
7	1	111.10	308.05	0.0	0.0	0.00		
8	1	170.19	308.07	0.0	0.0	0.00		
9	2	246.77	308.10	0.0	0.0	0.00		
10	2	342.85	308.13	0.0	0.0	0.00		
11	2	460.30	308.17	0.0	0.0	0.04		
12	3	598.62	308.22	0.0	0.0	0.04		
13	3	761.68	308.27	0.0	0.0	0.04		
14	4	950.92	308.32	0.0	0.0	0.04		
15	4	1167.64	308.39	0.0	0.0	0.04		
16	5	1412.96	308.45	0.0	0.0	0.04		
17	5	1687.83	308.53	0.0	0.0	0.04		
18	6	1993.01	308.61	0.0	0.0	0.04		
19	6	2329.10	308.70	0.0	0.0	0.04		
20	7	2696.46	308.79	0.0	0.0	0.04		
21	7	3095.32	308.89	0.0	0.0	0.04		
22	8	3525.65	308.99	0.0	0.0	0.04		
23	8	3987.28	309.11	0.0	0.0	0.04		
24	9	4479.82	309.22	0.0	0.0	0.04		
25	9	5002.70	309.34	0.0	0.0	0.04		
26	10	5555.16	309.47	0.0	0.0	0.04		
27	10	6136.25	309.60	0.0	0.0	0.04		
28	11	6744.85	309.73	0.0	0.0	0.04		
29	11	7379.68	309.87	0.0	0.0	0.04		
30	11	8039.29	310.02	0.2	0.1	0.04		
31	12	8714.38	310.16	3.9	3.9	0.04		
32	12	9186.24	310.26	8.0	8.0	0.04		
33	12	9430.51	310.31	10.5	10.5	0.04		
34	13	9543.72	310.34	11.7	11.7	0.04		
35	13	9599.31	310.35	12.3	12.3	0.04		
36	13	9631.34	310.35	12.7	12.7	0.04		
37	13	9652.86	310.36	12.9	12.9	0.04		
38	13	9668.47	310.36	13.1	13.1	0.04		
39	13	9679.69	310.36	13.2	13.2	0.04		
40	13	9687.04	310.37	13.3	13.3	0.04		
41	13	9690.68	310.37	13.4	13.3	0.04		
42	13	9690.67	310.37	13.4	13.3	0.04		
43	13	9687.04	310.37	13.3	13.3	0.04		
44	13	9679.83	310.36	13.2	13.2	0.04		
45	13	9669.07	310.36	13.1	13.1	0.04		
46	13	9654.82	310.36	13.0	12.9	0.04		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	12	9637.14	310.36	12.8	12.7	0.04		
48	12	9616.10	310.35	12.5	12.5	0.04		
49	12	9591.78	310.35	12.3	12.2	0.04		
50	11	9564.28	310.34	11.9	11.9	0.04		
51	8	9533.70	310.33	11.6	11.6	0.04		
52	7	9295.48	310.28	9.1	9.1	0.04		
53	7	9192.84	310.26	8.1	8.1	0.04		
54	7	9137.05	310.25	7.6	7.5	0.04		
55	7	9099.70	310.24	7.2	7.2	0.04		
56	6	9070.21	310.24	6.9	6.9	0.04		
57	6	9044.31	310.23	6.7	6.7	0.04		
58	6	9020.24	310.23	6.5	6.4	0.04		
59	6	8997.24	310.22	6.3	6.2	0.04		
60	6	8974.98	310.22	6.1	6.0	0.04		
61	6	8953.31	310.21	5.9	5.8	0.04		
62	5	8932.15	310.21	5.7	5.7	0.04		
63	5	8911.47	310.20	5.5	5.5	0.04		
64	5	8891.25	310.20	5.3	5.3	0.04		
65	5	8871.46	310.19	5.2	5.1	0.04		
66	5	8852.10	310.19	5.0	5.0	0.04		
67	5	8833.16	310.19	4.9	4.8	0.04		
68	4	8814.63	310.18	4.7	4.7	0.04		
69	4	8796.49	310.18	4.6	4.5	0.04		
70	4	8778.73	310.17	4.4	4.4	0.04		
71	4	8761.36	310.17	4.3	4.2	0.04		
72	4	8744.36	310.17	4.1	4.1	0.04		
73	4	8727.73	310.16	4.0	4.0	0.04		
74	4	8711.45	310.16	3.9	3.8	0.04		
75	4	8695.51	310.16	3.8	3.7	0.04		
76	3	8679.92	310.15	3.6	3.6	0.04		
77	3	8664.66	310.15	3.5	3.5	0.04		
78	3	8649.72	310.15	3.4	3.4	0.04		
79	3	8635.10	310.14	3.3	3.3	0.04		
80	3	8620.79	310.14	3.2	3.2	0.04		
81	3	8606.79	310.14	3.1	3.1	0.04		
82	3	8593.09	310.14	3.0	3.0	0.04		
83	3	8579.67	310.13	2.9	2.9	0.04		
84	3	8566.55	310.13	2.8	2.8	0.04		
85	3	8553.70	310.13	2.7	2.7	0.04		
86	2	8541.12	310.12	2.7	2.6	0.04		
87	2	8528.81	310.12	2.6	2.5	0.04		
88	2	8516.76	310.12	2.5	2.5	0.04		
89	2	8504.97	310.12	2.4	2.4	0.04		
90	2	8493.42	310.11	2.3	2.3	0.04		
91	2	8482.13	310.11	2.3	2.2	0.04		
92	2	8471.07	310.11	2.2	2.2	0.04		
93	2	8460.24	310.11	2.1	2.1	0.04		
94	2	8449.64	310.10	2.1	2.0	0.04		
95	2	8439.27	310.10	2.0	2.0	0.04		
96	2	8429.11	310.10	1.9	1.9	0.04		
97	2	8419.17	310.10	1.9	1.8	0.04		
98	2	8409.44	310.10	1.8	1.8	0.04		
99	2	8399.91	310.09	1.8	1.7	0.04		
100	2	8390.59	310.09	1.7	1.7	0.04		
101	2	8381.46	310.09	1.7	1.6	0.04		
102	1	8372.52	310.09	1.6	1.6	0.04		
103	1	8363.77	310.09	1.6	1.5	0.04		
104	1	8355.21	310.08	1.5	1.5	0.04		
105	1	8346.82	310.08	1.5	1.4	0.04		
106	1	8338.61	310.08	1.4	1.4	0.04		
107	1	8330.57	310.08	1.4	1.3	0.04		
108	1	8322.70	310.08	1.3	1.3	0.04		
109	1	8315.00	310.08	1.3	1.3	0.04		
110	1	8307.46	310.07	1.2	1.2	0.04		
111	1	8300.07	310.07	1.2	1.2	0.04		
112	1	8292.84	310.07	1.2	1.1	0.04		
113	1	8285.76	310.07	1.1	1.1	0.04		
114	1	8278.83	310.07	1.1	1.1	0.04		
115	1	8272.04	310.07	1.1	1.0	0.04		
116	1	8265.39	310.07	1.0	1.0	0.04		
117	1	8258.88	310.06	1.0	1.0	0.04		
118	1	8252.51	310.06	1.0	0.9	0.04		
119	1	8246.27	310.06	0.9	0.9	0.04		
120	1	8240.15	310.06	0.9	0.9	0.04		
121	1	8234.17	310.06	0.9	0.8	0.04		
122	1	8228.31	310.06	0.9	0.8	0.04		

**OUTPUT**

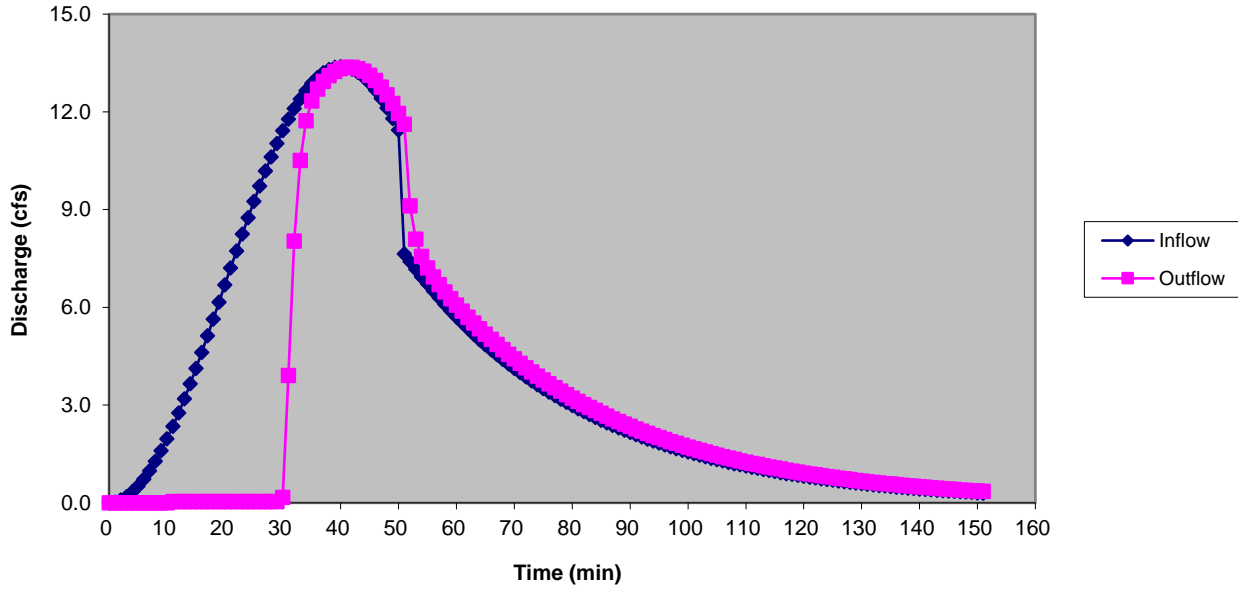
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	8222.57	310.06	0.8	0.8	0.04		
124	1	8216.95	310.05	0.8	0.8	0.04		
125	1	8211.44	310.05	0.8	0.7	0.04		
126	1	8206.05	310.05	0.8	0.7	0.04		
127	1	8200.77	310.05	0.7	0.7	0.04		
128	1	8195.60	310.05	0.7	0.7	0.04		
129	1	8190.53	310.05	0.7	0.7	0.04		
130	1	8185.57	310.05	0.7	0.6	0.04		
131	1	8180.71	310.05	0.6	0.6	0.04		
132	1	8175.95	310.05	0.6	0.6	0.04		
133	1	8171.29	310.04	0.6	0.6	0.04		
134	1	8166.73	310.04	0.6	0.6	0.04		
135	0	8162.25	310.04	0.6	0.5	0.04		
136	0	8157.87	310.04	0.6	0.5	0.04		
137	0	8153.58	310.04	0.5	0.5	0.04		
138	0	8149.37	310.04	0.5	0.5	0.04		
139	0	8145.25	310.04	0.5	0.5	0.04		
140	0	8141.22	310.04	0.5	0.5	0.04		
141	0	8137.26	310.04	0.5	0.4	0.04		
142	0	8133.39	310.04	0.5	0.4	0.04		
143	0	8129.59	310.04	0.4	0.4	0.04		
144	0	8125.87	310.04	0.4	0.4	0.04		
145	0	8122.22	310.03	0.4	0.4	0.04		
146	0	8118.65	310.03	0.4	0.4	0.04		
147	0	8115.15	310.03	0.4	0.4	0.04		
148	0	8111.72	310.03	0.4	0.3	0.04		
149	0	8108.36	310.03	0.4	0.3	0.04		
150	0	8105.07	310.03	0.4	0.3	0.04		
151	0	8101.84	310.03	0.3	0.3	0.04		



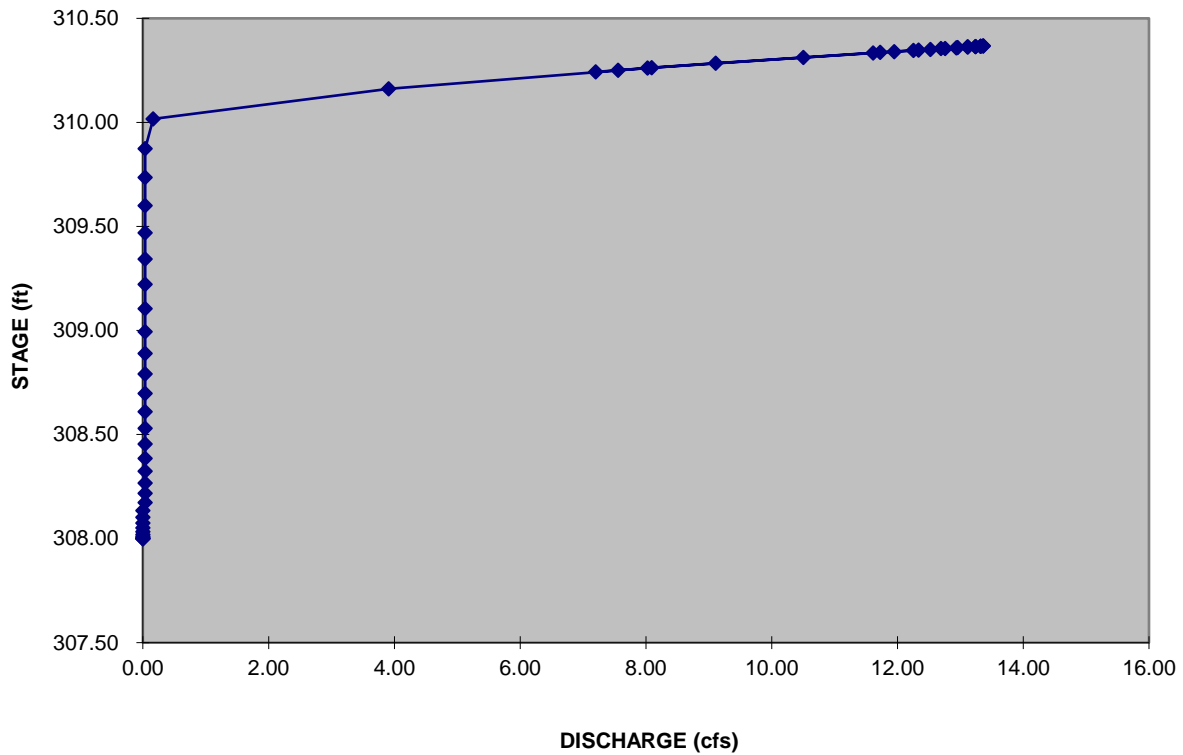
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-3  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID: ST-3

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	3.59 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	0.00 ac.	0.35
Undisturbed grassy area	0.00 ac.	3.59 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>3.59 ac.</b>	<b>3.59 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.25</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		<i>K Values</i>	
Length of flow path =	661 feet		Overland on grassed surfaces:	2.0
Height of watershed =	10 feet		Overland on paved surfaces:	0.4
Calculated t(c) =	5.8 minutes		Channel in natural channels:	1.0
Minimum t(c) =	5.0 minutes		Channel in mixed urban setting:	1.1
Time of concentration =	5.8 minutes		Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.54 in/hr
10	195	22	7.01 in/hr
25	232	23	8.05 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.54 in/hr	9.0 cfs	4.97 cfs
10-year storm	7.01 in/hr	11.3 cfs	6.29 cfs
25-year storm	8.05 in/hr	13.0 cfs	7.22 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	100
Bottom width =	50
Sediment depth =	2.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	2.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	4
Top of trap length =	124
Top of trap width =	74
Bottom elevation =	312

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	6462 cu. ft.	11890 cu. ft.
Sediment surface area =	3680 sq. ft.	6940 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	2
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.32 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.68 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 6,462 cubic feet

24 HOURS	Best Option =	1 - 3 inch Skimmers with a 2.4 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.9 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.6 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.2 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

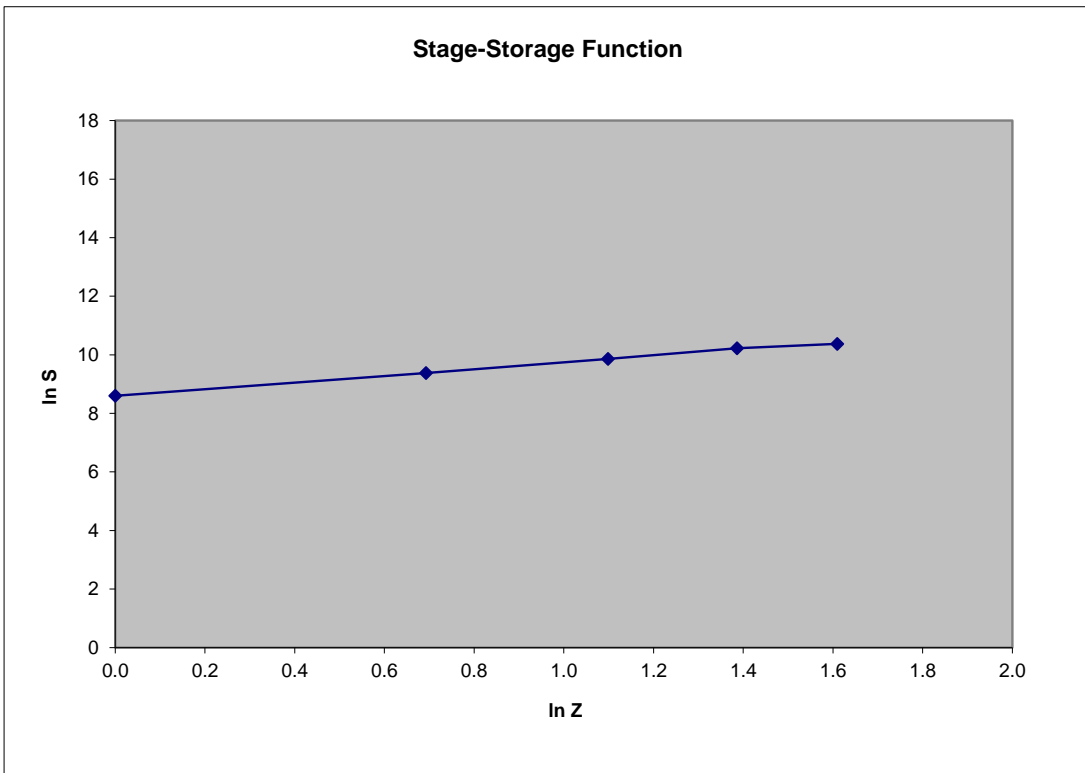
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
312	4979		0	0			
313	5881	5430	5430	1	8.5997	0.0000	1.00
314	6840	6360.5	11790.5	2	9.3750	0.6931	1.98
315	7856	7348	19138.5	3	9.8595	1.0986	3.04
316	8928	8392	27530.5	4	10.2230	1.3863	4.19
317	0	4464	31994.5	5	10.3733	1.6094	4.79

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.13$  and  $K_s = 5455$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	3.59 ac	<b>Use Kirpich Equation:</b> Tc = 5.8 min
Disturbed Area =	3.59 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.54 in/hr i <sub>10</sub> = 7.01 in/hr i <sub>25</sub> = 8.05 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	661 ft	
Vertical Fall =	10 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 9.0 cfs Q <sub>10</sub> = 11.3 cfs Q <sub>25</sub> = 13.0 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 30.5 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 40.3 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 43.2 minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	314.21	ft
	Rise =	2.21	ft
Freeboard =		1.79	ft
	Peak Outflow =	5.68	cfs

INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft
Qp =	8.95	cfs	N =
	30.5	min	L =
	1.0	min	Cw =
<b>Stage-Storage Results:</b>		Zcr =	314.00
Ks =	5455		
b =	1.13		
Z <sub>0</sub> =	312.0	ft (inv)	
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	312.00	ft	
			<b>Skimmer Orifice:</b>
			Number =
			1.00
			Ea
			Diameter =
			2.00
			Inches
			Head =
			1.56
			inches

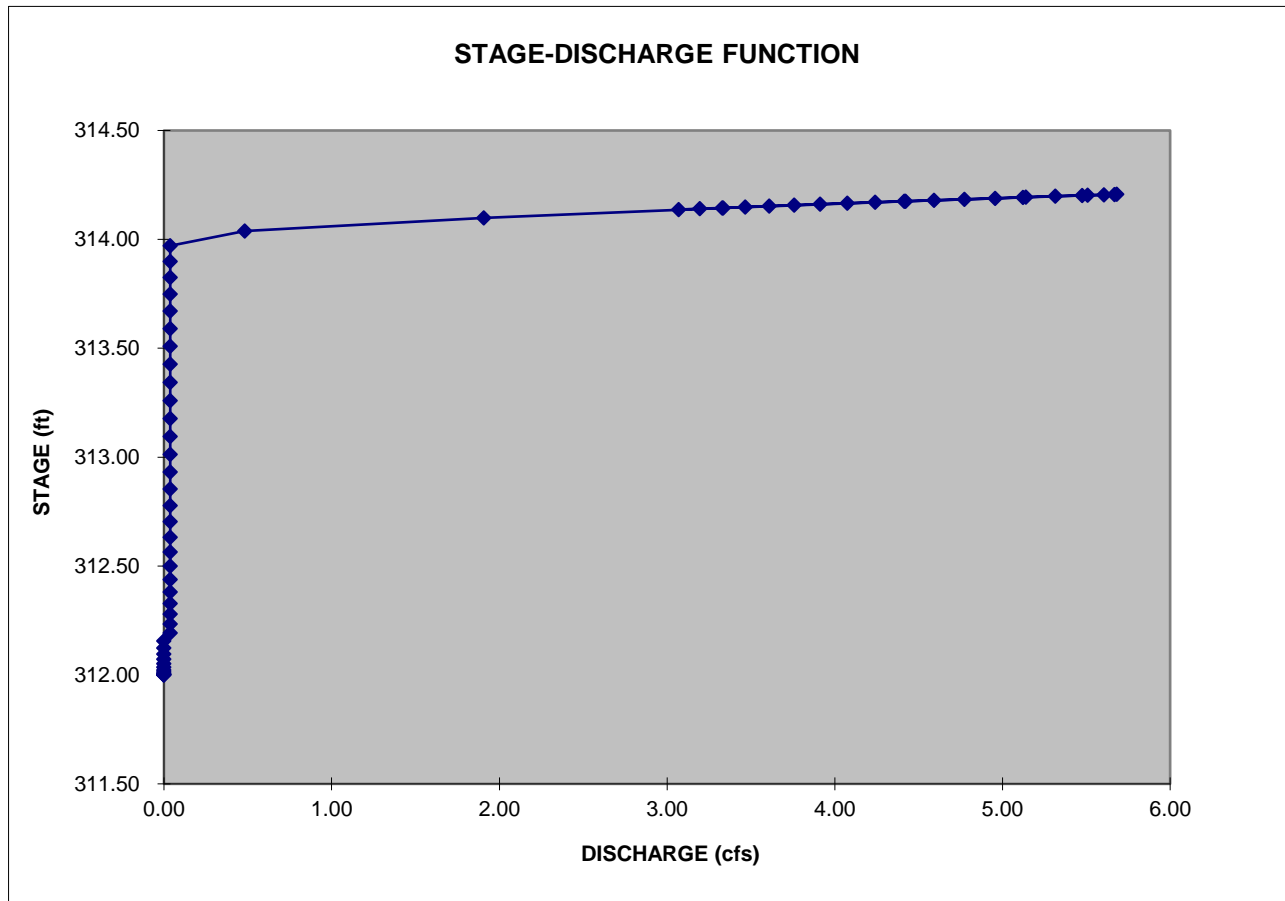
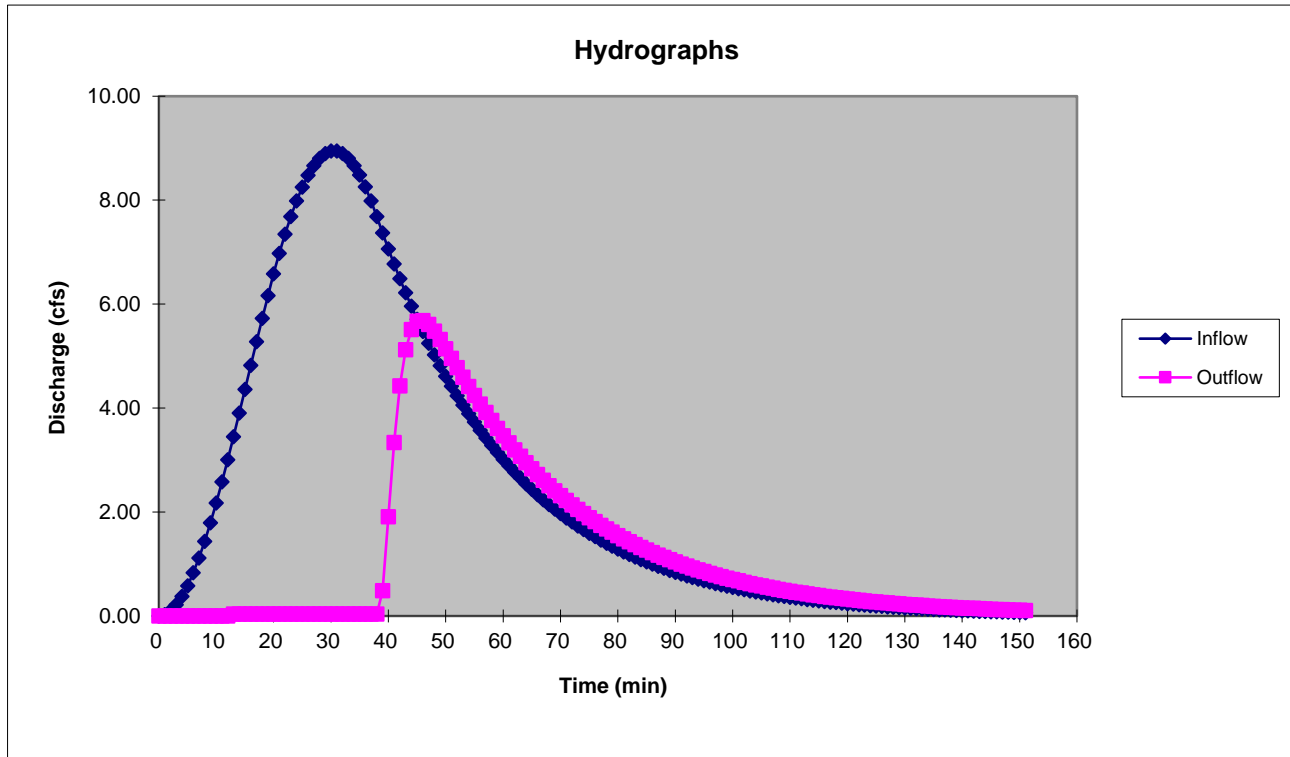
OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.42	312.00	0.0	0.0	0.00		
3	0	7.10	312.00	0.0	0.0	0.00		
4	0	19.82	312.01	0.0	0.0	0.00		
5	1	42.28	312.01	0.0	0.0	0.00		
6	1	77.11	312.02	0.0	0.0	0.00		
7	1	126.77	312.04	0.0	0.0	0.00		
8	1	193.59	312.05	0.0	0.0	0.00		
9	2	279.70	312.07	0.0	0.0	0.00		
10	2	387.04	312.10	0.0	0.0	0.00		
11	3	517.32	312.12	0.0	0.0	0.00		
12	3	671.99	312.16	0.0	0.0	0.00		
13	3	852.27	312.19	0.0	0.0	0.04		
14	4	1056.86	312.23	0.0	0.0	0.04		
15	4	1288.65	312.28	0.0	0.0	0.04		
16	5	1547.99	312.33	0.0	0.0	0.04		
17	5	1834.98	312.38	0.0	0.0	0.04		
18	6	2149.38	312.44	0.0	0.0	0.04		
19	6	2490.69	312.50	0.0	0.0	0.04		
20	7	2858.11	312.56	0.0	0.0	0.04		
21	7	3250.57	312.63	0.0	0.0	0.04		
22	7	3666.74	312.70	0.0	0.0	0.04		
23	8	4105.02	312.78	0.0	0.0	0.04		
24	8	4563.60	312.85	0.0	0.0	0.04		
25	8	5040.43	312.93	0.0	0.0	0.04		
26	8	5533.29	313.01	0.0	0.0	0.04		
27	9	6039.76	313.09	0.0	0.0	0.04		
28	9	6557.32	313.18	0.0	0.0	0.04		
29	9	7083.29	313.26	0.0	0.0	0.04		
30	9	7614.93	313.34	0.0	0.0	0.04		
31	9	8149.41	313.43	0.0	0.0	0.04		
32	9	8683.91	313.51	0.0	0.0	0.04		
33	9	9215.57	313.59	0.0	0.0	0.04		
34	9	9741.59	313.67	0.0	0.0	0.04		
35	8	10259.21	313.75	0.0	0.0	0.04		
36	8	10765.76	313.83	0.0	0.0	0.04		
37	8	11258.71	313.90	0.0	0.0	0.04		
38	8	11735.65	313.97	0.0	0.0	0.04		
39	7	12194.35	314.04	0.5	0.4	0.04		
40	7	12607.68	314.10	1.9	1.9	0.04		





# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.68	ft	
Peak Stage =	314.32	ft	
Rise =	2.32	ft	
Peak Outflow =	10.77	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	316.00 ft
Qp = 11.32 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 40.3 min	L = 20 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Head = 1.56 inches	
Ks = 5455			
b = 1.13			
Z <sub>0</sub> = 312.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 312.00 ft			

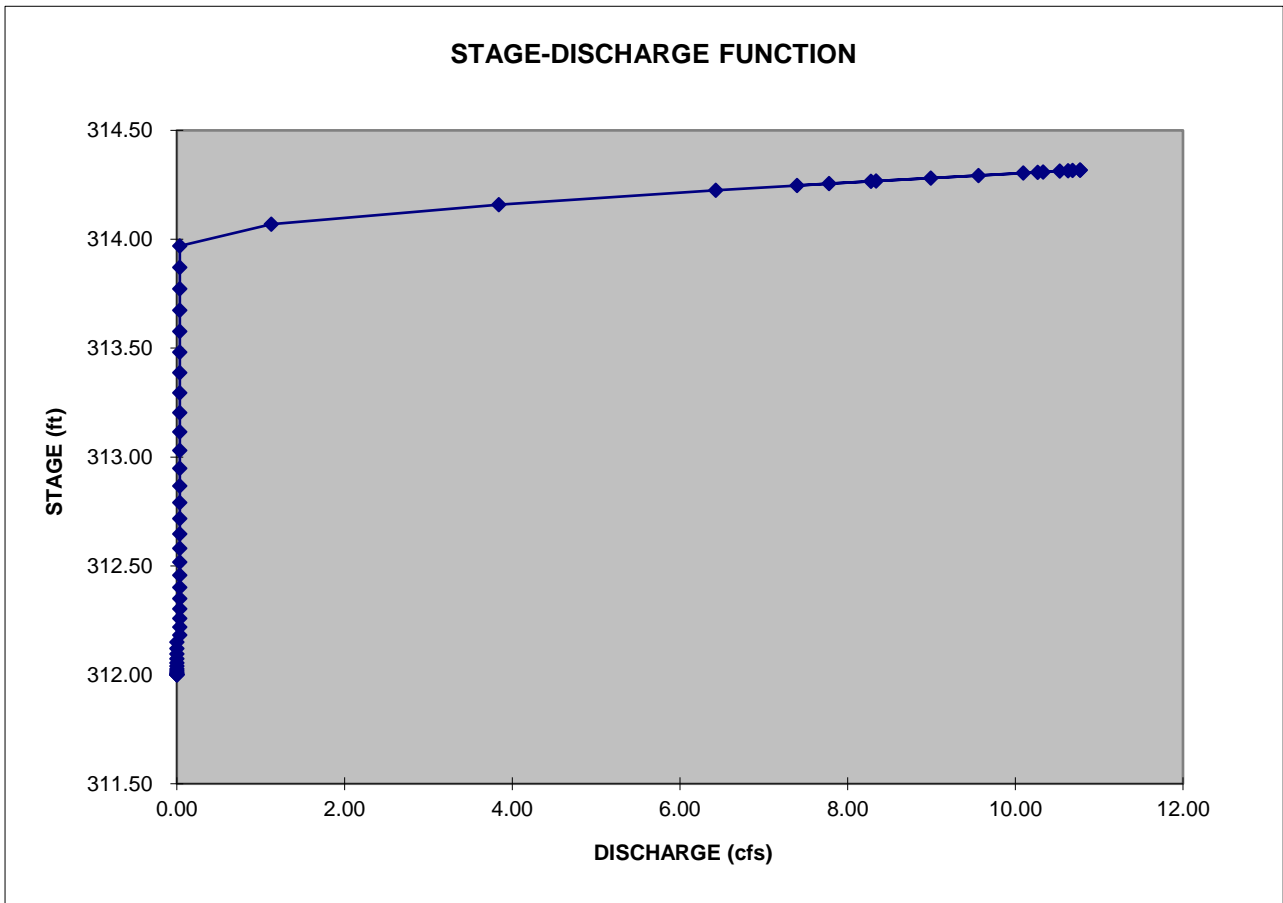
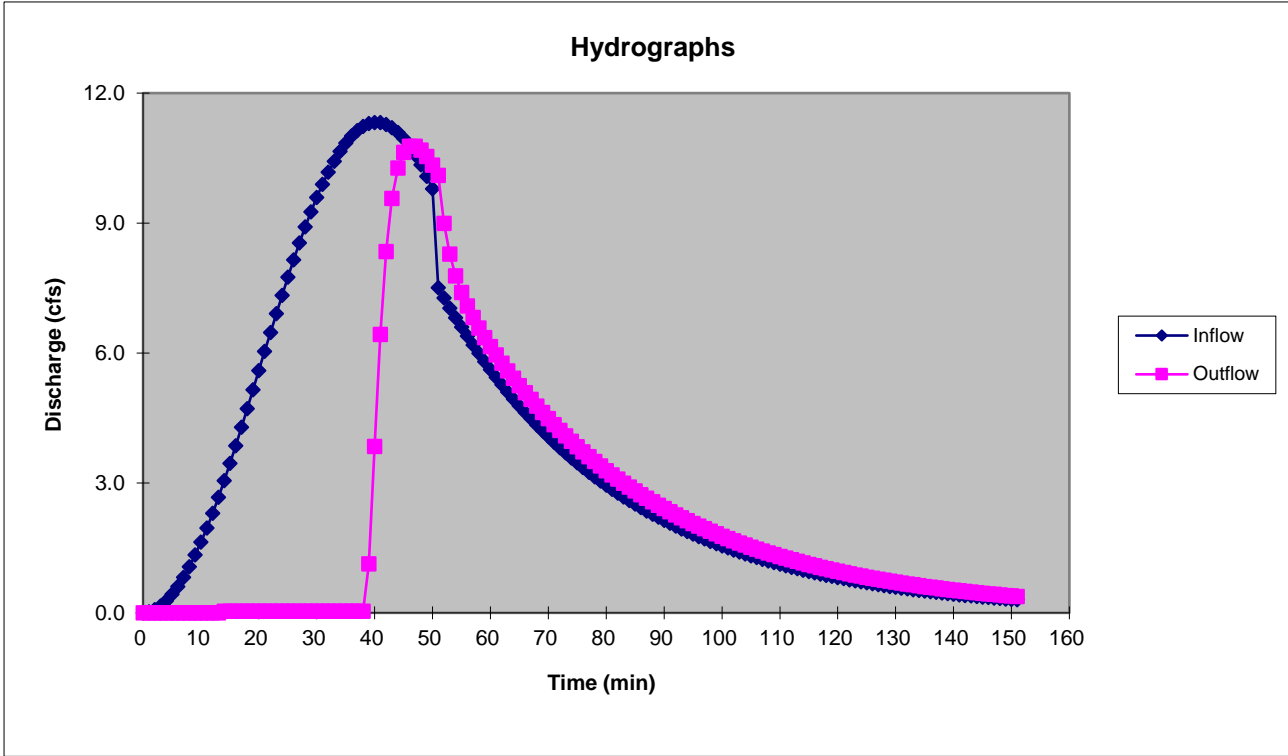
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.03	312.00	0.0	0.0	0.00		
3	0	5.15	312.00	0.0	0.0	0.00		
4	0	14.38	312.01	0.0	0.0	0.00		
5	0	30.75	312.01	0.0	0.0	0.00		
6	1	56.20	312.02	0.0	0.0	0.00		
7	1	92.65	312.03	0.0	0.0	0.00		
8	1	141.93	312.04	0.0	0.0	0.00		
9	1	205.81	312.05	0.0	0.0	0.00		
10	2	285.95	312.07	0.0	0.0	0.00		
11	2	383.95	312.10	0.0	0.0	0.00		
12	2	501.25	312.12	0.0	0.0	0.00		
13	3	639.22	312.15	0.0	0.0	0.00		
14	3	799.07	312.18	0.0	0.0	0.04		
15	3	979.66	312.22	0.0	0.0	0.04		
16	4	1184.17	312.26	0.0	0.0	0.04		
17	4	1413.42	312.30	0.0	0.0	0.04		
18	5	1668.07	312.35	0.0	0.0	0.04		
19	5	1948.60	312.40	0.0	0.0	0.04		
20	6	2255.37	312.46	0.0	0.0	0.04		
21	6	2588.57	312.52	0.0	0.0	0.04		
22	6	2948.22	312.58	0.0	0.0	0.04		
23	7	3334.18	312.65	0.0	0.0	0.04		
24	7	3746.16	312.72	0.0	0.0	0.04		
25	8	4183.71	312.79	0.0	0.0	0.04		
26	8	4646.22	312.87	0.0	0.0	0.04		
27	9	5132.94	312.95	0.0	0.0	0.04		
28	9	5642.95	313.03	0.0	0.0	0.04		
29	9	6175.21	313.12	0.0	0.0	0.04		
30	10	6728.53	313.20	0.0	0.0	0.04		
31	10	7301.62	313.29	0.0	0.0	0.04		
32	10	7893.03	313.39	0.0	0.0	0.04		
33	10	8501.22	313.48	0.0	0.0	0.04		
34	11	9124.57	313.58	0.0	0.0	0.04		
35	11	9761.32	313.67	0.0	0.0	0.04		
36	11	10409.66	313.77	0.0	0.0	0.04		
37	11	11067.70	313.87	0.0	0.0	0.04		
38	11	11733.51	313.97	0.0	0.0	0.04		
39	11	12405.09	314.07	1.1	1.1	0.04		
40	11	13014.92	314.16	3.8	3.8	0.04		
41	11	13463.78	314.22	6.4	6.4	0.04		
42	11	13756.90	314.27	8.3	8.3	0.04		
43	11	13932.92	314.29	9.6	9.5	0.04		
44	11	14031.10	314.31	10.3	10.2	0.04		
45	11	14080.48	314.31	10.6	10.6	0.04		
46	11	14099.76	314.32	10.8	10.7	0.04		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	11	14100.13	314.32	10.8	10.7	0.04		
48	10	14088.08	314.32	10.7	10.6	0.04		
49	10	14067.32	314.31	10.5	10.5	0.04		
50	10	14039.96	314.31	10.3	10.3	0.04		
51	8	14007.23	314.30	10.1	10.1	0.04		
52	7	13851.71	314.28	9.0	9.0	0.04		
53	7	13748.09	314.27	8.3	8.2	0.04		
54	7	13673.34	314.26	7.8	7.7	0.04		
55	7	13615.29	314.25	7.4	7.4	0.04		
56	6	13567.20	314.24	7.1	7.0	0.04		
57	6	13525.28	314.23	6.8	6.8	0.04		
58	6	13487.30	314.23	6.6	6.5	0.04		
59	6	13451.95	314.22	6.4	6.3	0.04		
60	6	13418.47	314.22	6.1	6.1	0.04		
61	5	13386.37	314.21	6.0	5.9	0.04		
62	5	13355.37	314.21	5.8	5.7	0.04		
63	5	13325.27	314.20	5.6	5.5	0.04		
64	5	13295.98	314.20	5.4	5.4	0.04		
65	5	13267.40	314.20	5.2	5.2	0.04		
66	5	13239.49	314.19	5.1	5.0	0.04		
67	4	13212.21	314.19	4.9	4.9	0.04		
68	4	13185.54	314.18	4.8	4.7	0.04		
69	4	13159.44	314.18	4.6	4.6	0.04		
70	4	13133.90	314.18	4.5	4.4	0.04		
71	4	13108.90	314.17	4.3	4.3	0.04		
72	4	13084.44	314.17	4.2	4.2	0.04		
73	4	13060.49	314.17	4.1	4.0	0.04		
74	4	13037.06	314.16	4.0	3.9	0.04		
75	3	13014.11	314.16	3.8	3.8	0.04		
76	3	12991.65	314.16	3.7	3.7	0.04		
77	3	12969.66	314.15	3.6	3.6	0.04		
78	3	12948.13	314.15	3.5	3.5	0.04		
79	3	12927.06	314.15	3.4	3.3	0.04		
80	3	12906.43	314.14	3.3	3.2	0.04		
81	3	12886.23	314.14	3.2	3.1	0.04		
82	3	12866.45	314.14	3.1	3.0	0.04		
83	3	12847.09	314.13	3.0	3.0	0.04		
84	3	12828.14	314.13	2.9	2.9	0.04		
85	3	12809.58	314.13	2.8	2.8	0.04		
86	2	12791.41	314.13	2.7	2.7	0.04		
87	2	12773.62	314.12	2.6	2.6	0.04		
88	2	12756.20	314.12	2.6	2.5	0.04		
89	2	12739.15	314.12	2.5	2.4	0.04		
90	2	12722.45	314.12	2.4	2.4	0.04		
91	2	12706.10	314.11	2.3	2.3	0.04		
92	2	12690.09	314.11	2.3	2.2	0.04		
93	2	12674.41	314.11	2.2	2.2	0.04		
94	2	12659.06	314.11	2.1	2.1	0.04		
95	2	12644.03	314.10	2.1	2.0	0.04		
96	2	12629.32	314.10	2.0	2.0	0.04		
97	2	12614.91	314.10	1.9	1.9	0.04		
98	2	12600.79	314.10	1.9	1.8	0.04		
99	2	12586.98	314.10	1.8	1.8	0.04		
100	2	12573.44	314.09	1.8	1.7	0.04		
101	1	12560.19	314.09	1.7	1.7	0.04		
102	1	12547.22	314.09	1.7	1.6	0.04		
103	1	12534.51	314.09	1.6	1.6	0.04		
104	1	12522.07	314.09	1.6	1.5	0.04		
105	1	12509.88	314.08	1.5	1.5	0.04		
106	1	12497.94	314.08	1.5	1.4	0.04		
107	1	12486.26	314.08	1.4	1.4	0.04		
108	1	12474.81	314.08	1.4	1.3	0.04		
109	1	12463.60	314.08	1.3	1.3	0.04		
110	1	12452.62	314.08	1.3	1.3	0.04		
111	1	12441.87	314.07	1.3	1.2	0.04		
112	1	12431.33	314.07	1.2	1.2	0.04		
113	1	12421.02	314.07	1.2	1.1	0.04		
114	1	12410.92	314.07	1.1	1.1	0.04		
115	1	12401.02	314.07	1.1	1.1	0.04		
116	1	12391.33	314.07	1.1	1.0	0.04		
117	1	12381.83	314.07	1.0	1.0	0.04		
118	1	12372.53	314.06	1.0	1.0	0.04		
119	1	12363.42	314.06	1.0	0.9	0.04		
120	1	12354.50	314.06	1.0	0.9	0.04		
121	1	12345.76	314.06	0.9	0.9	0.04		
122	1	12337.20	314.06	0.9	0.9	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	12328.81	314.06	0.9	0.8	0.04		
124	1	12320.60	314.06	0.8	0.8	0.04		
125	1	12312.55	314.06	0.8	0.8	0.04		
126	1	12304.66	314.05	0.8	0.8	0.04		
127	1	12296.94	314.05	0.8	0.7	0.04		
128	1	12289.37	314.05	0.8	0.7	0.04		
129	1	12281.96	314.05	0.7	0.7	0.04		
130	1	12274.69	314.05	0.7	0.7	0.04		
131	1	12267.57	314.05	0.7	0.6	0.04		
132	1	12260.60	314.05	0.7	0.6	0.04		
133	1	12253.77	314.05	0.6	0.6	0.04		
134	1	12247.07	314.05	0.6	0.6	0.04		
135	1	12240.51	314.04	0.6	0.6	0.04		
136	0	12234.09	314.04	0.6	0.6	0.04		
137	0	12227.79	314.04	0.6	0.5	0.04		
138	0	12221.61	314.04	0.6	0.5	0.04		
139	0	12215.57	314.04	0.5	0.5	0.04		
140	0	12209.64	314.04	0.5	0.5	0.04		
141	0	12203.83	314.04	0.5	0.5	0.04		
142	0	12198.14	314.04	0.5	0.5	0.04		
143	0	12192.56	314.04	0.5	0.4	0.04		
144	0	12187.09	314.04	0.5	0.4	0.04		
145	0	12181.73	314.04	0.4	0.4	0.04		
146	0	12176.48	314.04	0.4	0.4	0.04		
147	0	12171.33	314.03	0.4	0.4	0.04		
148	0	12166.28	314.03	0.4	0.4	0.04		
149	0	12161.33	314.03	0.4	0.4	0.04		
150	0	12156.48	314.03	0.4	0.4	0.04		
151	0	12151.73	314.03	0.4	0.3	0.04		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	314.35	ft
	Rise =	2.35	ft
	Peak Outflow =	12.71	cfs
Freeboard =	1.65	ft	

INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	
Qp =	13.00 cfs	N =	1
Tp =	40.3 min	L =	20 ft
dT =	1.0 min	Cw =	3.00
<b>Stage-Storage Results:</b>		Zcr =	314.00 ft
Ks =	5455	<b>Top of Dam:</b> 316.00 ft	
b =	1.13	<b>Skimmer Orifice:</b>	
Z <sub>0</sub> =	312.0 ft (inv)	Number =	1.00 Ea
<b>Initial Water Level:</b>		Diameter =	2.00 Inches
Z <sub>i</sub> =	312.00 ft	Head =	1.56 inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.18	312.00	0.0	0.0	0.00		
3	0	5.91	312.00	0.0	0.0	0.00		
4	0	16.52	312.01	0.0	0.0	0.00		
5	0	35.31	312.01	0.0	0.0	0.00		
6	1	64.54	312.02	0.0	0.0	0.00		
7	1	106.40	312.03	0.0	0.0	0.00		
8	1	163.00	312.04	0.0	0.0	0.00		
9	2	236.36	312.06	0.0	0.0	0.00		
10	2	328.41	312.08	0.0	0.0	0.00		
11	2	440.95	312.11	0.0	0.0	0.00		
12	3	575.67	312.14	0.0	0.0	0.00		
13	3	734.12	312.17	0.0	0.0	0.04		
14	3	915.47	312.21	0.0	0.0	0.04		
15	4	1123.20	312.25	0.0	0.0	0.04		
16	4	1358.41	312.29	0.0	0.0	0.04		
17	5	1622.03	312.34	0.0	0.0	0.04		
18	5	1914.81	312.40	0.0	0.0	0.04		
19	6	2237.32	312.45	0.0	0.0	0.04		
20	6	2589.97	312.52	0.0	0.0	0.04		
21	7	2972.97	312.58	0.0	0.0	0.04		
22	7	3386.34	312.66	0.0	0.0	0.04		
23	8	3829.94	312.73	0.0	0.0	0.04		
24	8	4303.42	312.81	0.0	0.0	0.04		
25	9	4806.26	312.89	0.0	0.0	0.04		
26	9	5337.77	312.98	0.0	0.0	0.04		
27	10	5897.08	313.07	0.0	0.0	0.04		
28	10	6483.14	313.17	0.0	0.0	0.04		
29	11	7094.76	313.26	0.0	0.0	0.04		
30	11	7730.56	313.36	0.0	0.0	0.04		
31	11	8389.06	313.46	0.0	0.0	0.04		
32	12	9068.61	313.57	0.0	0.0	0.04		
33	12	9767.44	313.67	0.0	0.0	0.04		
34	12	10483.65	313.78	0.0	0.0	0.04		
35	12	11215.27	313.89	0.0	0.0	0.04		
36	13	11960.20	314.00	0.0	0.0	0.04		
37	13	12715.58	314.11	2.4	2.3	0.04		
38	13	13340.28	314.21	5.7	5.6	0.04		
39	13	13773.69	314.27	8.5	8.4	0.04		
40	13	14044.56	314.31	10.4	10.3	0.04		
41	13	14202.67	314.33	11.5	11.5	0.04		
42	13	14290.00	314.35	12.2	12.2	0.04		
43	13	14334.66	314.35	12.5	12.5	0.04		
44	13	14353.51	314.35	12.7	12.7	0.04		
45	13	14356.13	314.35	12.7	12.7	0.04		
46	12	14347.80	314.35	12.7	12.6	0.04		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	12	14331.43	314.35	12.5	12.5	0.04		
48	12	14308.60	314.35	12.3	12.3	0.04		
49	12	14280.24	314.34	12.1	12.1	0.04		
50	11	14246.87	314.34	11.9	11.8	0.04		
51	8	14208.87	314.33	11.6	11.5	0.04		
52	7	13963.94	314.30	9.8	9.7	0.04		
53	7	13812.80	314.28	8.7	8.7	0.04		
54	7	13711.52	314.26	8.0	8.0	0.04		
55	7	13638.17	314.25	7.5	7.5	0.04		
56	6	13581.09	314.24	7.2	7.1	0.04		
57	6	13533.79	314.23	6.9	6.8	0.04		
58	6	13492.55	314.23	6.6	6.6	0.04		
59	6	13455.23	314.22	6.4	6.3	0.04		
60	6	13420.52	314.22	6.2	6.1	0.04		
61	5	13387.66	314.21	6.0	5.9	0.04		
62	5	13356.19	314.21	5.8	5.7	0.04		
63	5	13325.80	314.20	5.6	5.6	0.04		
64	5	13296.32	314.20	5.4	5.4	0.04		
65	5	13267.62	314.20	5.2	5.2	0.04		
66	5	13239.64	314.19	5.1	5.0	0.04		
67	4	13212.31	314.19	4.9	4.9	0.04		
68	4	13185.60	314.18	4.8	4.7	0.04		
69	4	13159.48	314.18	4.6	4.6	0.04		
70	4	13133.92	314.18	4.5	4.4	0.04		
71	4	13108.92	314.17	4.3	4.3	0.04		
72	4	13084.45	314.17	4.2	4.2	0.04		
73	4	13060.50	314.17	4.1	4.0	0.04		
74	4	13037.06	314.16	4.0	3.9	0.04		
75	3	13014.11	314.16	3.8	3.8	0.04		
76	3	12991.65	314.16	3.7	3.7	0.04		
77	3	12969.66	314.15	3.6	3.6	0.04		
78	3	12948.14	314.15	3.5	3.5	0.04		
79	3	12927.06	314.15	3.4	3.3	0.04		
80	3	12906.43	314.14	3.3	3.2	0.04		
81	3	12886.23	314.14	3.2	3.1	0.04		
82	3	12866.45	314.14	3.1	3.0	0.04		
83	3	12847.09	314.13	3.0	3.0	0.04		
84	3	12828.14	314.13	2.9	2.9	0.04		
85	3	12809.58	314.13	2.8	2.8	0.04		
86	2	12791.41	314.13	2.7	2.7	0.04		
87	2	12773.62	314.12	2.6	2.6	0.04		
88	2	12756.20	314.12	2.6	2.5	0.04		
89	2	12739.15	314.12	2.5	2.4	0.04		
90	2	12722.45	314.12	2.4	2.4	0.04		
91	2	12706.10	314.11	2.3	2.3	0.04		
92	2	12690.09	314.11	2.3	2.2	0.04		
93	2	12674.41	314.11	2.2	2.2	0.04		
94	2	12659.06	314.11	2.1	2.1	0.04		
95	2	12644.03	314.10	2.1	2.0	0.04		
96	2	12629.32	314.10	2.0	2.0	0.04		
97	2	12614.91	314.10	1.9	1.9	0.04		
98	2	12600.79	314.10	1.9	1.8	0.04		
99	2	12586.98	314.10	1.8	1.8	0.04		
100	2	12573.44	314.09	1.8	1.7	0.04		
101	1	12560.19	314.09	1.7	1.7	0.04		
102	1	12547.22	314.09	1.7	1.6	0.04		
103	1	12534.51	314.09	1.6	1.6	0.04		
104	1	12522.07	314.09	1.6	1.5	0.04		
105	1	12509.88	314.08	1.5	1.5	0.04		
106	1	12497.94	314.08	1.5	1.4	0.04		
107	1	12486.26	314.08	1.4	1.4	0.04		
108	1	12474.81	314.08	1.4	1.3	0.04		
109	1	12463.60	314.08	1.3	1.3	0.04		
110	1	12452.62	314.08	1.3	1.3	0.04		
111	1	12441.87	314.07	1.3	1.2	0.04		
112	1	12431.33	314.07	1.2	1.2	0.04		
113	1	12421.02	314.07	1.2	1.1	0.04		
114	1	12410.92	314.07	1.1	1.1	0.04		
115	1	12401.02	314.07	1.1	1.1	0.04		
116	1	12391.33	314.07	1.1	1.0	0.04		
117	1	12381.83	314.07	1.0	1.0	0.04		
118	1	12372.53	314.06	1.0	1.0	0.04		
119	1	12363.42	314.06	1.0	0.9	0.04		
120	1	12354.50	314.06	1.0	0.9	0.04		
121	1	12345.76	314.06	0.9	0.9	0.04		
122	1	12337.20	314.06	0.9	0.9	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	12328.81	314.06	0.9	0.8	0.04		
124	1	12320.60	314.06	0.8	0.8	0.04		
125	1	12312.55	314.06	0.8	0.8	0.04		
126	1	12304.66	314.05	0.8	0.8	0.04		
127	1	12296.94	314.05	0.8	0.7	0.04		
128	1	12289.37	314.05	0.8	0.7	0.04		
129	1	12281.96	314.05	0.7	0.7	0.04		
130	1	12274.69	314.05	0.7	0.7	0.04		
131	1	12267.57	314.05	0.7	0.6	0.04		
132	1	12260.60	314.05	0.7	0.6	0.04		
133	1	12253.77	314.05	0.6	0.6	0.04		
134	1	12247.07	314.05	0.6	0.6	0.04		
135	1	12240.51	314.04	0.6	0.6	0.04		
136	0	12234.09	314.04	0.6	0.6	0.04		
137	0	12227.79	314.04	0.6	0.5	0.04		
138	0	12221.61	314.04	0.6	0.5	0.04		
139	0	12215.57	314.04	0.5	0.5	0.04		
140	0	12209.64	314.04	0.5	0.5	0.04		
141	0	12203.83	314.04	0.5	0.5	0.04		
142	0	12198.14	314.04	0.5	0.5	0.04		
143	0	12192.56	314.04	0.5	0.4	0.04		
144	0	12187.09	314.04	0.5	0.4	0.04		
145	0	12181.73	314.04	0.4	0.4	0.04		
146	0	12176.48	314.04	0.4	0.4	0.04		
147	0	12171.33	314.03	0.4	0.4	0.04		
148	0	12166.28	314.03	0.4	0.4	0.04		
149	0	12161.33	314.03	0.4	0.4	0.04		
150	0	12156.48	314.03	0.4	0.4	0.04		
151	0	12151.73	314.03	0.4	0.3	0.04		

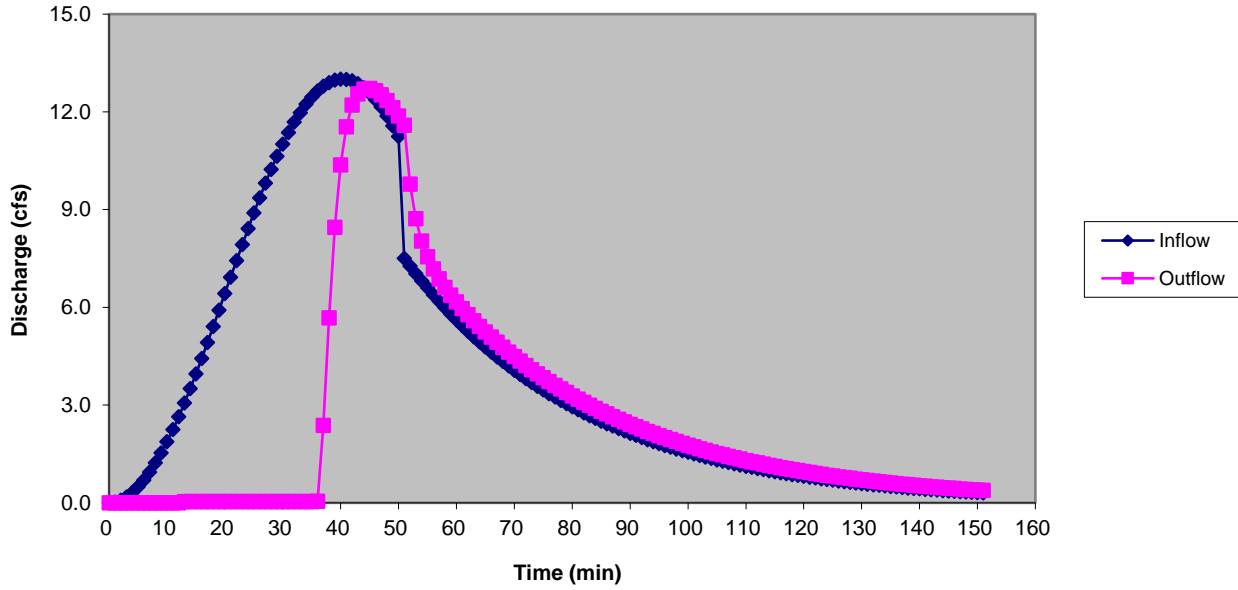




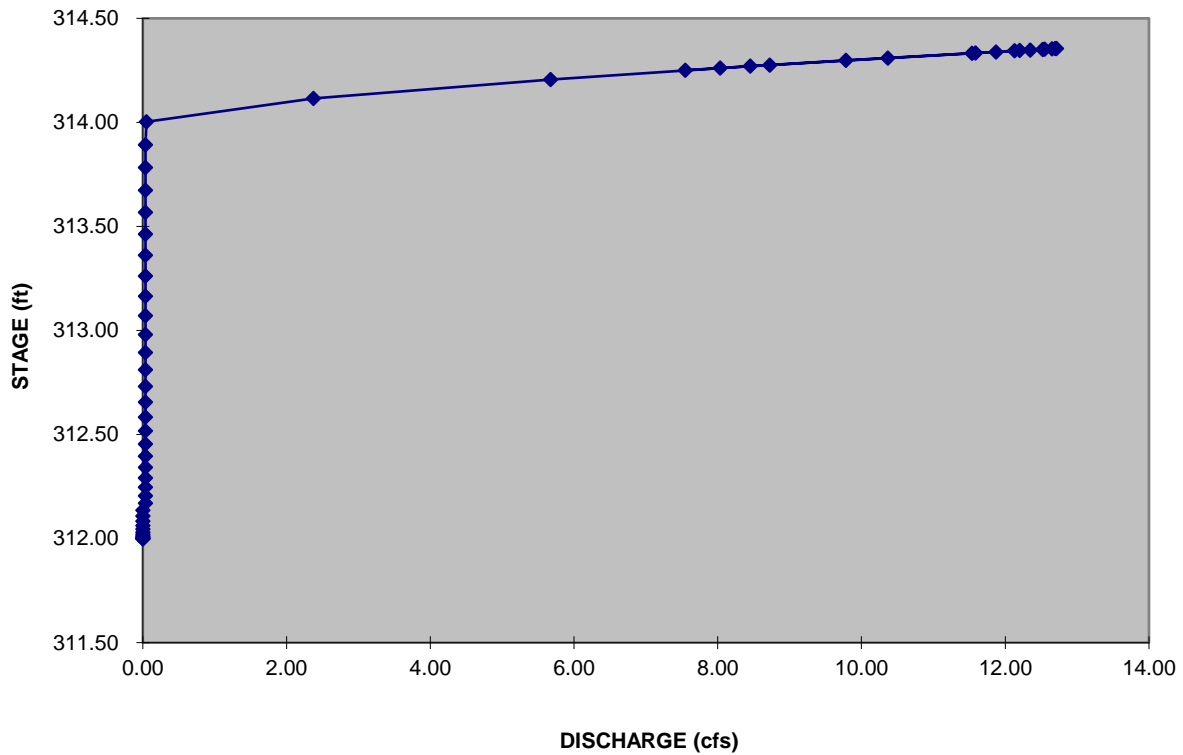
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-4  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID: ST-4

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	3.39 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	0.39 ac.	0.35
Undisturbed grassy area	0.00 ac.	3.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>3.39 ac.</b>	<b>3.39 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.26</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		<i>K Values</i>	
Length of flow path =	612 feet		Overland on grassed surfaces:	2.0
Height of watershed =	12 feet		Overland on paved surfaces:	0.4
Calculated t(c) =	5.0 minutes		Channel in natural channels:	1.0
Minimum t(c) =	5.0 minutes		Channel in mixed urban setting:	1.1
Time of concentration =	5.0 minutes		Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.74 in/hr
10	195	22	7.22 in/hr
25	232	23	8.29 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.74 in/hr	8.8 cfs	5.09 cfs
10-year storm	7.22 in/hr	11.0 cfs	6.40 cfs
25-year storm	8.29 in/hr	12.6 cfs	7.35 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	100
Bottom width =	50
Sediment depth =	2.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	2.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	4
Top of trap length =	124
Top of trap width =	74
Bottom elevation =	312

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	6102 cu. ft.	11890 cu. ft.
Sediment surface area =	3580 sq. ft.	6940 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	2
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.32 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.68 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 6,102 cubic feet

24 HOURS	Best Option =	1 - 3 inch Skimmers with a 2.4 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.9 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.5 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.2 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

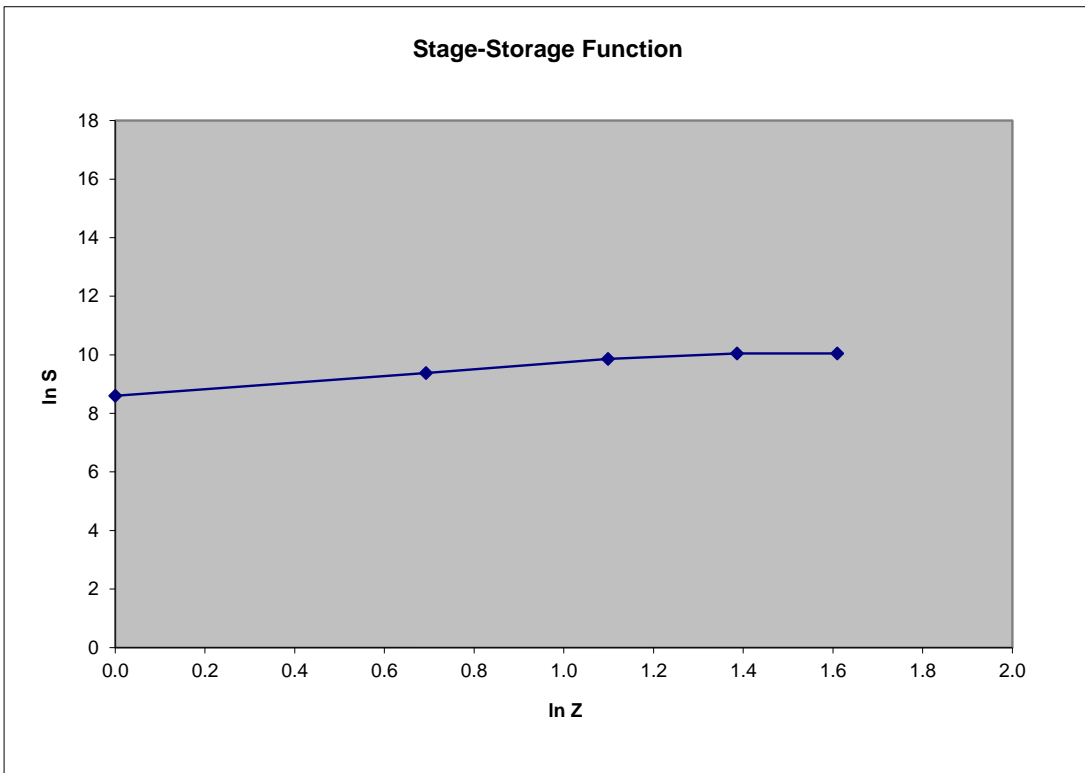
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
312	4979		0	0			
313	5881	5430	5430	1	8.5997	0.0000	0.92
314	6840	6360.5	11790.5	2	9.3750	0.6931	2.09
315	7856	7348	19138.5	3	9.8595	1.0986	3.48
316	0	3928	23066.5	4	10.0461	1.3863	4.23
317	0	0	23066.5	5	10.0461	1.6094	4.23

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 0.95$  and  $K_s = 5854$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	<b>3.39</b> ac	<b>Use Kirpich Equation:</b> Tc = <b>5.0 min</b>
Disturbed Area =	<b>3.39</b> ac	
Undisturbed Woods =	<b>0.00</b> ac	<b>Use Malcom Method:</b> i <sub>1</sub> = <b>5.74 in/hr</b> i <sub>10</sub> = <b>7.22 in/hr</b> i <sub>25</sub> = <b>8.29 in/hr</b>
Undisturbed Grass =	<b>0.00</b> ac	
Hydraulic Length =	<b>612</b> ft	
Vertical Fall =	<b>12</b> ft	<b>Use Rational Method:</b> Q <sub>2</sub> = <b>8.8 cfs</b> Q <sub>10</sub> = <b>11.0 cfs</b> Q <sub>25</sub> = <b>12.6 cfs</b>
For 1 yr Storm, g =	<b>132</b>	
h =	<b>18</b>	
For 10 yr Storm, g =	<b>195</b>	
h =	<b>22</b>	
For 25 yr Storm, g =	<b>232</b>	
h =	<b>23</b>	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u><u>0.45</u></u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	<b>2.7</b> in (2 Year Storm)	S = <b>0.99</b> in
CN =	<b>91</b> (newly graded areas)	Q* = <b>1.75</b> in
		T <sub>p2</sub> = <b>29.4</b> minutes
P <sub>n6</sub> =	<b>3.9</b> in (10 Year Storm)	Q* = <b>2.92</b> in
		T <sub>p10</sub> = <b>39.1</b> minutes
P <sub>n6</sub> =	<b>4.6</b> in (25 Year Storm)	Q* = <b>3.59</b> in
		T <sub>p25</sub> = <b>42.0</b> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	<b>Peak Stage =</b>	<b>314.21</b>	<b>ft</b>
	<b>Rise =</b>	<b>2.21</b>	<b>ft</b>
<b>Freeboard =</b>	<b>1.79</b>		
	<b>Peak Outflow =</b>	<b>5.84</b>	<b>cfs</b>

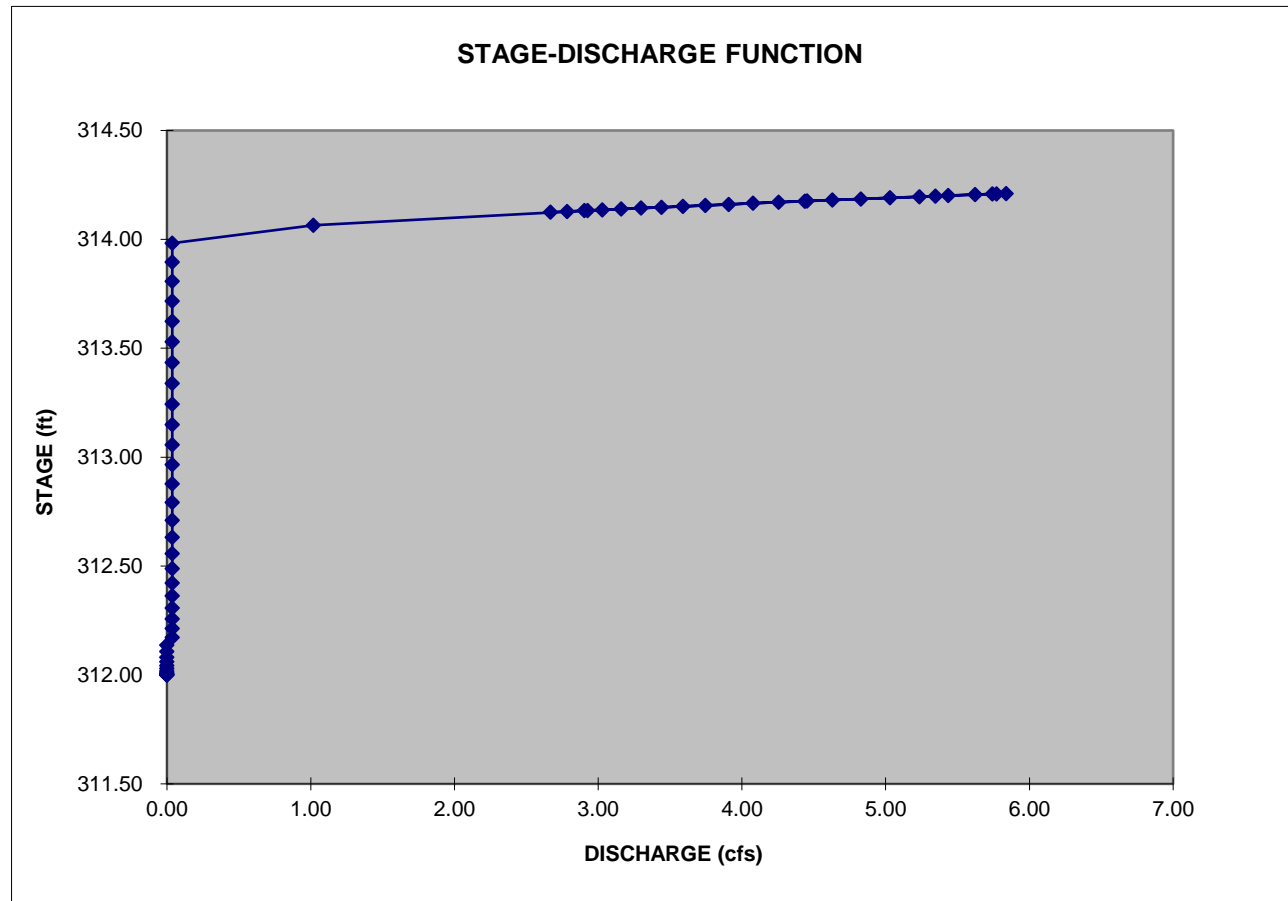
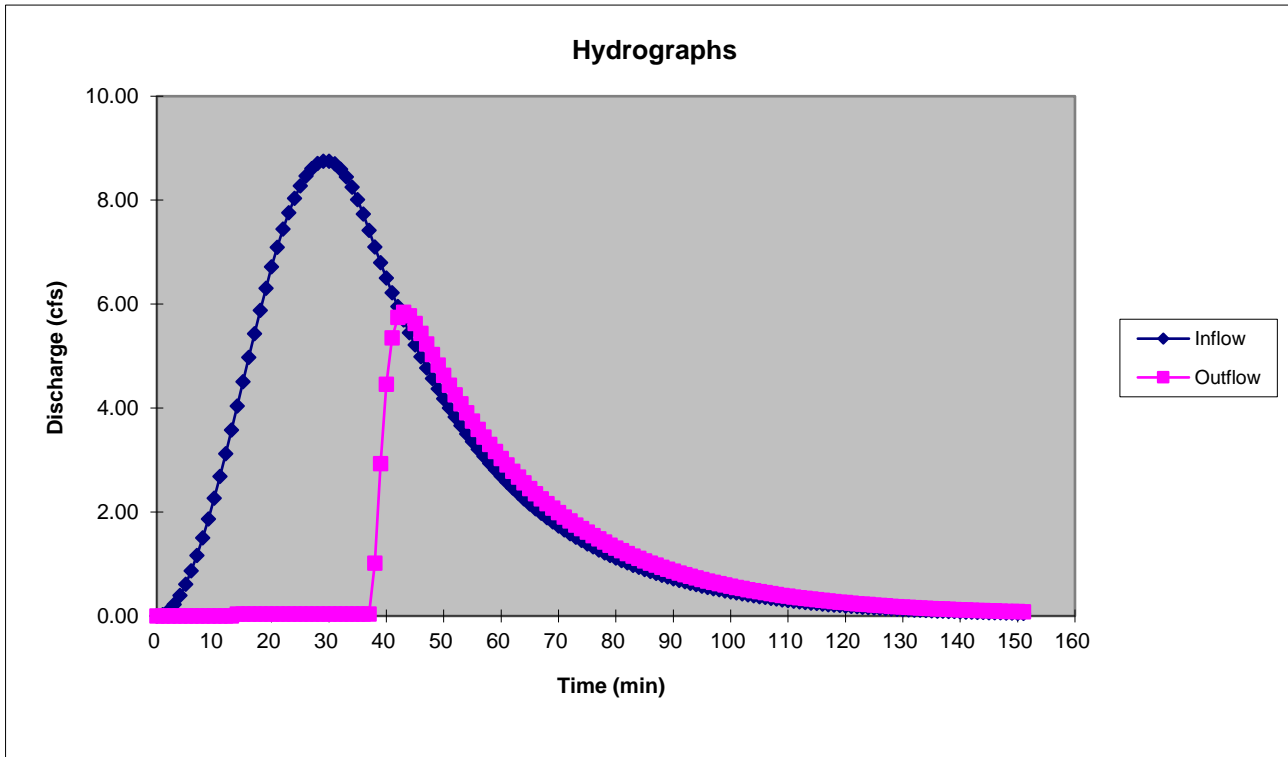
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> <b>316.00</b> ft
Qp = <b>8.76</b> cfs		N = <b>1</b>	
Tp = <b>29.4</b> min		L = <b>20</b> ft	
dT = <b>1.0</b> min		Cw = <b>3.00</b>	
<b>Stage-Storage Results:</b>		Zcr = <b>314.00</b> ft	<b>Skimmer Orifice:</b>
Ks = <b>5854</b>			Number = <b>1.00</b> Ea
b = <b>0.95</b>			Diameter = <b>2.00</b> Inches
Z <sub>0</sub> = <b>312.0</b> ft (inv)			Head = <b>1.52</b> inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = <b>312.00</b> ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.49	312.00	0.0	0.0	0.00		
3	0	7.45	312.00	0.0	0.0	0.00		
4	0	20.78	312.00	0.0	0.0	0.00		
5	1	44.33	312.01	0.0	0.0	0.00		
6	1	80.82	312.01	0.0	0.0	0.00		
7	1	132.81	312.02	0.0	0.0	0.00		
8	1	202.70	312.03	0.0	0.0	0.00		
9	2	292.68	312.04	0.0	0.0	0.00		
10	2	404.71	312.06	0.0	0.0	0.00		
11	3	540.51	312.08	0.0	0.0	0.00		
12	3	701.52	312.11	0.0	0.0	0.00		
13	4	888.90	312.14	0.0	0.0	0.00		
14	4	1103.49	312.17	0.0	0.0	0.04		
15	5	1343.65	312.21	0.0	0.0	0.04		
16	5	1611.80	312.26	0.0	0.0	0.04		
17	5	1907.86	312.31	0.0	0.0	0.04		
18	6	2231.43	312.36	0.0	0.0	0.04		
19	6	2581.79	312.42	0.0	0.0	0.04		
20	7	2957.91	312.49	0.0	0.0	0.04		
21	7	3358.48	312.56	0.0	0.0	0.04		
22	7	3781.90	312.63	0.0	0.0	0.04		
23	8	4226.34	312.71	0.0	0.0	0.04		
24	8	4689.68	312.79	0.0	0.0	0.04		
25	8	5169.63	312.88	0.0	0.0	0.04		
26	8	5663.69	312.97	0.0	0.0	0.04		
27	9	6169.20	313.06	0.0	0.0	0.04		
28	9	6683.38	313.15	0.0	0.0	0.04		
29	9	7203.34	313.24	0.0	0.0	0.04		
30	9	7726.14	313.34	0.0	0.0	0.04		
31	9	8248.78	313.43	0.0	0.0	0.04		
32	9	8768.30	313.53	0.0	0.0	0.04		
33	8	9281.73	313.62	0.0	0.0	0.04		
34	8	9786.22	313.72	0.0	0.0	0.04		
35	8	10278.98	313.81	0.0	0.0	0.04		
36	8	10757.37	313.90	0.0	0.0	0.04		
37	7	11218.91	313.98	0.0	0.0	0.04		
38	7	11661.92	314.06	1.0	1.0	0.04		
39	7	12026.86	314.13	2.9	2.9	0.04		
40	6	12258.80	314.18	4.5	4.4	0.04		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.68	ft	
Peak Stage =	314.32	ft	
Rise =	2.32	ft	
Peak Outflow =	10.71	cfs	

INPUT					
<b>Hydrograph Results:</b>		<b>Weir:</b>		<b>Top of Dam:</b> 316.00 ft	
Qp =	11.02 cfs	N =	1		
Tp =	39.1 min	L =	20 ft	<b>Skimmer Orifice:</b>	
dT =	1.0 min	Cw =	3.00	Number =	1 Ea
<b>Stage-Storage Results:</b>		Zcr =	314.00 ft	Diameter =	2.00 Inches
Ks =	5854			Head =	1.52 inches
b =	0.95				
Z <sub>0</sub> =	312.0 ft (inv)				
<b>Initial Water Level:</b>					
Z <sub>i</sub> =	312.00 ft				

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.06	312.00	0.0	0.0	0.00		
3	0	5.32	312.00	0.0	0.0	0.00		
4	0	14.86	312.00	0.0	0.0	0.00		
5	0	31.76	312.00	0.0	0.0	0.00		
6	1	58.04	312.01	0.0	0.0	0.00		
7	1	95.65	312.01	0.0	0.0	0.00		
8	1	146.49	312.02	0.0	0.0	0.00		
9	1	212.36	312.03	0.0	0.0	0.00		
10	2	294.97	312.04	0.0	0.0	0.00		
11	2	395.90	312.06	0.0	0.0	0.00		
12	2	516.64	312.08	0.0	0.0	0.00		
13	3	658.55	312.10	0.0	0.0	0.00		
14	3	822.83	312.13	0.0	0.0	0.00		
15	4	1010.56	312.16	0.0	0.0	0.00		
16	4	1222.65	312.19	0.0	0.0	0.04		
17	4	1457.67	312.23	0.0	0.0	0.04		
18	5	1718.42	312.28	0.0	0.0	0.04		
19	5	2005.34	312.32	0.0	0.0	0.04		
20	6	2318.70	312.38	0.0	0.0	0.04		
21	6	2658.58	312.44	0.0	0.0	0.04		
22	7	3024.92	312.50	0.0	0.0	0.04		
23	7	3417.47	312.57	0.0	0.0	0.04		
24	7	3835.82	312.64	0.0	0.0	0.04		
25	8	4279.38	312.72	0.0	0.0	0.04		
26	8	4747.42	312.80	0.0	0.0	0.04		
27	9	5239.03	312.89	0.0	0.0	0.04		
28	9	5753.16	312.98	0.0	0.0	0.04		
29	9	6288.62	313.08	0.0	0.0	0.04		
30	10	6844.07	313.18	0.0	0.0	0.04		
31	10	7418.05	313.28	0.0	0.0	0.04		
32	10	8008.97	313.39	0.0	0.0	0.04		
33	10	8615.14	313.50	0.0	0.0	0.04		
34	11	9234.78	313.62	0.0	0.0	0.04		
35	11	9866.00	313.73	0.0	0.0	0.04		
36	11	10506.85	313.85	0.0	0.0	0.04		
37	11	11155.32	313.97	0.0	0.0	0.04		
38	11	11809.36	314.09	1.7	1.7	0.04		
39	11	12366.61	314.20	5.2	5.2	0.04		
40	11	12713.51	314.26	8.0	8.0	0.04		
41	11	12892.52	314.29	9.6	9.6	0.04		
42	11	12973.39	314.31	10.4	10.3	0.04		
43	11	13004.34	314.32	10.6	10.6	0.04		
44	11	13010.71	314.32	10.7	10.7	0.04		
45	10	13004.35	314.32	10.6	10.6	0.04		
46	10	12990.50	314.31	10.5	10.5	0.04		

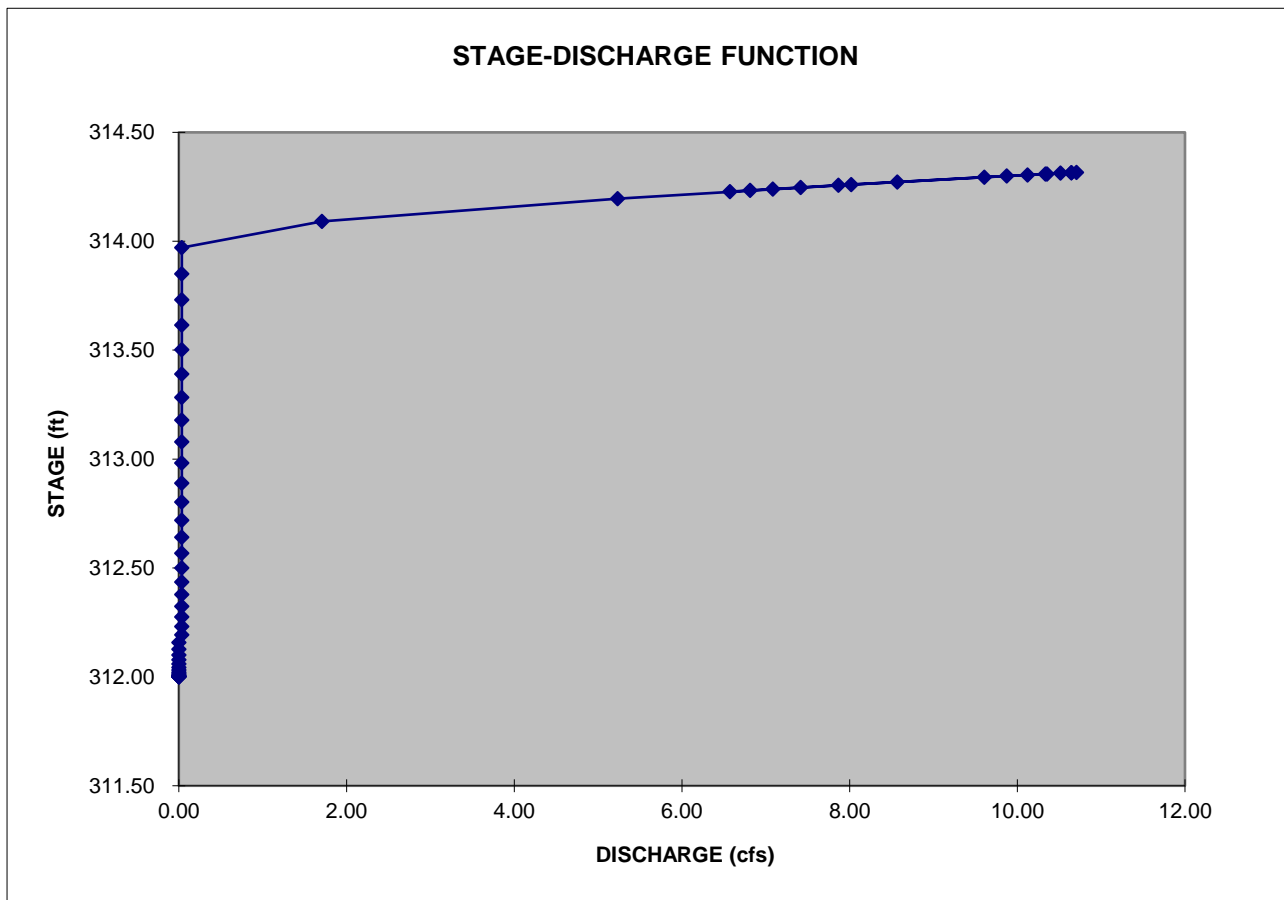
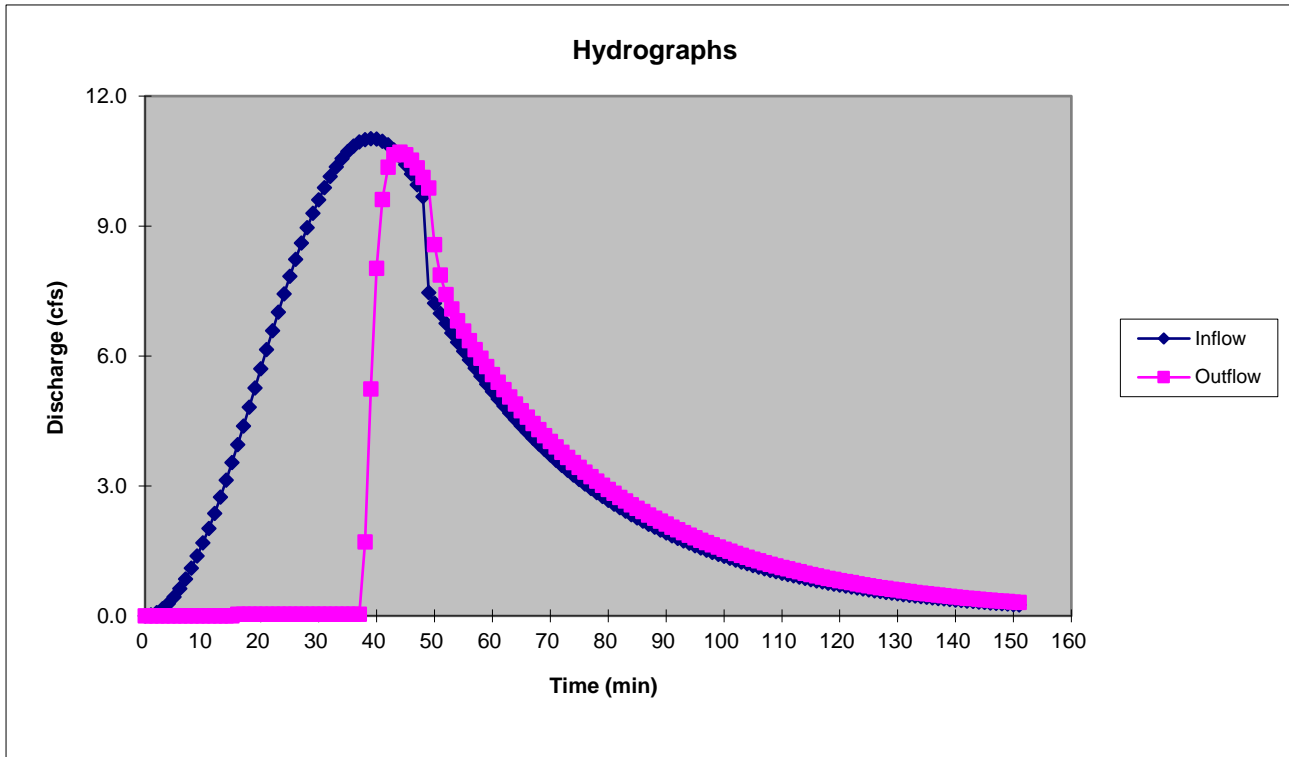


OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	10	12971.51	314.31	10.3	10.3	0.04		
48	10	12948.42	314.30	10.1	10.1	0.04		
49	7	12921.74	314.30	9.9	9.8	0.04		
50	7	12776.78	314.27	8.6	8.5	0.04		
51	7	12695.52	314.26	7.9	7.8	0.04		
52	7	12642.35	314.25	7.4	7.4	0.04		
53	7	12602.46	314.24	7.1	7.0	0.04		
54	6	12569.21	314.23	6.8	6.8	0.04		
55	6	12539.48	314.23	6.6	6.5	0.04		
56	6	12511.78	314.22	6.4	6.3	0.04		
57	6	12485.37	314.22	6.1	6.1	0.04		
58	6	12459.88	314.21	5.9	5.9	0.04		
59	5	12435.12	314.21	5.8	5.7	0.04		
60	5	12410.98	314.20	5.6	5.5	0.04		
61	5	12387.41	314.20	5.4	5.4	0.04		
62	5	12364.38	314.20	5.2	5.2	0.04		
63	5	12341.85	314.19	5.1	5.0	0.04		
64	5	12319.81	314.19	4.9	4.9	0.04		
65	4	12298.25	314.18	4.7	4.7	0.04		
66	4	12277.15	314.18	4.6	4.5	0.04		
67	4	12256.51	314.18	4.4	4.4	0.04		
68	4	12236.31	314.17	4.3	4.3	0.04		
69	4	12216.55	314.17	4.2	4.1	0.04		
70	4	12197.21	314.16	4.0	4.0	0.04		
71	4	12178.28	314.16	3.9	3.9	0.04		
72	3	12159.76	314.16	3.8	3.7	0.04		
73	3	12141.64	314.15	3.7	3.6	0.04		
74	3	12123.91	314.15	3.5	3.5	0.04		
75	3	12106.55	314.15	3.4	3.4	0.04		
76	3	12089.57	314.14	3.3	3.3	0.04		
77	3	12072.95	314.14	3.2	3.2	0.04		
78	3	12056.69	314.14	3.1	3.1	0.04		
79	3	12040.77	314.14	3.0	3.0	0.04		
80	3	12025.19	314.13	2.9	2.9	0.04		
81	3	12009.95	314.13	2.8	2.8	0.04		
82	2	11995.04	314.13	2.7	2.7	0.04		
83	2	11980.44	314.12	2.6	2.6	0.04		
84	2	11966.16	314.12	2.6	2.5	0.04		
85	2	11952.18	314.12	2.5	2.4	0.04		
86	2	11938.49	314.12	2.4	2.4	0.04		
87	2	11925.10	314.11	2.3	2.3	0.04		
88	2	11912.00	314.11	2.3	2.2	0.04		
89	2	11899.18	314.11	2.2	2.1	0.04		
90	2	11886.63	314.11	2.1	2.1	0.04		
91	2	11874.34	314.10	2.0	2.0	0.04		
92	2	11862.32	314.10	2.0	1.9	0.04		
93	2	11850.56	314.10	1.9	1.9	0.04		
94	2	11839.04	314.10	1.9	1.8	0.04		
95	2	11827.77	314.10	1.8	1.8	0.04		
96	2	11816.74	314.09	1.7	1.7	0.04		
97	2	11805.95	314.09	1.7	1.7	0.04		
98	1	11795.38	314.09	1.6	1.6	0.04		
99	1	11785.04	314.09	1.6	1.5	0.04		
100	1	11774.92	314.09	1.5	1.5	0.04		
101	1	11765.01	314.08	1.5	1.5	0.04		
102	1	11755.32	314.08	1.4	1.4	0.04		
103	1	11745.83	314.08	1.4	1.4	0.04		
104	1	11736.54	314.08	1.4	1.3	0.04		
105	1	11727.44	314.08	1.3	1.3	0.04		
106	1	11718.54	314.07	1.3	1.2	0.04		
107	1	11709.83	314.07	1.2	1.2	0.04		
108	1	11701.30	314.07	1.2	1.2	0.04		
109	1	11692.96	314.07	1.2	1.1	0.04		
110	1	11684.78	314.07	1.1	1.1	0.04		
111	1	11676.78	314.07	1.1	1.0	0.04		
112	1	11668.95	314.07	1.0	1.0	0.04		
113	1	11661.29	314.06	1.0	1.0	0.04		
114	1	11653.78	314.06	1.0	0.9	0.04		
115	1	11646.44	314.06	1.0	0.9	0.04		
116	1	11639.25	314.06	0.9	0.9	0.04		
117	1	11632.21	314.06	0.9	0.9	0.04		
118	1	11625.31	314.06	0.9	0.8	0.04		
119	1	11618.57	314.06	0.8	0.8	0.04		
120	1	11611.96	314.06	0.8	0.8	0.04		
121	1	11605.49	314.05	0.8	0.8	0.04		
122	1	11599.16	314.05	0.8	0.7	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	11592.96	314.05	0.7	0.7	0.04		
124	1	11586.89	314.05	0.7	0.7	0.04		
125	1	11580.94	314.05	0.7	0.7	0.04		
126	1	11575.12	314.05	0.7	0.6	0.04		
127	1	11569.42	314.05	0.7	0.6	0.04		
128	1	11563.84	314.05	0.6	0.6	0.04		
129	1	11558.37	314.05	0.6	0.6	0.04		
130	1	11553.02	314.04	0.6	0.6	0.04		
131	0	11547.78	314.04	0.6	0.5	0.04		
132	0	11542.65	314.04	0.6	0.5	0.04		
133	0	11537.62	314.04	0.5	0.5	0.04		
134	0	11532.70	314.04	0.5	0.5	0.04		
135	0	11527.88	314.04	0.5	0.5	0.04		
136	0	11523.16	314.04	0.5	0.5	0.04		
137	0	11518.54	314.04	0.5	0.4	0.04		
138	0	11514.01	314.04	0.5	0.4	0.04		
139	0	11509.57	314.04	0.4	0.4	0.04		
140	0	11505.23	314.04	0.4	0.4	0.04		
141	0	11500.97	314.03	0.4	0.4	0.04		
142	0	11496.80	314.03	0.4	0.4	0.04		
143	0	11492.72	314.03	0.4	0.4	0.04		
144	0	11488.71	314.03	0.4	0.3	0.04		
145	0	11484.79	314.03	0.4	0.3	0.04		
146	0	11480.95	314.03	0.4	0.3	0.04		
147	0	11477.19	314.03	0.3	0.3	0.04		
148	0	11473.50	314.03	0.3	0.3	0.04		
149	0	11469.89	314.03	0.3	0.3	0.04		
150	0	11466.35	314.03	0.3	0.3	0.04		
151	0	11462.88	314.03	0.3	0.3	0.04		

 **McADAMS**  
**GRAPHED DATA FOR 10 YEAR STORM**





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.65	ft	
Peak Stage =	314.35	ft	
Rise =	2.35	ft	
Peak Outflow =	12.52	cfs	

INPUT					
<b>Hydrograph Results:</b>		<b>Weir:</b>		<b>Top of Dam:</b> 316.00 ft	
Qp =	12.64 cfs	N =	1		
Tp =	39.1 min	L =	20 ft		
dT =	1.0 min	Cw =	3.00		
<b>Stage-Storage Results:</b>		Zcr =	314.00 ft	<b>Skimmer Orifice:</b>	
Ks =	5854			Number =	1.00 Ea
b =	0.95			Diameter =	2.00 Inches
Z <sub>0</sub> =	312.0 ft (inv)			Head =	1.52 inches
<b>Initial Water Level:</b>					
Z <sub>i</sub> =	312.00 ft				

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.0		
2	0	1.22	312.00	0.0	0.0	0.0		
3	0	6.10	312.00	0.0	0.0	0.0		
4	0	17.05	312.00	0.0	0.0	0.0		
5	1	36.43	312.00	0.0	0.0	0.0		
6	1	66.58	312.01	0.0	0.0	0.0		
7	1	109.74	312.02	0.0	0.0	0.0		
8	1	168.07	312.02	0.0	0.0	0.0		
9	2	243.64	312.04	0.0	0.0	0.0		
10	2	338.40	312.05	0.0	0.0	0.0		
11	2	454.20	312.07	0.0	0.0	0.0		
12	3	592.72	312.09	0.0	0.0	0.0		
13	3	755.52	312.12	0.0	0.0	0.0		
14	4	943.99	312.15	0.0	0.0	0.0		
15	4	1159.36	312.18	0.0	0.0	0.0		0.04
16	5	1400.48	312.22	0.0	0.0	0.0		0.04
17	5	1670.44	312.27	0.0	0.0	0.0		0.04
18	6	1969.91	312.32	0.0	0.0	0.0		0.04
19	6	2299.40	312.37	0.0	0.0	0.0		0.04
20	7	2659.22	312.44	0.0	0.0	0.0		0.04
21	7	3049.48	312.50	0.0	0.0	0.0		0.04
22	8	3470.09	312.58	0.0	0.0	0.0		0.04
23	8	3920.77	312.66	0.0	0.0	0.0		0.04
24	9	4401.04	312.74	0.0	0.0	0.0		0.04
25	9	4910.25	312.83	0.0	0.0	0.0		0.04
26	9	5447.53	312.93	0.0	0.0	0.0		0.04
27	10	6011.86	313.03	0.0	0.0	0.0		0.04
28	10	6602.02	313.13	0.0	0.0	0.0		0.04
29	11	7216.66	313.25	0.0	0.0	0.0		0.04
30	11	7854.22	313.36	0.0	0.0	0.0		0.04
31	11	8513.04	313.48	0.0	0.0	0.0		0.04
32	12	9191.30	313.61	0.0	0.0	0.0		0.04
33	12	9887.06	313.74	0.0	0.0	0.0		0.04
34	12	10598.26	313.87	0.0	0.0	0.0		0.04
35	12	11322.76	314.00	0.0	0.0	0.0		0.04
36	12	12058.13	314.14	3.1	3.1	0.0		0.04
37	13	12617.39	314.24	7.2	7.2	0.0		0.04
38	13	12937.71	314.30	10.0	10.0	0.0		0.04
39	13	13093.14	314.33	11.5	11.5	0.0		0.04
40	13	13161.62	314.34	12.2	12.1	0.0		0.04
41	13	13188.91	314.35	12.4	12.4	0.0		0.04
42	12	13196.59	314.35	12.5	12.5	0.0		0.04
43	12	13193.90	314.35	12.5	12.5	0.0		0.04
44	12	13184.69	314.35	12.4	12.4	0.0		0.04
45	12	13170.53	314.35	12.3	12.2	0.0		0.04
46	12	13152.08	314.34	12.1	12.0	0.0		0.04

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	11	13129.69	314.34	11.9	11.8	0.04		
48	11	13103.54	314.33	11.6	11.6	0.04		
49	7	13073.77	314.33	11.3	11.3	0.04		
50	7	12842.71	314.28	9.2	9.1	0.04		
51	7	12726.30	314.26	8.1	8.1	0.04		
52	7	12657.28	314.25	7.5	7.5	0.04		
53	7	12609.87	314.24	7.1	7.1	0.04		
54	6	12572.95	314.23	6.8	6.8	0.04		
55	6	12541.39	314.23	6.6	6.6	0.04		
56	6	12512.77	314.22	6.4	6.3	0.04		
57	6	12485.89	314.22	6.1	6.1	0.04		
58	6	12460.15	314.21	5.9	5.9	0.04		
59	5	12435.27	314.21	5.8	5.7	0.04		
60	5	12411.06	314.20	5.6	5.5	0.04		
61	5	12387.46	314.20	5.4	5.4	0.04		
62	5	12364.40	314.20	5.2	5.2	0.04		
63	5	12341.86	314.19	5.1	5.0	0.04		
64	5	12319.82	314.19	4.9	4.9	0.04		
65	4	12298.25	314.18	4.7	4.7	0.04		
66	4	12277.16	314.18	4.6	4.5	0.04		
67	4	12256.51	314.18	4.4	4.4	0.04		
68	4	12236.31	314.17	4.3	4.3	0.04		
69	4	12216.55	314.17	4.2	4.1	0.04		
70	4	12197.21	314.16	4.0	4.0	0.04		
71	4	12178.28	314.16	3.9	3.9	0.04		
72	3	12159.76	314.16	3.8	3.7	0.04		
73	3	12141.64	314.15	3.7	3.6	0.04		
74	3	12123.91	314.15	3.5	3.5	0.04		
75	3	12106.55	314.15	3.4	3.4	0.04		
76	3	12089.57	314.14	3.3	3.3	0.04		
77	3	12072.95	314.14	3.2	3.2	0.04		
78	3	12056.69	314.14	3.1	3.1	0.04		
79	3	12040.77	314.14	3.0	3.0	0.04		
80	3	12025.19	314.13	2.9	2.9	0.04		
81	3	12009.95	314.13	2.8	2.8	0.04		
82	2	11995.04	314.13	2.7	2.7	0.04		
83	2	11980.44	314.12	2.6	2.6	0.04		
84	2	11966.16	314.12	2.6	2.5	0.04		
85	2	11952.18	314.12	2.5	2.4	0.04		
86	2	11938.49	314.12	2.4	2.4	0.04		
87	2	11925.10	314.11	2.3	2.3	0.04		
88	2	11912.00	314.11	2.3	2.2	0.04		
89	2	11899.18	314.11	2.2	2.1	0.04		
90	2	11886.63	314.11	2.1	2.1	0.04		
91	2	11874.34	314.10	2.0	2.0	0.04		
92	2	11862.32	314.10	2.0	1.9	0.04		
93	2	11850.56	314.10	1.9	1.9	0.04		
94	2	11839.04	314.10	1.9	1.8	0.04		
95	2	11827.77	314.10	1.8	1.8	0.04		
96	2	11816.74	314.09	1.7	1.7	0.04		
97	2	11805.95	314.09	1.7	1.7	0.04		
98	1	11795.38	314.09	1.6	1.6	0.04		
99	1	11785.04	314.09	1.6	1.5	0.04		
100	1	11774.92	314.09	1.5	1.5	0.04		
101	1	11765.01	314.08	1.5	1.5	0.04		
102	1	11755.32	314.08	1.4	1.4	0.04		
103	1	11745.83	314.08	1.4	1.4	0.04		
104	1	11736.54	314.08	1.4	1.3	0.04		
105	1	11727.44	314.08	1.3	1.3	0.04		
106	1	11718.54	314.07	1.3	1.2	0.04		
107	1	11709.83	314.07	1.2	1.2	0.04		
108	1	11701.30	314.07	1.2	1.2	0.04		
109	1	11692.96	314.07	1.2	1.1	0.04		
110	1	11684.78	314.07	1.1	1.1	0.04		
111	1	11676.78	314.07	1.1	1.0	0.04		
112	1	11668.95	314.07	1.0	1.0	0.04		
113	1	11661.29	314.06	1.0	1.0	0.04		
114	1	11653.78	314.06	1.0	0.9	0.04		
115	1	11646.44	314.06	1.0	0.9	0.04		
116	1	11639.25	314.06	0.9	0.9	0.04		
117	1	11632.21	314.06	0.9	0.9	0.04		
118	1	11625.31	314.06	0.9	0.8	0.04		
119	1	11618.57	314.06	0.8	0.8	0.04		
120	1	11611.96	314.06	0.8	0.8	0.04		
121	1	11605.49	314.05	0.8	0.8	0.04		
122	1	11599.16	314.05	0.8	0.7	0.04		

**OUTPUT**

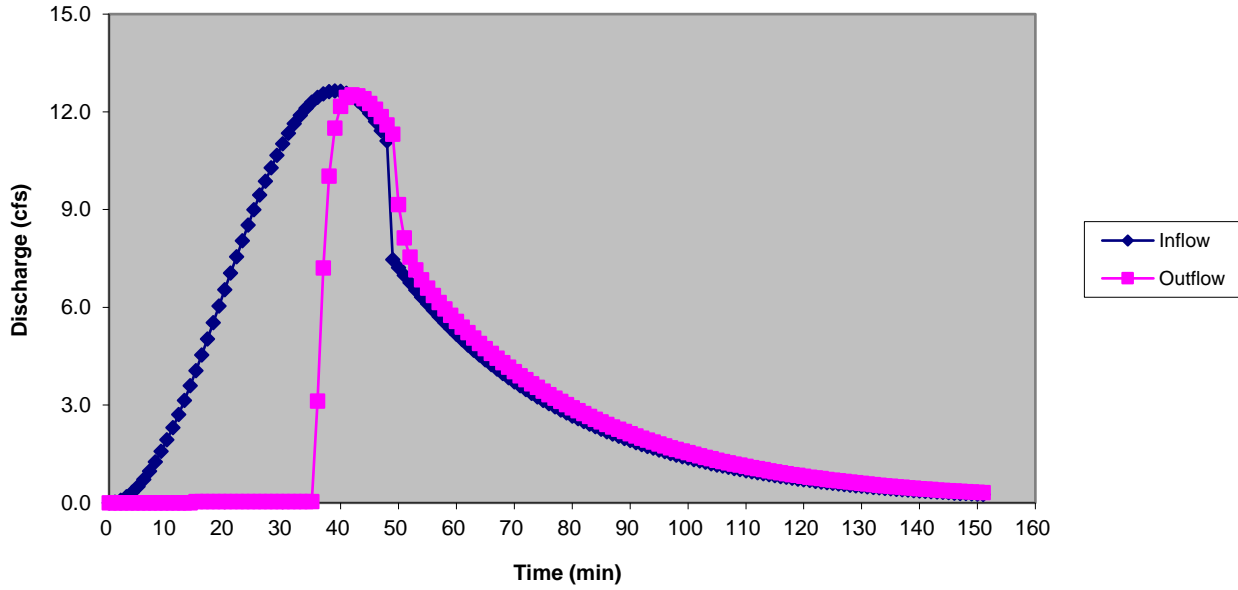
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	11592.96	314.05	0.7	0.7	0.04		
124	1	11586.89	314.05	0.7	0.7	0.04		
125	1	11580.94	314.05	0.7	0.7	0.04		
126	1	11575.12	314.05	0.7	0.6	0.04		
127	1	11569.42	314.05	0.7	0.6	0.04		
128	1	11563.84	314.05	0.6	0.6	0.04		
129	1	11558.37	314.05	0.6	0.6	0.04		
130	1	11553.02	314.04	0.6	0.6	0.04		
131	0	11547.78	314.04	0.6	0.5	0.04		
132	0	11542.65	314.04	0.6	0.5	0.04		
133	0	11537.62	314.04	0.5	0.5	0.04		
134	0	11532.70	314.04	0.5	0.5	0.04		
135	0	11527.88	314.04	0.5	0.5	0.04		
136	0	11523.16	314.04	0.5	0.5	0.04		
137	0	11518.54	314.04	0.5	0.4	0.04		
138	0	11514.01	314.04	0.5	0.4	0.04		
139	0	11509.57	314.04	0.4	0.4	0.04		
140	0	11505.23	314.04	0.4	0.4	0.04		
141	0	11500.97	314.03	0.4	0.4	0.04		
142	0	11496.80	314.03	0.4	0.4	0.04		
143	0	11492.72	314.03	0.4	0.4	0.04		
144	0	11488.71	314.03	0.4	0.3	0.04		
145	0	11484.79	314.03	0.4	0.3	0.04		
146	0	11480.95	314.03	0.4	0.3	0.04		
147	0	11477.19	314.03	0.3	0.3	0.04		
148	0	11473.50	314.03	0.3	0.3	0.04		
149	0	11469.89	314.03	0.3	0.3	0.04		
150	0	11466.35	314.03	0.3	0.3	0.04		
151	0	11462.88	314.03	0.3	0.3	0.04		



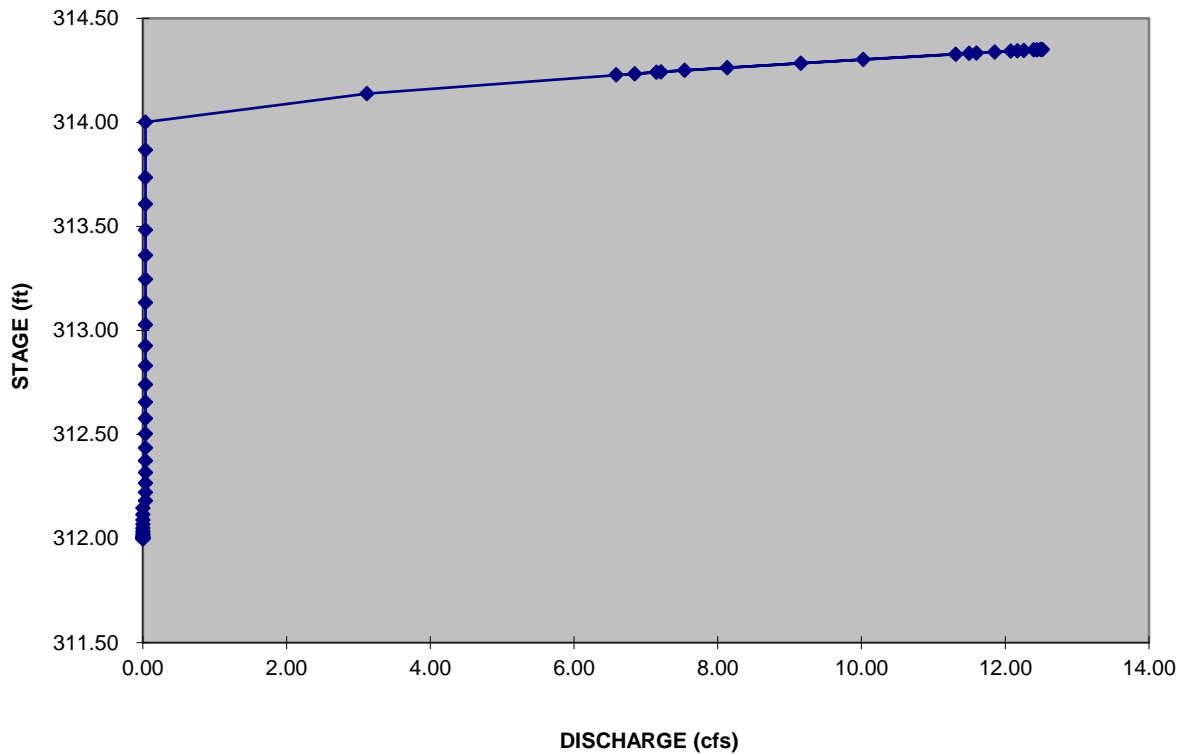
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-5  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm



# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID: ST-5

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	2.54 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	2.54 ac.	0.35
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>2.54 ac.</b>	<b>2.54 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		<i>K Values</i>	
Length of flow path =	481 feet		Overland on grassed surfaces:	2.0
Height of watershed =	4 feet		Overland on paved surfaces:	0.4
Calculated t(c) =	5.7 minutes		Channel in natural channels:	1.0
Minimum t(c) =	5.0 minutes		Channel in mixed urban setting:	1.1
Time of concentration =	5.7 minutes		Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.56 in/hr
10	195	22	7.03 in/hr
25	232	23	8.07 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.56 in/hr	6.4 cfs	4.94 cfs
10-year storm	7.03 in/hr	8.0 cfs	6.25 cfs
25-year storm	8.07 in/hr	9.2 cfs	7.18 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	100
Bottom width =	50
Sediment depth =	1.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	1.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	3
Top of trap length =	118
Top of trap width =	68
Bottom elevation =	318

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	4572 cu. ft.	5460 cu. ft.
Sediment surface area =	2610 sq. ft.	5940 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	1
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.26 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.74 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 4,572 cubic feet

24 HOURS	Best Option =	1 - 2.5 inch Skimmers with a 2.3 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.6 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.3 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
318	4979		0	0			
319	5881	5430	5430	1	8.5997	0.0000	0.92
320	6840	6360.5	11790.5	2	9.3750	0.6931	2.09
321	7856	7348	19138.5	3	9.8595	1.0986	3.48
322	0	3928	23066.5	4	10.0461	1.3863	4.23
323	0	0	23066.5	5	10.0461	1.6094	4.23

**STAGE-STORAGE FUNCTION:**

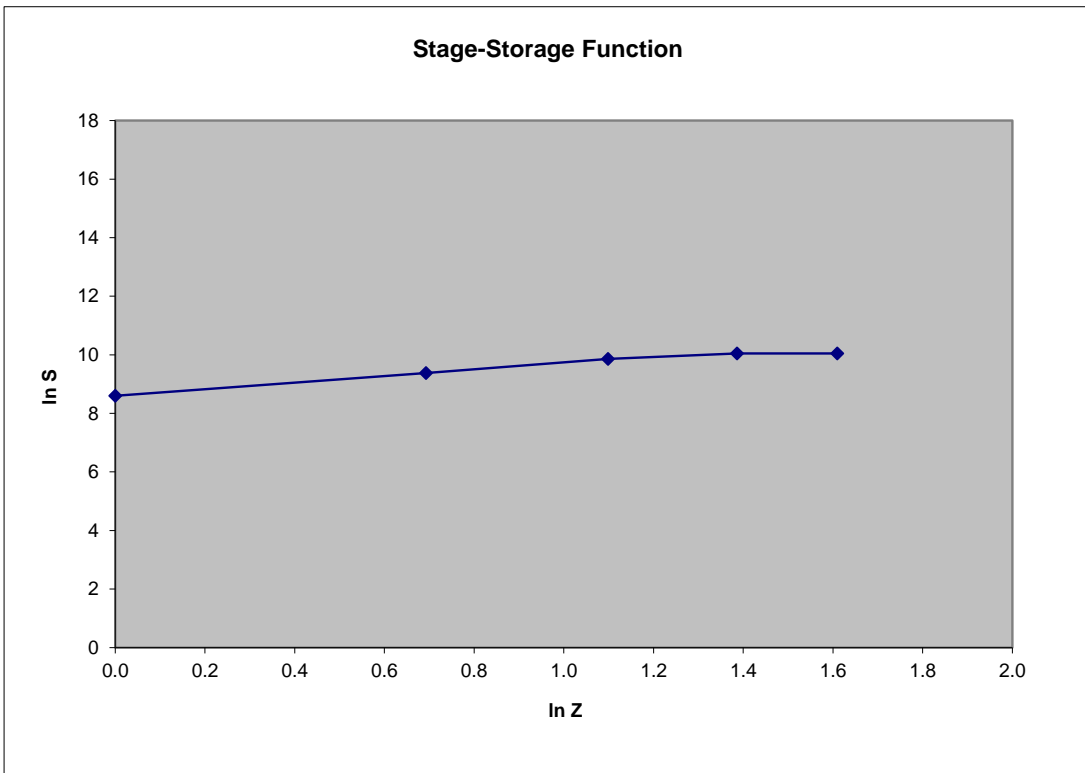
$$S = K_s * Z^b$$

where:

**b = 0.95**

and

**K<sub>s</sub> = 5854**





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT	OUTPUT
Drainage Area, DA = <span style="border: 1px solid gray; padding: 2px;">2.54</span> ac Disturbed Area = <span style="border: 1px solid gray; padding: 2px;">2.54</span> ac Undisturbed Woods = <span style="border: 1px solid gray; padding: 2px;">0.00</span> ac Undisturbed Grass = <span style="border: 1px solid gray; padding: 2px;">0.00</span> ac Hydraulic Length = <span style="border: 1px solid gray; padding: 2px;">481</span> ft Vertical Fall = <span style="border: 1px solid gray; padding: 2px;">4</span> ft For 1 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">132</span> h = <span style="border: 1px solid gray; padding: 2px;">18</span> For 10 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">195</span> h = <span style="border: 1px solid gray; padding: 2px;">22</span> For 25 yr Storm, g = <span style="border: 1px solid gray; padding: 2px;">232</span> h = <span style="border: 1px solid gray; padding: 2px;">23</span>	<b>Use Kirpich Equation:</b> Tc = <span style="border: 1px solid gray; padding: 2px;">5.7 min</span>  <b>Use Malcom Method:</b> i <sub>1</sub> = <span style="border: 1px solid gray; padding: 2px;">5.56 in/hr</span> i <sub>10</sub> = <span style="border: 1px solid gray; padding: 2px;">7.03 in/hr</span> i <sub>25</sub> = <span style="border: 1px solid gray; padding: 2px;">8.07 in/hr</span>  <b>Use Rational Method:</b> Q <sub>2</sub> = <span style="border: 1px solid gray; padding: 2px;">6.4 cfs</span> Q <sub>10</sub> = <span style="border: 1px solid gray; padding: 2px;">8.0 cfs</span> Q <sub>25</sub> = <span style="border: 1px solid gray; padding: 2px;">9.2 cfs</span>
"C" CALCULATION - See Sediment Trap Sizing  <b>"C" = <span style="border-bottom: 3px double orange; padding: 0 20px;">0.45</span></b>	

**SOLUTION - Tp:**

INPUT	OUTPUT
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">2.7</span> in (2 Year Storm) CN = <span style="border: 1px solid gray; padding: 2px;">91</span> (newly graded areas)	S = <span style="border: 1px solid gray; padding: 2px;">0.99</span> in Q* = <span style="border: 1px solid gray; padding: 2px;">1.75</span> in  T <sub>p2</sub> = <span style="border: 1px solid gray; padding: 2px;">30.4</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">3.9</span> in (10 Year Storm)	Q* = <span style="border: 1px solid gray; padding: 2px;">2.92</span> in  T <sub>p10</sub> = <span style="border: 1px solid gray; padding: 2px;">40.2</span> minutes
P <sub>n6</sub> = <span style="border: 1px solid gray; padding: 2px;">4.6</span> in (25 Year Storm)	Q* = <span style="border: 1px solid gray; padding: 2px;">3.59</span> in  T <sub>p25</sub> = <span style="border: 1px solid gray; padding: 2px;">43.1</span> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	<b>Peak Stage =</b>	<b>319.20</b>	<b>ft</b>
	<b>Rise =</b>	<b>1.20</b>	<b>ft</b>
<b>Freeboard =</b>	<b>1.80</b>		
	<b>Peak Outflow =</b>	<b>5.58</b>	<b>cfs</b>

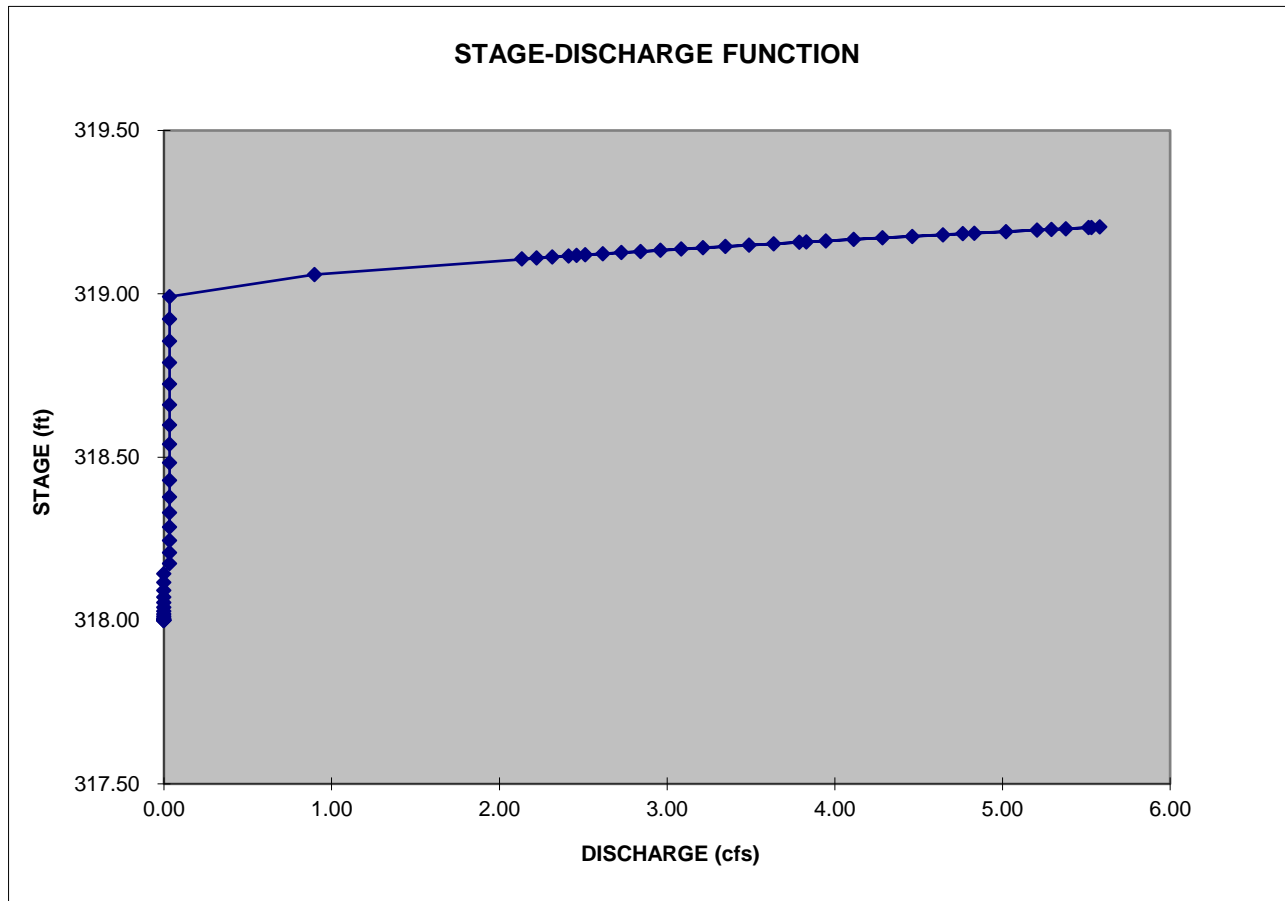
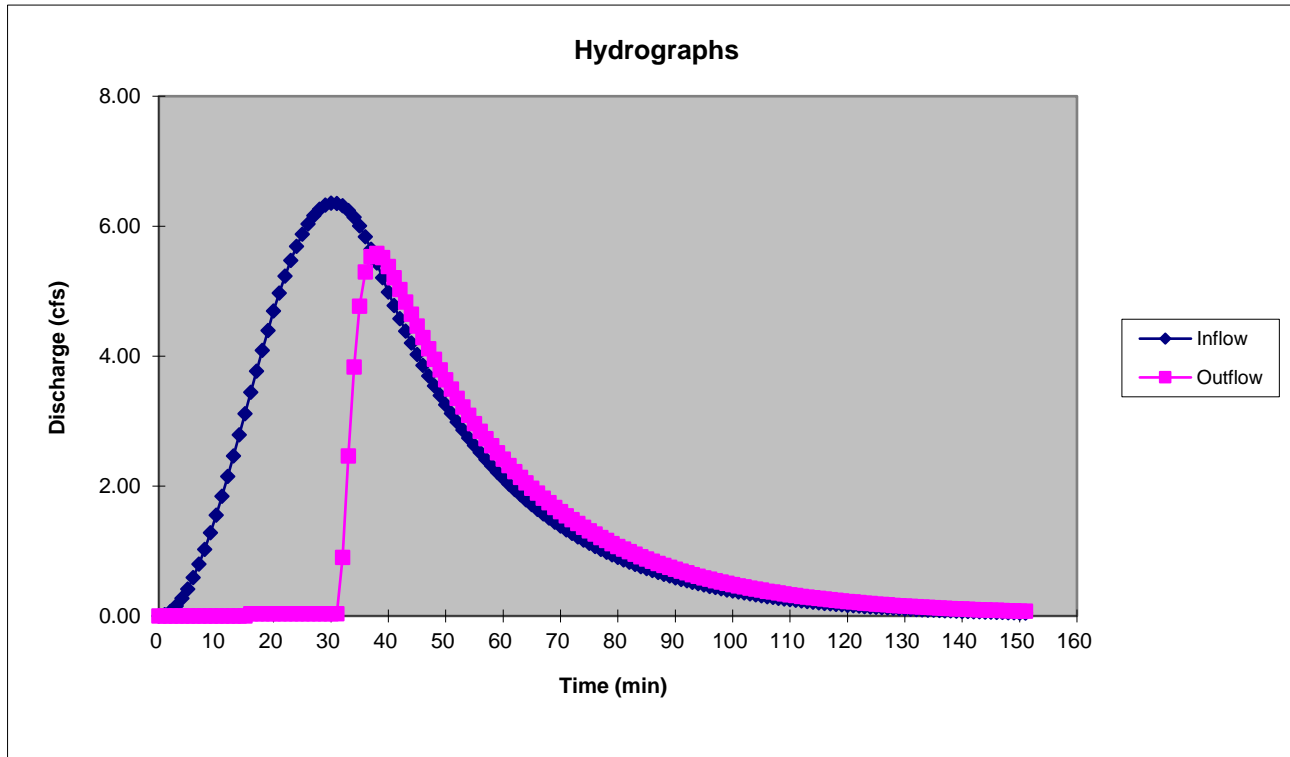
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> <b>321.00</b> ft
Qp = <b>6.36</b> cfs		N = <b>1</b>	
Tp = <b>30.4</b> min		L = <b>20</b> ft	
dT = <b>1.0</b> min		Cw = <b>3.00</b>	<b>Skimmer Orifice:</b>
<b>Stage-Storage Results:</b>		Zcr = <b>319.00</b> ft	Number = <b>1.00</b> Ea
Ks = <b>5854</b>			Diameter = <b>2.00</b> Inches
b = <b>0.95</b>			Head = <b>1.31</b> inches
Z <sub>0</sub> = <b>318.0</b> ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = <b>318.00</b> ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	318.00	0.0	0.0	0.0		
1	0	0.00	318.00	0.0	0.0	0.00		
2	0	1.02	318.00	0.0	0.0	0.00		
3	0	5.08	318.00	0.0	0.0	0.00		
4	0	14.17	318.00	0.0	0.0	0.00		
5	0	30.23	318.00	0.0	0.0	0.00		
6	1	55.13	318.01	0.0	0.0	0.00		
7	1	90.63	318.01	0.0	0.0	0.00		
8	1	138.39	318.02	0.0	0.0	0.00		
9	1	199.93	318.03	0.0	0.0	0.00		
10	2	276.64	318.04	0.0	0.0	0.00		
11	2	369.72	318.05	0.0	0.0	0.00		
12	2	480.23	318.07	0.0	0.0	0.00		
13	2	609.01	318.09	0.0	0.0	0.00		
14	3	756.72	318.12	0.0	0.0	0.00		
15	3	923.83	318.14	0.0	0.0	0.00		
16	3	1110.59	318.17	0.0	0.0	0.03		
17	4	1314.98	318.21	0.0	0.0	0.03		
18	4	1538.89	318.25	0.0	0.0	0.03		
19	4	1781.95	318.29	0.0	0.0	0.03		
20	5	2043.56	318.33	0.0	0.0	0.03		
21	5	2322.96	318.38	0.0	0.0	0.03		
22	5	2619.17	318.43	0.0	0.0	0.03		
23	5	2931.05	318.48	0.0	0.0	0.03		
24	6	3257.27	318.54	0.0	0.0	0.03		
25	6	3596.38	318.60	0.0	0.0	0.03		
26	6	3946.76	318.66	0.0	0.0	0.03		
27	6	4306.69	318.72	0.0	0.0	0.03		
28	6	4674.34	318.79	0.0	0.0	0.03		
29	6	5047.80	318.86	0.0	0.0	0.03		
30	6	5425.09	318.92	0.0	0.0	0.03		
31	6	5804.21	318.99	0.0	0.0	0.03		
32	6	6183.12	319.06	0.9	0.9	0.03		
33	6	6507.93	319.12	2.5	2.4	0.03		
34	6	6734.80	319.16	3.8	3.8	0.03		
35	6	6873.16	319.18	4.8	4.7	0.03		
36	6	6947.43	319.20	5.3	5.3	0.03		
37	6	6980.06	319.20	5.5	5.5	0.03		
38	5	6986.78	319.20	5.6	5.5	0.03		
39	5	6977.73	319.20	5.5	5.5	0.03		
40	5	6959.03	319.20	5.4	5.3	0.03		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.74	ft	
Peak Stage =	319.26	ft	
Rise =	1.26	ft	
Peak Outflow =	7.98	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	321.00 ft
Qp = 8.03 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 40.2 min	L = 20 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 319.00 ft	Head = 1.31 inches	
Ks = 5854			
b = 0.95			
Z <sub>0</sub> = 318.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 318.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	318.00	0.0	0.0	0.0		
1	0	0.00	318.00	0.0	0.0	0.00		
2	0	0.74	318.00	0.0	0.0	0.00		
3	0	3.67	318.00	0.0	0.0	0.00		
4	0	10.27	318.00	0.0	0.0	0.00		
5	0	21.95	318.00	0.0	0.0	0.00		
6	0	40.12	318.01	0.0	0.0	0.00		
7	1	66.14	318.01	0.0	0.0	0.00		
8	1	101.31	318.01	0.0	0.0	0.00		
9	1	146.91	318.02	0.0	0.0	0.00		
10	1	204.11	318.03	0.0	0.0	0.00		
11	1	274.05	318.04	0.0	0.0	0.00		
12	2	357.76	318.05	0.0	0.0	0.00		
13	2	456.21	318.07	0.0	0.0	0.00		
14	2	570.27	318.09	0.0	0.0	0.00		
15	2	700.71	318.11	0.0	0.0	0.00		
16	3	848.21	318.13	0.0	0.0	0.00		
17	3	1013.34	318.16	0.0	0.0	0.00		
18	3	1196.56	318.19	0.0	0.0	0.03		
19	4	1396.17	318.22	0.0	0.0	0.03		
20	4	1614.47	318.26	0.0	0.0	0.03		
21	4	1851.58	318.30	0.0	0.0	0.03		
22	5	2107.52	318.34	0.0	0.0	0.03		
23	5	2382.18	318.39	0.0	0.0	0.03		
24	5	2675.33	318.44	0.0	0.0	0.03		
25	6	2986.67	318.49	0.0	0.0	0.03		
26	6	3315.73	318.55	0.0	0.0	0.03		
27	6	3661.97	318.61	0.0	0.0	0.03		
28	6	4024.74	318.67	0.0	0.0	0.03		
29	7	4403.27	318.74	0.0	0.0	0.03		
30	7	4796.73	318.81	0.0	0.0	0.03		
31	7	5204.17	318.88	0.0	0.0	0.03		
32	7	5624.55	318.96	0.0	0.0	0.03		
33	7	6056.77	319.04	0.5	0.4	0.03		
34	8	6474.60	319.11	2.3	2.2	0.03		
35	8	6792.37	319.17	4.2	4.2	0.03		
36	8	7002.14	319.21	5.7	5.7	0.03		
37	8	7129.67	319.23	6.7	6.6	0.03		
38	8	7204.13	319.24	7.3	7.2	0.03		
39	8	7246.99	319.25	7.6	7.6	0.03		
40	8	7271.42	319.26	7.8	7.8	0.03		
41	8	7284.81	319.26	7.9	7.9	0.03		
42	8	7291.09	319.26	8.0	7.9	0.03		
43	8	7292.32	319.26	8.0	8.0	0.03		
44	8	7289.57	319.26	8.0	7.9	0.03		
45	8	7283.39	319.26	7.9	7.9	0.03		
46	8	7274.09	319.26	7.8	7.8	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	7	7261.84	319.25	7.7	7.7	0.03		
48	7	7246.76	319.25	7.6	7.6	0.03		
49	7	7228.95	319.25	7.5	7.4	0.03		
50	7	7208.50	319.24	7.3	7.3	0.03		
51	5	7185.50	319.24	7.1	7.1	0.03		
52	5	7076.88	319.22	6.3	6.2	0.03		
53	5	7009.27	319.21	5.7	5.7	0.03		
54	5	6962.54	319.20	5.4	5.4	0.03		
55	5	6927.00	319.19	5.1	5.1	0.03		
56	5	6897.71	319.19	4.9	4.9	0.03		
57	4	6872.05	319.18	4.8	4.7	0.03		
58	4	6848.61	319.18	4.6	4.6	0.03		
59	4	6826.60	319.18	4.4	4.4	0.03		
60	4	6805.59	319.17	4.3	4.3	0.03		
61	4	6785.32	319.17	4.2	4.1	0.03		
62	4	6765.65	319.16	4.0	4.0	0.03		
63	4	6746.49	319.16	3.9	3.9	0.03		
64	3	6727.80	319.16	3.8	3.8	0.03		
65	3	6709.53	319.15	3.7	3.6	0.03		
66	3	6691.66	319.15	3.6	3.5	0.03		
67	3	6674.17	319.15	3.4	3.4	0.03		
68	3	6657.05	319.14	3.3	3.3	0.03		
69	3	6640.30	319.14	3.2	3.2	0.03		
70	3	6623.90	319.14	3.1	3.1	0.03		
71	3	6607.84	319.14	3.0	3.0	0.03		
72	3	6592.11	319.13	2.9	2.9	0.03		
73	3	6576.71	319.13	2.9	2.8	0.03		
74	3	6561.64	319.13	2.8	2.7	0.03		
75	2	6546.87	319.12	2.7	2.6	0.03		
76	2	6532.42	319.12	2.6	2.6	0.03		
77	2	6518.26	319.12	2.5	2.5	0.03		
78	2	6504.39	319.12	2.4	2.4	0.03		
79	2	6490.82	319.11	2.4	2.3	0.03		
80	2	6477.52	319.11	2.3	2.3	0.03		
81	2	6464.51	319.11	2.2	2.2	0.03		
82	2	6451.76	319.11	2.2	2.1	0.03		
83	2	6439.27	319.11	2.1	2.1	0.03		
84	2	6427.05	319.10	2.0	2.0	0.03		
85	2	6415.07	319.10	2.0	1.9	0.03		
86	2	6403.35	319.10	1.9	1.9	0.03		
87	2	6391.87	319.10	1.8	1.8	0.03		
88	2	6380.62	319.09	1.8	1.8	0.03		
89	2	6369.61	319.09	1.7	1.7	0.03		
90	2	6358.82	319.09	1.7	1.6	0.03		
91	1	6348.26	319.09	1.6	1.6	0.03		
92	1	6337.92	319.09	1.6	1.5	0.03		
93	1	6327.79	319.09	1.5	1.5	0.03		
94	1	6317.87	319.08	1.5	1.4	0.03		
95	1	6308.15	319.08	1.4	1.4	0.03		
96	1	6298.64	319.08	1.4	1.4	0.03		
97	1	6289.32	319.08	1.3	1.3	0.03		
98	1	6280.19	319.08	1.3	1.3	0.03		
99	1	6271.25	319.08	1.3	1.2	0.03		
100	1	6262.49	319.07	1.2	1.2	0.03		
101	1	6253.92	319.07	1.2	1.2	0.03		
102	1	6245.52	319.07	1.2	1.1	0.03		
103	1	6237.29	319.07	1.1	1.1	0.03		
104	1	6229.23	319.07	1.1	1.1	0.03		
105	1	6221.34	319.07	1.1	1.0	0.03		
106	1	6213.61	319.06	1.0	1.0	0.03		
107	1	6206.03	319.06	1.0	1.0	0.03		
108	1	6198.62	319.06	1.0	0.9	0.03		
109	1	6191.35	319.06	0.9	0.9	0.03		
110	1	6184.23	319.06	0.9	0.9	0.03		
111	1	6177.26	319.06	0.9	0.8	0.03		
112	1	6170.43	319.06	0.8	0.8	0.03		
113	1	6163.74	319.06	0.8	0.8	0.03		
114	1	6157.19	319.05	0.8	0.8	0.03		
115	1	6150.77	319.05	0.8	0.7	0.03		
116	1	6144.48	319.05	0.8	0.7	0.03		
117	1	6138.32	319.05	0.7	0.7	0.03		
118	1	6132.29	319.05	0.7	0.7	0.03		
119	1	6126.37	319.05	0.7	0.7	0.03		
120	1	6120.58	319.05	0.7	0.6	0.03		
121	1	6114.90	319.05	0.6	0.6	0.03		
122	1	6109.34	319.05	0.6	0.6	0.03		

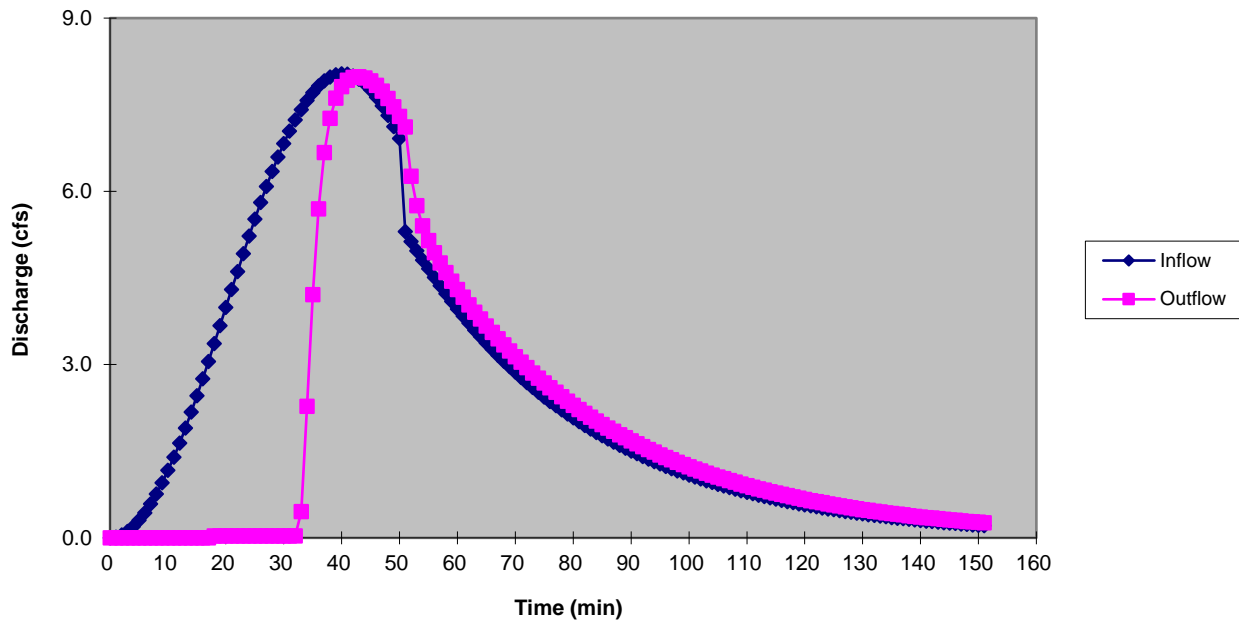
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	6103.89	319.04	0.6	0.6	0.03		
124	1	6098.56	319.04	0.6	0.6	0.03		
125	0	6093.33	319.04	0.6	0.5	0.03		
126	0	6088.20	319.04	0.6	0.5	0.03		
127	0	6083.18	319.04	0.5	0.5	0.03		
128	0	6078.26	319.04	0.5	0.5	0.03		
129	0	6073.44	319.04	0.5	0.5	0.03		
130	0	6068.71	319.04	0.5	0.5	0.03		
131	0	6064.08	319.04	0.5	0.4	0.03		
132	0	6059.55	319.04	0.5	0.4	0.03		
133	0	6055.10	319.04	0.4	0.4	0.03		
134	0	6050.74	319.04	0.4	0.4	0.03		
135	0	6046.47	319.03	0.4	0.4	0.03		
136	0	6042.28	319.03	0.4	0.4	0.03		
137	0	6038.18	319.03	0.4	0.4	0.03		
138	0	6034.16	319.03	0.4	0.3	0.03		
139	0	6030.22	319.03	0.4	0.3	0.03		
140	0	6026.36	319.03	0.4	0.3	0.03		
141	0	6022.57	319.03	0.4	0.3	0.03		
142	0	6018.86	319.03	0.3	0.3	0.03		
143	0	6015.22	319.03	0.3	0.3	0.03		
144	0	6011.65	319.03	0.3	0.3	0.03		
145	0	6008.15	319.03	0.3	0.3	0.03		
146	0	6004.72	319.03	0.3	0.3	0.03		
147	0	6001.36	319.03	0.3	0.3	0.03		
148	0	5998.07	319.03	0.3	0.2	0.03		
149	0	5994.83	319.03	0.3	0.2	0.03		
150	0	5991.66	319.02	0.3	0.2	0.03		
151	0	5988.56	319.02	0.3	0.2	0.03		



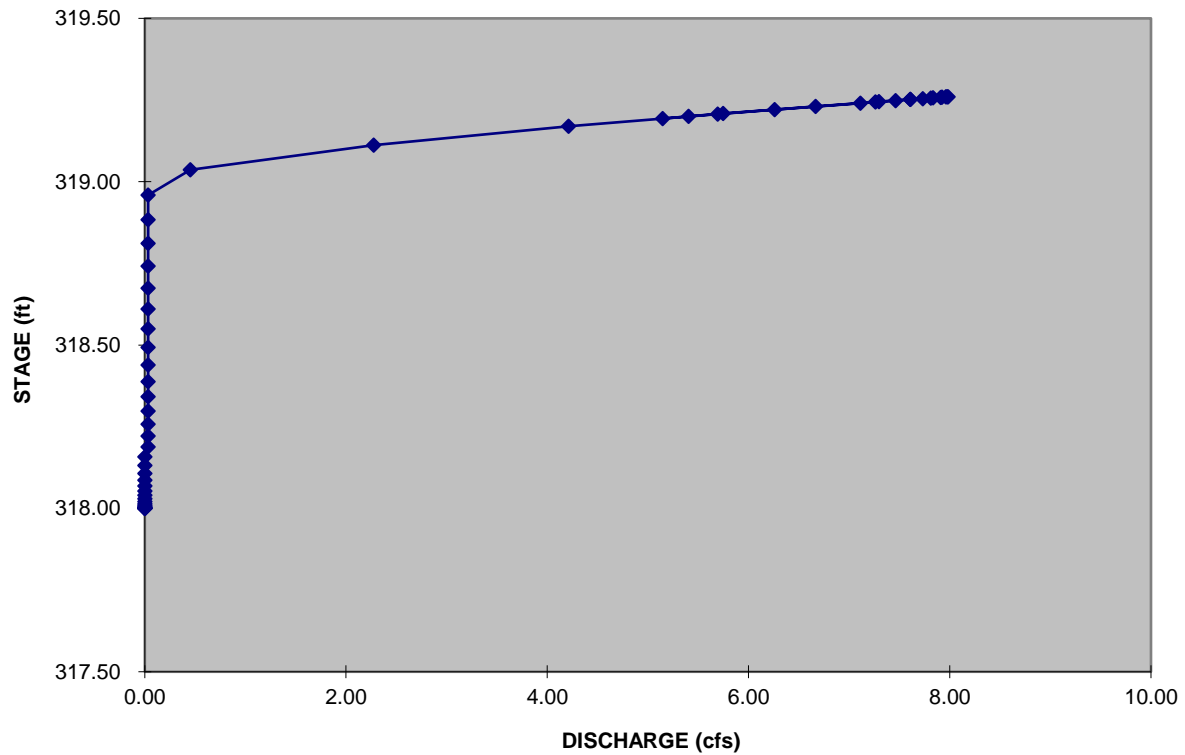
# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.71	ft	
Peak Stage =	319.29	ft	
Rise =	1.29	ft	
Peak Outflow =	9.19	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	321.00 ft
Qp = 9.23 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 40.2 min	L = 20 ft	Number = 1.00 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 319.00 ft	Head = 1.31 inches	
Ks = 5854			
b = 0.95			
Z <sub>0</sub> = 318.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 318.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	318.00	0.0	0.0	0.0		
1	0	0.00	318.00	0.0	0.0	0.00		
2	0	0.84	318.00	0.0	0.0	0.00		
3	0	4.22	318.00	0.0	0.0	0.00		
4	0	11.79	318.00	0.0	0.0	0.00		
5	0	25.21	318.00	0.0	0.0	0.00		
6	0	46.07	318.01	0.0	0.0	0.00		
7	1	75.95	318.01	0.0	0.0	0.00		
8	1	116.34	318.02	0.0	0.0	0.00		
9	1	168.70	318.02	0.0	0.0	0.00		
10	1	234.39	318.03	0.0	0.0	0.00		
11	2	314.70	318.05	0.0	0.0	0.00		
12	2	410.83	318.06	0.0	0.0	0.00		
13	2	523.89	318.08	0.0	0.0	0.00		
14	2	654.87	318.10	0.0	0.0	0.00		
15	3	804.66	318.12	0.0	0.0	0.00		
16	3	974.04	318.15	0.0	0.0	0.00		
17	4	1163.66	318.18	0.0	0.0	0.03		
18	4	1372.01	318.22	0.0	0.0	0.03		
19	4	1601.54	318.26	0.0	0.0	0.03		
20	5	1852.53	318.30	0.0	0.0	0.03		
21	5	2125.12	318.34	0.0	0.0	0.03		
22	5	2419.33	318.39	0.0	0.0	0.03		
23	6	2735.03	318.45	0.0	0.0	0.03		
24	6	3071.98	318.51	0.0	0.0	0.03		
25	6	3429.80	318.57	0.0	0.0	0.03		
26	7	3807.98	318.64	0.0	0.0	0.03		
27	7	4205.89	318.71	0.0	0.0	0.03		
28	7	4622.78	318.78	0.0	0.0	0.03		
29	8	5057.78	318.86	0.0	0.0	0.03		
30	8	5509.91	318.94	0.0	0.0	0.03		
31	8	5978.09	319.02	0.2	0.2	0.03		
32	8	6449.15	319.11	2.1	2.1	0.03		
33	9	6819.47	319.17	4.4	4.4	0.03		
34	9	7066.74	319.22	6.2	6.1	0.03		
35	9	7217.59	319.25	7.4	7.3	0.03		
36	9	7306.44	319.26	8.1	8.1	0.03		
37	9	7359.10	319.27	8.5	8.5	0.03		
38	9	7391.20	319.28	8.8	8.8	0.03		
39	9	7411.31	319.28	9.0	9.0	0.03		
40	9	7423.83	319.28	9.1	9.1	0.03		
41	9	7431.00	319.29	9.2	9.1	0.03		
42	9	7433.91	319.29	9.2	9.2	0.03		
43	9	7433.10	319.29	9.2	9.2	0.03		
44	9	7428.85	319.28	9.2	9.1	0.03		
45	9	7421.31	319.28	9.1	9.1	0.03		
46	9	7410.59	319.28	9.0	9.0	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	9	7396.76	319.28	8.9	8.8	0.03		
48	8	7379.89	319.28	8.7	8.7	0.03		
49	8	7360.07	319.27	8.6	8.5	0.03		
50	8	7337.37	319.27	8.4	8.3	0.03		
51	5	7311.88	319.26	8.1	8.1	0.03		
52	5	7141.04	319.23	6.8	6.7	0.03		
53	5	7043.50	319.21	6.0	6.0	0.03		
54	5	6981.35	319.20	5.5	5.5	0.03		
55	5	6937.54	319.20	5.2	5.2	0.03		
56	5	6903.70	319.19	5.0	4.9	0.03		
57	4	6875.50	319.18	4.8	4.7	0.03		
58	4	6850.61	319.18	4.6	4.6	0.03		
59	4	6827.77	319.18	4.5	4.4	0.03		
60	4	6806.28	319.17	4.3	4.3	0.03		
61	4	6785.73	319.17	4.2	4.1	0.03		
62	4	6765.90	319.16	4.0	4.0	0.03		
63	4	6746.64	319.16	3.9	3.9	0.03		
64	3	6727.89	319.16	3.8	3.8	0.03		
65	3	6709.58	319.15	3.7	3.6	0.03		
66	3	6691.69	319.15	3.6	3.5	0.03		
67	3	6674.19	319.15	3.4	3.4	0.03		
68	3	6657.07	319.14	3.3	3.3	0.03		
69	3	6640.31	319.14	3.2	3.2	0.03		
70	3	6623.90	319.14	3.1	3.1	0.03		
71	3	6607.84	319.14	3.0	3.0	0.03		
72	3	6592.11	319.13	2.9	2.9	0.03		
73	3	6576.72	319.13	2.9	2.8	0.03		
74	3	6561.64	319.13	2.8	2.7	0.03		
75	2	6546.87	319.12	2.7	2.6	0.03		
76	2	6532.42	319.12	2.6	2.6	0.03		
77	2	6518.26	319.12	2.5	2.5	0.03		
78	2	6504.39	319.12	2.4	2.4	0.03		
79	2	6490.82	319.11	2.4	2.3	0.03		
80	2	6477.52	319.11	2.3	2.3	0.03		
81	2	6464.51	319.11	2.2	2.2	0.03		
82	2	6451.76	319.11	2.2	2.1	0.03		
83	2	6439.27	319.11	2.1	2.1	0.03		
84	2	6427.05	319.10	2.0	2.0	0.03		
85	2	6415.07	319.10	2.0	1.9	0.03		
86	2	6403.35	319.10	1.9	1.9	0.03		
87	2	6391.87	319.10	1.8	1.8	0.03		
88	2	6380.62	319.09	1.8	1.8	0.03		
89	2	6369.61	319.09	1.7	1.7	0.03		
90	2	6358.82	319.09	1.7	1.6	0.03		
91	1	6348.26	319.09	1.6	1.6	0.03		
92	1	6337.92	319.09	1.6	1.5	0.03		
93	1	6327.79	319.09	1.5	1.5	0.03		
94	1	6317.87	319.08	1.5	1.4	0.03		
95	1	6308.15	319.08	1.4	1.4	0.03		
96	1	6298.64	319.08	1.4	1.4	0.03		
97	1	6289.32	319.08	1.3	1.3	0.03		
98	1	6280.19	319.08	1.3	1.3	0.03		
99	1	6271.25	319.08	1.3	1.2	0.03		
100	1	6262.49	319.07	1.2	1.2	0.03		
101	1	6253.92	319.07	1.2	1.2	0.03		
102	1	6245.52	319.07	1.2	1.1	0.03		
103	1	6237.29	319.07	1.1	1.1	0.03		
104	1	6229.23	319.07	1.1	1.1	0.03		
105	1	6221.34	319.07	1.1	1.0	0.03		
106	1	6213.61	319.06	1.0	1.0	0.03		
107	1	6206.03	319.06	1.0	1.0	0.03		
108	1	6198.62	319.06	1.0	0.9	0.03		
109	1	6191.35	319.06	0.9	0.9	0.03		
110	1	6184.23	319.06	0.9	0.9	0.03		
111	1	6177.26	319.06	0.9	0.8	0.03		
112	1	6170.43	319.06	0.8	0.8	0.03		
113	1	6163.74	319.06	0.8	0.8	0.03		
114	1	6157.19	319.05	0.8	0.8	0.03		
115	1	6150.77	319.05	0.8	0.7	0.03		
116	1	6144.48	319.05	0.8	0.7	0.03		
117	1	6138.32	319.05	0.7	0.7	0.03		
118	1	6132.29	319.05	0.7	0.7	0.03		
119	1	6126.37	319.05	0.7	0.7	0.03		
120	1	6120.58	319.05	0.7	0.6	0.03		
121	1	6114.90	319.05	0.6	0.6	0.03		
122	1	6109.34	319.05	0.6	0.6	0.03		

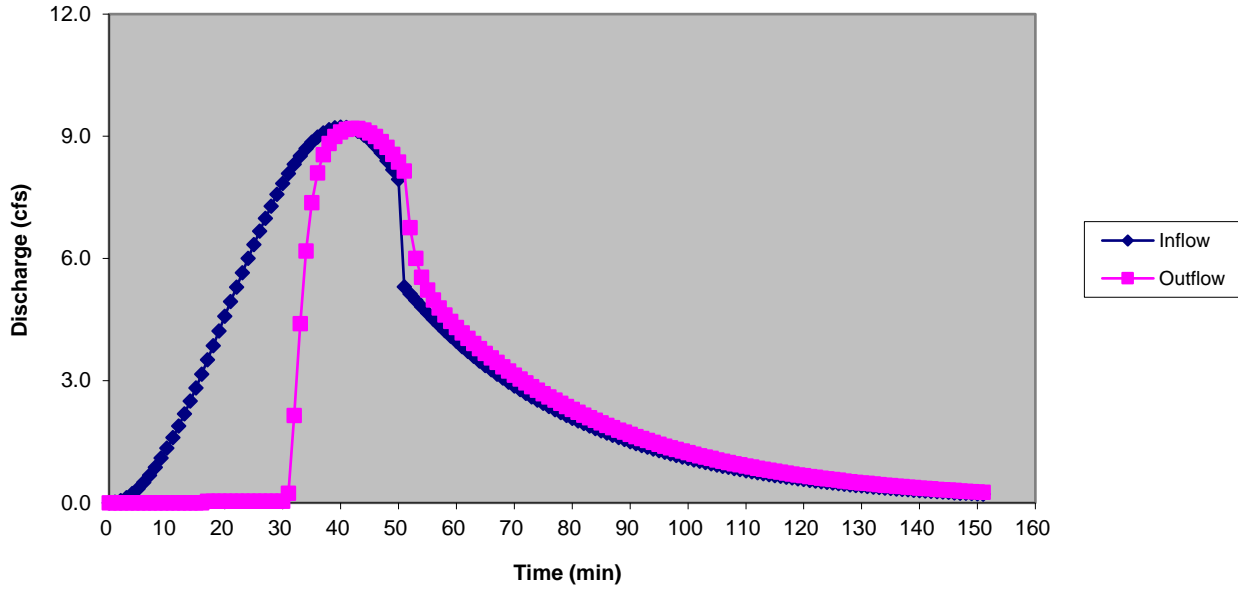
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	6103.89	319.04	0.6	0.6	0.03		
124	1	6098.56	319.04	0.6	0.6	0.03		
125	0	6093.33	319.04	0.6	0.5	0.03		
126	0	6088.20	319.04	0.6	0.5	0.03		
127	0	6083.18	319.04	0.5	0.5	0.03		
128	0	6078.26	319.04	0.5	0.5	0.03		
129	0	6073.44	319.04	0.5	0.5	0.03		
130	0	6068.71	319.04	0.5	0.5	0.03		
131	0	6064.08	319.04	0.5	0.4	0.03		
132	0	6059.55	319.04	0.5	0.4	0.03		
133	0	6055.10	319.04	0.4	0.4	0.03		
134	0	6050.74	319.04	0.4	0.4	0.03		
135	0	6046.47	319.03	0.4	0.4	0.03		
136	0	6042.28	319.03	0.4	0.4	0.03		
137	0	6038.18	319.03	0.4	0.4	0.03		
138	0	6034.16	319.03	0.4	0.3	0.03		
139	0	6030.22	319.03	0.4	0.3	0.03		
140	0	6026.36	319.03	0.4	0.3	0.03		
141	0	6022.57	319.03	0.4	0.3	0.03		
142	0	6018.86	319.03	0.3	0.3	0.03		
143	0	6015.22	319.03	0.3	0.3	0.03		
144	0	6011.65	319.03	0.3	0.3	0.03		
145	0	6008.15	319.03	0.3	0.3	0.03		
146	0	6004.72	319.03	0.3	0.3	0.03		
147	0	6001.36	319.03	0.3	0.3	0.03		
148	0	5998.07	319.03	0.3	0.2	0.03		
149	0	5994.83	319.03	0.3	0.2	0.03		
150	0	5991.66	319.02	0.3	0.2	0.03		
151	0	5988.56	319.02	0.3	0.2	0.03		



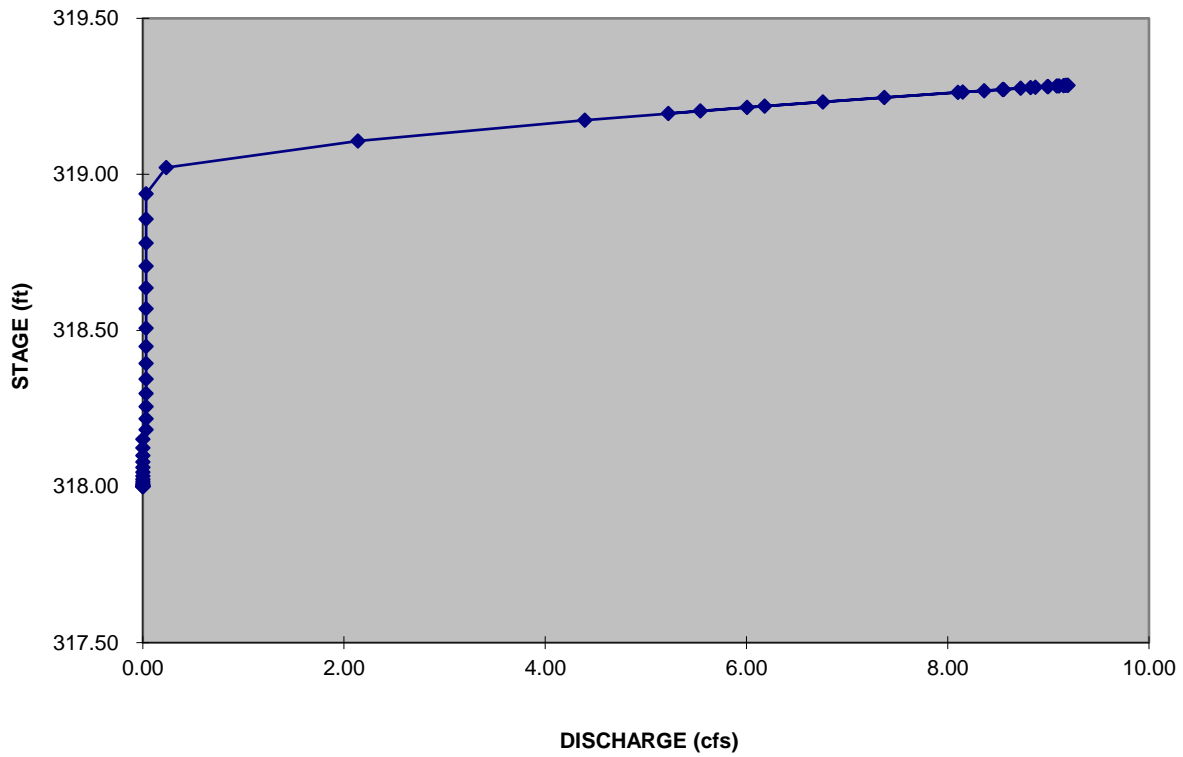
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-6  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID ST-6

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	2.30 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	2.30 ac.	0.35
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>2.30 ac.</b>	<b>2.30 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	551	feet	
Height of watershed =	8	feet	
Calculated t(c) =	5.1	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	5.1	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.70 in/hr
10	195	22	7.18 in/hr
25	232	23	8.24 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.70 in/hr	5.9 cfs	4.59 cfs
10-year storm	7.18 in/hr	7.4 cfs	5.78 cfs
25-year storm	8.24 in/hr	8.5 cfs	6.64 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	100
Bottom width =	50
Sediment depth =	1.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	1.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	3
Top of trap length =	118
Top of trap width =	68
Bottom elevation =	313

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	4140 cu. ft.	5460 cu. ft.
Sediment surface area =	2420 sq. ft.	5940 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	1
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.25 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.75 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 4,140 cubic feet

24 HOURS	Best Option =	1 - 2.5 inch Skimmers with a 2.1 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.5 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.3 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
313	4979		0	0			
314	5881	5430	5430	1	8.5997	0.0000	0.92
315	6840	6360.5	11790.5	2	9.3750	0.6931	2.09
316	7856	7348	19138.5	3	9.8595	1.0986	3.48
317	0	3928	23066.5	4	10.0461	1.3863	4.23
318	0	0	23066.5	5	10.0461	1.6094	4.23

**STAGE-STORAGE FUNCTION:**

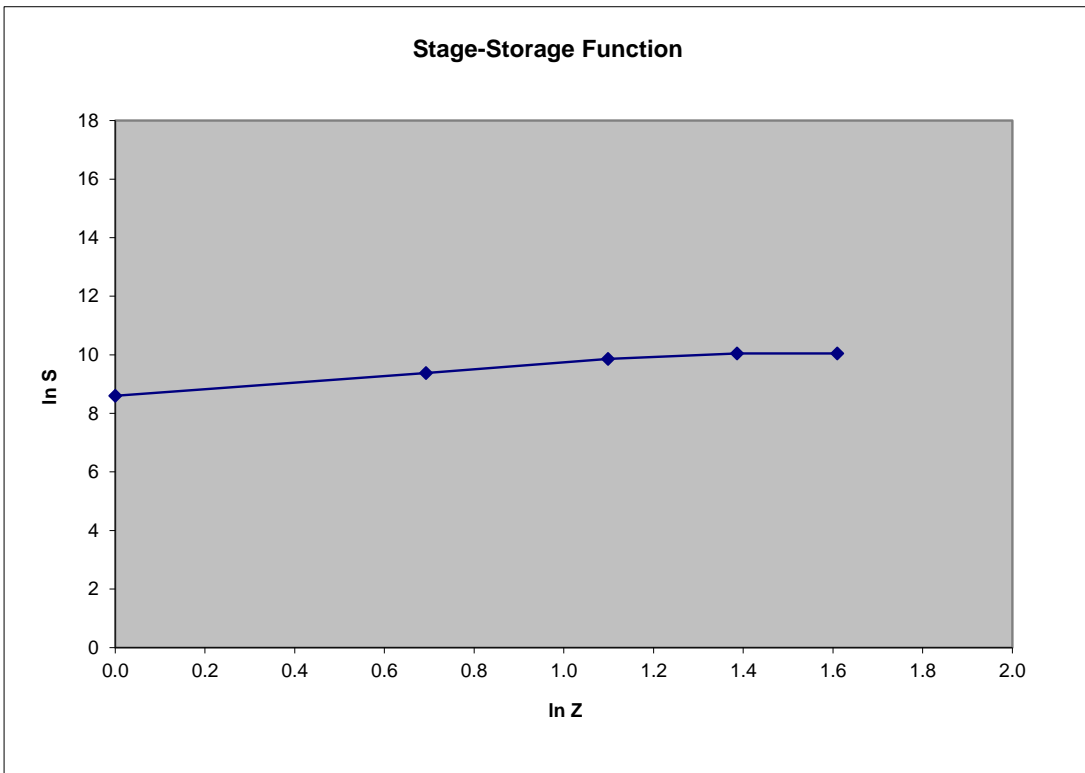
$$S = K_s * Z^b$$

where:

$b = 0.95$

and

$K_s = 5854$







# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	<b>2.30</b> ac	<b>Use Kirpich Equation:</b> Tc = <b>5.1 min</b>
Disturbed Area =	<b>2.30</b> ac	
Undisturbed Woods =	<b>0.00</b> ac	<b>Use Malcom Method:</b> i <sub>1</sub> = <b>5.70 in/hr</b> i <sub>10</sub> = <b>7.18 in/hr</b> i <sub>25</sub> = <b>8.24 in/hr</b>
Undisturbed Grass =	<b>0.00</b> ac	
Hydraulic Length =	<b>551</b> ft	
Vertical Fall =	<b>8</b> ft	<b>Use Rational Method:</b> Q <sub>2</sub> = <b>5.9 cfs</b> Q <sub>10</sub> = <b>7.4 cfs</b> Q <sub>25</sub> = <b>8.5 cfs</b>
For 1 yr Storm, g =	<b>132</b>	
h =	<b>18</b>	
For 10 yr Storm, g =	<b>195</b>	
h =	<b>22</b>	
For 25 yr Storm, g =	<b>232</b>	
h =	<b>23</b>	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u><u>0.45</u></u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	<b>2.7</b> in (2 Year Storm)	S = <b>0.99</b> in
CN =	<b>91</b> (newly graded areas)	Q* = <b>1.75</b> in
		T <sub>p2</sub> = <b>29.6</b> minutes
P <sub>n6</sub> =	<b>3.9</b> in (10 Year Storm)	Q* = <b>2.92</b> in
		T <sub>p10</sub> = <b>39.3</b> minutes
P <sub>n6</sub> =	<b>4.6</b> in (25 Year Storm)	Q* = <b>3.59</b> in
		T <sub>p25</sub> = <b>42.2</b> minutes

# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
	Peak Stage =	314.19	ft
	Rise =	1.19	ft
Freeboard =		1.81	ft
	Peak Outflow =	4.84	cfs

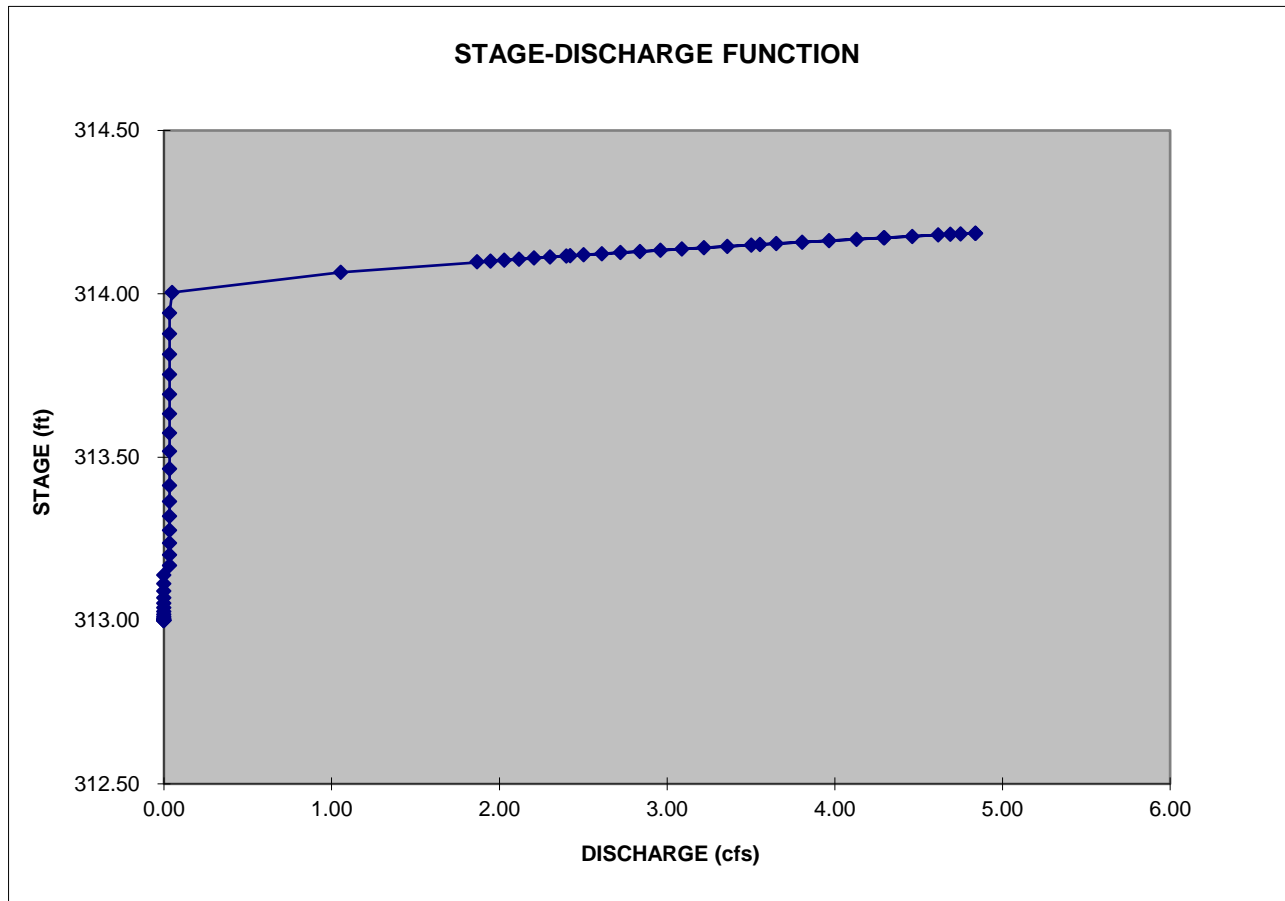
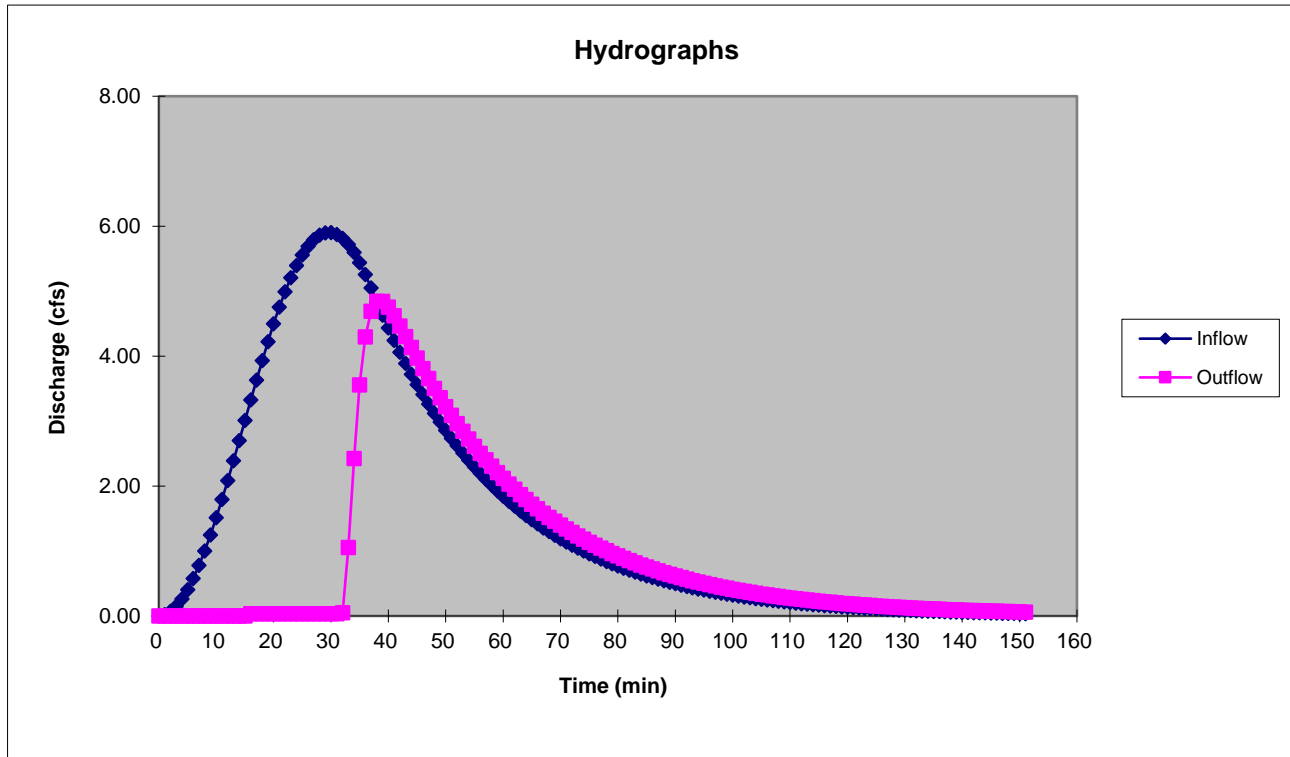
INPUT			
<b>Hydrograph Results:</b>		<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft
Qp =	5.90	cfs	N =
	29.6	min	L =
	1.0	min	Cw =
<b>Stage-Storage Results:</b>		Zcr =	314.00
Ks =	5854		
b =	0.95		
Z <sub>0</sub> =	313.0	ft (inv)	
<b>Initial Water Level:</b>			
Z <sub>i</sub> =	313.00	ft	
			<b>Skimmer Orifice:</b>
			Number =
			1.00
			Ea
			Diameter =
			2.00
			Inches
			Head =
			1.25
			inches

OUTPUT								
1	2	3	4	5	6	7	8	9
Time	Inflow	Storage	Stage	Outflow	Weir	Skimmer		
T	I	S	Z	O	Flow	Flow		
min	cts	cu-ft	ft	cts	cts	cts		
0	0	0.00	313.00	0.0	0.0	0.0		
1	0	0.00	313.00	0.0	0.0	0.0		
2	0	0.99	313.00	0.0	0.0	0.0		
3	0	4.96	313.00	0.0	0.0	0.0		
4	0	13.84	313.00	0.0	0.0	0.0		
5	0	29.53	313.00	0.0	0.0	0.0		
6	1	53.84	313.01	0.0	0.0	0.0		
7	1	88.48	313.01	0.0	0.0	0.0		
8	1	135.06	313.02	0.0	0.0	0.0		
9	1	195.04	313.03	0.0	0.0	0.0		
10	2	269.73	313.04	0.0	0.0	0.0		
11	2	360.29	313.05	0.0	0.0	0.0		
12	2	467.68	313.07	0.0	0.0	0.0		
13	2	592.70	313.09	0.0	0.0	0.0		
14	3	735.92	313.11	0.0	0.0	0.0		
15	3	897.73	313.14	0.0	0.0	0.0		
16	3	1078.29	313.17	0.0	0.0	0.0		0.03
17	4	1275.58	313.20	0.0	0.0	0.0		0.03
18	4	1491.33	313.24	0.0	0.0	0.0		0.03
19	4	1725.10	313.28	0.0	0.0	0.0		0.03
20	4	1976.22	313.32	0.0	0.0	0.0		0.03
21	5	2243.84	313.36	0.0	0.0	0.0		0.03
22	5	2526.93	313.41	0.0	0.0	0.0		0.03
23	5	2824.26	313.46	0.0	0.0	0.0		0.03
24	5	3134.47	313.52	0.0	0.0	0.0		0.03
25	6	3456.04	313.57	0.0	0.0	0.0		0.03
26	6	3787.33	313.63	0.0	0.0	0.0		0.03
27	6	4126.57	313.69	0.0	0.0	0.0		0.03
28	6	4471.94	313.75	0.0	0.0	0.0		0.03
29	6	4821.51	313.82	0.0	0.0	0.0		0.03
30	6	5173.32	313.88	0.0	0.0	0.0		0.03
31	6	5525.40	313.94	0.0	0.0	0.0		0.03
32	6	5875.75	314.00	0.0	0.0	0.0		0.03
33	6	6221.53	314.07	1.1	1.0	0.0		0.03
34	6	6501.36	314.12	2.4	2.4	0.0		0.03
35	5	6691.56	314.15	3.6	3.5	0.0		0.03
36	5	6804.63	314.17	4.3	4.3	0.0		0.03
37	5	6862.40	314.18	4.7	4.7	0.0		0.03
38	5	6883.93	314.19	4.8	4.8	0.0		0.03
39	5	6883.78	314.19	4.8	4.8	0.0		0.03
40	4	6871.24	314.18	4.8	4.7	0.0		0.03



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.75	ft	
Peak Stage =	314.25	ft	
Rise =	1.25	ft	
Peak Outflow =	7.35	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft	
Qp = 7.44 cfs	N = 1		
Tp = 39.3 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Number = 1 Ea	
Ks = 5854		Diameter = 2.00 Inches	
b = 0.95		Head = 1.25 inches	
Z <sub>0</sub> = 313.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 313.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	313.00	0.0	0.0	0.0		
1	0	0.00	313.00	0.0	0.0	0.00		
2	0	0.71	313.00	0.0	0.0	0.00		
3	0	3.55	313.00	0.0	0.0	0.00		
4	0	9.92	313.00	0.0	0.0	0.00		
5	0	21.21	313.00	0.0	0.0	0.00		
6	0	38.76	313.01	0.0	0.0	0.00		
7	1	63.89	313.01	0.0	0.0	0.00		
8	1	97.86	313.01	0.0	0.0	0.00		
9	1	141.87	313.02	0.0	0.0	0.00		
10	1	197.06	313.03	0.0	0.0	0.00		
11	1	264.51	313.04	0.0	0.0	0.00		
12	2	345.21	313.05	0.0	0.0	0.00		
13	2	440.06	313.07	0.0	0.0	0.00		
14	2	549.88	313.08	0.0	0.0	0.00		
15	2	675.40	313.10	0.0	0.0	0.00		
16	3	817.24	313.13	0.0	0.0	0.00		
17	3	975.91	313.15	0.0	0.0	0.00		
18	3	1151.82	313.18	0.0	0.0	0.03		
19	4	1343.29	313.21	0.0	0.0	0.03		
20	4	1552.48	313.25	0.0	0.0	0.03		
21	4	1779.49	313.29	0.0	0.0	0.03		
22	4	2024.27	313.33	0.0	0.0	0.03		
23	5	2286.67	313.37	0.0	0.0	0.03		
24	5	2566.43	313.42	0.0	0.0	0.03		
25	5	2863.18	313.47	0.0	0.0	0.03		
26	6	3176.42	313.53	0.0	0.0	0.03		
27	6	3505.58	313.58	0.0	0.0	0.03		
28	6	3849.97	313.64	0.0	0.0	0.03		
29	6	4208.79	313.71	0.0	0.0	0.03		
30	6	4581.17	313.77	0.0	0.0	0.03		
31	7	4966.15	313.84	0.0	0.0	0.03		
32	7	5362.69	313.91	0.0	0.0	0.03		
33	7	5769.65	313.98	0.0	0.0	0.03		
34	7	6185.86	314.06	0.9	0.9	0.03		
35	7	6557.57	314.13	2.7	2.7	0.03		
36	7	6826.01	314.18	4.4	4.4	0.03		
37	7	6998.01	314.21	5.7	5.6	0.03		
38	7	7100.49	314.23	6.4	6.4	0.03		
39	7	7158.94	314.24	6.9	6.9	0.03		
40	7	7191.08	314.24	7.2	7.1	0.03		
41	7	7207.59	314.24	7.3	7.3	0.03		
42	7	7214.47	314.25	7.3	7.3	0.03		
43	7	7214.94	314.25	7.3	7.3	0.03		
44	7	7210.72	314.25	7.3	7.3	0.03		
45	7	7202.74	314.24	7.2	7.2	0.03		
46	7	7191.48	314.24	7.2	7.1	0.03		

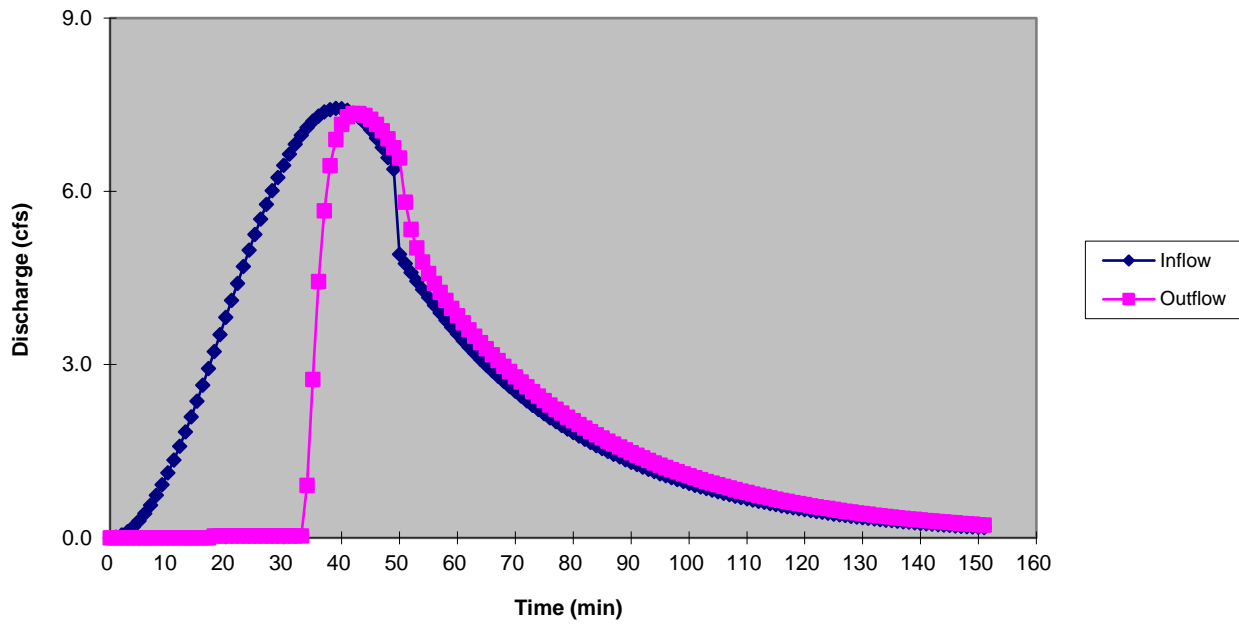
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	7	7177.24	314.24	7.0	7.0	0.03		
48	7	7160.20	314.24	6.9	6.9	0.03		
49	6	7140.49	314.23	6.8	6.7	0.03		
50	5	7118.22	314.23	6.6	6.5	0.03		
51	5	7018.01	314.21	5.8	5.8	0.03		
52	5	6954.18	314.20	5.3	5.3	0.03		
53	4	6909.34	314.19	5.0	5.0	0.03		
54	4	6874.89	314.18	4.8	4.7	0.03		
55	4	6846.36	314.18	4.6	4.5	0.03		
56	4	6821.33	314.17	4.4	4.4	0.03		
57	4	6798.48	314.17	4.3	4.2	0.03		
58	4	6777.05	314.17	4.1	4.1	0.03		
59	4	6756.62	314.16	4.0	3.9	0.03		
60	4	6736.94	314.16	3.8	3.8	0.03		
61	3	6717.87	314.16	3.7	3.7	0.03		
62	3	6699.32	314.15	3.6	3.6	0.03		
63	3	6681.22	314.15	3.5	3.5	0.03		
64	3	6663.55	314.15	3.4	3.3	0.03		
65	3	6646.28	314.14	3.3	3.2	0.03		
66	3	6629.39	314.14	3.2	3.1	0.03		
67	3	6612.86	314.14	3.1	3.0	0.03		
68	3	6596.70	314.13	3.0	2.9	0.03		
69	3	6580.88	314.13	2.9	2.8	0.03		
70	3	6565.39	314.13	2.8	2.8	0.03		
71	2	6550.24	314.13	2.7	2.7	0.03		
72	2	6535.41	314.12	2.6	2.6	0.03		
73	2	6520.90	314.12	2.5	2.5	0.03		
74	2	6506.69	314.12	2.5	2.4	0.03		
75	2	6492.78	314.12	2.4	2.3	0.03		
76	2	6479.17	314.11	2.3	2.3	0.03		
77	2	6465.85	314.11	2.2	2.2	0.03		
78	2	6452.81	314.11	2.2	2.1	0.03		
79	2	6440.05	314.11	2.1	2.1	0.03		
80	2	6427.55	314.10	2.0	2.0	0.03		
81	2	6415.32	314.10	2.0	1.9	0.03		
82	2	6403.35	314.10	1.9	1.9	0.03		
83	2	6391.63	314.10	1.8	1.8	0.03		
84	2	6380.16	314.09	1.8	1.7	0.03		
85	2	6368.93	314.09	1.7	1.7	0.03		
86	1	6357.94	314.09	1.7	1.6	0.03		
87	1	6347.18	314.09	1.6	1.6	0.03		
88	1	6336.65	314.09	1.6	1.5	0.03		
89	1	6326.34	314.09	1.5	1.5	0.03		
90	1	6316.25	314.08	1.5	1.4	0.03		
91	1	6306.37	314.08	1.4	1.4	0.03		
92	1	6296.70	314.08	1.4	1.3	0.03		
93	1	6287.23	314.08	1.3	1.3	0.03		
94	1	6277.96	314.08	1.3	1.3	0.03		
95	1	6268.89	314.07	1.3	1.2	0.03		
96	1	6260.00	314.07	1.2	1.2	0.03		
97	1	6251.31	314.07	1.2	1.1	0.03		
98	1	6242.79	314.07	1.1	1.1	0.03		
99	1	6234.46	314.07	1.1	1.1	0.03		
100	1	6226.30	314.07	1.1	1.0	0.03		
101	1	6218.31	314.07	1.0	1.0	0.03		
102	1	6210.49	314.06	1.0	1.0	0.03		
103	1	6202.83	314.06	1.0	0.9	0.03		
104	1	6195.33	314.06	0.9	0.9	0.03		
105	1	6187.99	314.06	0.9	0.9	0.03		
106	1	6180.80	314.06	0.9	0.9	0.03		
107	1	6173.76	314.06	0.9	0.8	0.03		
108	1	6166.87	314.06	0.8	0.8	0.03		
109	1	6160.12	314.06	0.8	0.8	0.03		
110	1	6153.52	314.05	0.8	0.8	0.03		
111	1	6147.05	314.05	0.8	0.7	0.03		
112	1	6140.71	314.05	0.7	0.7	0.03		
113	1	6134.51	314.05	0.7	0.7	0.03		
114	1	6128.44	314.05	0.7	0.7	0.03		
115	1	6122.49	314.05	0.7	0.6	0.03		
116	1	6116.66	314.05	0.6	0.6	0.03		
117	1	6110.96	314.05	0.6	0.6	0.03		
118	1	6105.37	314.05	0.6	0.6	0.03		
119	1	6099.90	314.04	0.6	0.6	0.03		
120	0	6094.54	314.04	0.6	0.5	0.03		
121	0	6089.30	314.04	0.6	0.5	0.03		
122	0	6084.16	314.04	0.5	0.5	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	6079.12	314.04	0.5	0.5	0.03		
124	0	6074.19	314.04	0.5	0.5	0.03		
125	0	6069.36	314.04	0.5	0.5	0.03		
126	0	6064.63	314.04	0.5	0.4	0.03		
127	0	6060.00	314.04	0.5	0.4	0.03		
128	0	6055.46	314.04	0.4	0.4	0.03		
129	0	6051.01	314.04	0.4	0.4	0.03		
130	0	6046.66	314.03	0.4	0.4	0.03		
131	0	6042.39	314.03	0.4	0.4	0.03		
132	0	6038.21	314.03	0.4	0.4	0.03		
133	0	6034.12	314.03	0.4	0.3	0.03		
134	0	6030.11	314.03	0.4	0.3	0.03		
135	0	6026.18	314.03	0.4	0.3	0.03		
136	0	6022.32	314.03	0.3	0.3	0.03		
137	0	6018.55	314.03	0.3	0.3	0.03		
138	0	6014.85	314.03	0.3	0.3	0.03		
139	0	6011.23	314.03	0.3	0.3	0.03		
140	0	6007.68	314.03	0.3	0.3	0.03		
141	0	6004.20	314.03	0.3	0.3	0.03		
142	0	6000.79	314.03	0.3	0.3	0.03		
143	0	5997.45	314.03	0.3	0.2	0.03		
144	0	5994.17	314.03	0.3	0.2	0.03		
145	0	5990.96	314.02	0.3	0.2	0.03		
146	0	5987.81	314.02	0.3	0.2	0.03		
147	0	5984.73	314.02	0.2	0.2	0.03		
148	0	5981.71	314.02	0.2	0.2	0.03		
149	0	5978.74	314.02	0.2	0.2	0.03		
150	0	5975.83	314.02	0.2	0.2	0.03		
151	0	5972.99	314.02	0.2	0.2	0.03		

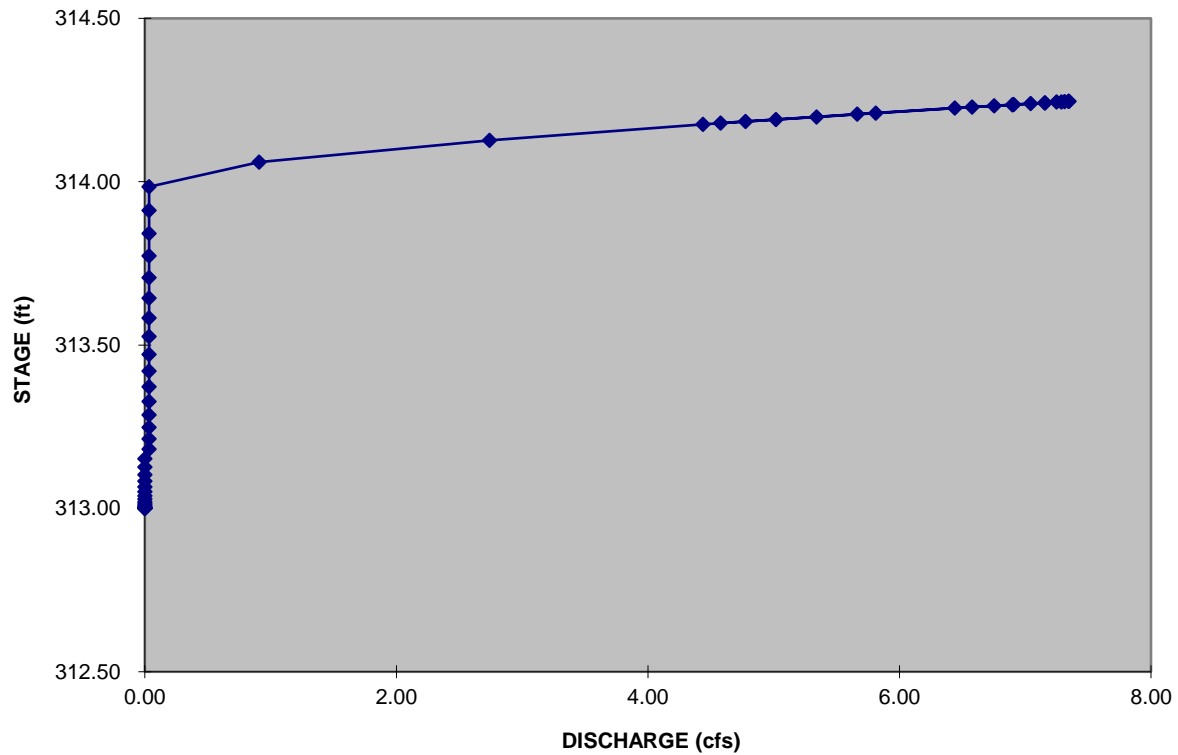
# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.73	ft	
Peak Stage =	314.27	ft	
Rise =	1.27	ft	
Peak Outflow =	8.49	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 316.00 ft	
Qp = 8.53 cfs	N = 1		
Tp = 39.3 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 314.00 ft	Number = 1.00 Ea	
Ks = 5854		Diameter = 2.00 Inches	
b = 0.95		Head = 1.25 inches	
Z <sub>0</sub> = 313.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 313.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	313.00	0.0	0.0	0.0		
1	0	0.00	313.00	0.0	0.0	0.00		
2	0	0.82	313.00	0.0	0.0	0.00		
3	0	4.07	313.00	0.0	0.0	0.00		
4	0	11.39	313.00	0.0	0.0	0.00		
5	0	24.34	313.00	0.0	0.0	0.00		
6	0	44.48	313.01	0.0	0.0	0.00		
7	1	73.32	313.01	0.0	0.0	0.00		
8	1	112.29	313.02	0.0	0.0	0.00		
9	1	162.79	313.02	0.0	0.0	0.00		
10	1	226.12	313.03	0.0	0.0	0.00		
11	2	303.52	313.04	0.0	0.0	0.00		
12	2	396.11	313.06	0.0	0.0	0.00		
13	2	504.95	313.08	0.0	0.0	0.00		
14	2	630.97	313.10	0.0	0.0	0.00		
15	3	775.00	313.12	0.0	0.0	0.00		
16	3	937.75	313.15	0.0	0.0	0.00		
17	3	1119.82	313.18	0.0	0.0	0.03		
18	4	1319.68	313.21	0.0	0.0	0.03		
19	4	1539.67	313.25	0.0	0.0	0.03		
20	4	1780.01	313.29	0.0	0.0	0.03		
21	5	2040.79	313.33	0.0	0.0	0.03		
22	5	2321.96	313.38	0.0	0.0	0.03		
23	5	2623.36	313.43	0.0	0.0	0.03		
24	6	2944.67	313.49	0.0	0.0	0.03		
25	6	3285.47	313.54	0.0	0.0	0.03		
26	6	3645.20	313.61	0.0	0.0	0.03		
27	7	4023.20	313.67	0.0	0.0	0.03		
28	7	4418.66	313.74	0.0	0.0	0.03		
29	7	4830.69	313.82	0.0	0.0	0.03		
30	7	5258.29	313.89	0.0	0.0	0.03		
31	8	5700.33	313.97	0.0	0.0	0.03		
32	8	6155.64	314.05	0.8	0.8	0.03		
33	8	6577.42	314.13	2.9	2.8	0.03		
34	8	6885.87	314.19	4.9	4.8	0.03		
35	8	7083.76	314.22	6.3	6.3	0.03		
36	8	7201.84	314.24	7.2	7.2	0.03		
37	8	7270.24	314.26	7.8	7.8	0.03		
38	9	7309.71	314.26	8.1	8.1	0.03		
39	9	7332.43	314.27	8.3	8.3	0.03		
40	9	7345.03	314.27	8.4	8.4	0.03		
41	8	7350.95	314.27	8.5	8.4	0.03		
42	8	7351.95	314.27	8.5	8.5	0.03		
43	8	7348.93	314.27	8.5	8.4	0.03		
44	8	7342.33	314.27	8.4	8.4	0.03		
45	8	7332.42	314.27	8.3	8.3	0.03		
46	8	7319.35	314.26	8.2	8.2	0.03		



OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	8	7303.24	314.26	8.1	8.0	0.03		
48	8	7284.17	314.26	7.9	7.9	0.03		
49	7	7262.24	314.25	7.7	7.7	0.03		
50	5	7237.54	314.25	7.5	7.5	0.03		
51	5	7080.12	314.22	6.3	6.3	0.03		
52	5	6988.04	314.20	5.6	5.6	0.03		
53	4	6928.31	314.19	5.2	5.1	0.03		
54	4	6885.73	314.19	4.9	4.8	0.03		
55	4	6852.63	314.18	4.6	4.6	0.03		
56	4	6825.00	314.18	4.4	4.4	0.03		
57	4	6800.65	314.17	4.3	4.2	0.03		
58	4	6778.34	314.17	4.1	4.1	0.03		
59	4	6757.40	314.16	4.0	3.9	0.03		
60	4	6737.42	314.16	3.8	3.8	0.03		
61	3	6718.16	314.16	3.7	3.7	0.03		
62	3	6699.49	314.15	3.6	3.6	0.03		
63	3	6681.33	314.15	3.5	3.5	0.03		
64	3	6663.62	314.15	3.4	3.3	0.03		
65	3	6646.32	314.14	3.3	3.2	0.03		
66	3	6629.41	314.14	3.2	3.1	0.03		
67	3	6612.88	314.14	3.1	3.0	0.03		
68	3	6596.71	314.13	3.0	2.9	0.03		
69	3	6580.88	314.13	2.9	2.8	0.03		
70	3	6565.40	314.13	2.8	2.8	0.03		
71	2	6550.25	314.13	2.7	2.7	0.03		
72	2	6535.41	314.12	2.6	2.6	0.03		
73	2	6520.90	314.12	2.5	2.5	0.03		
74	2	6506.69	314.12	2.5	2.4	0.03		
75	2	6492.78	314.12	2.4	2.3	0.03		
76	2	6479.17	314.11	2.3	2.3	0.03		
77	2	6465.85	314.11	2.2	2.2	0.03		
78	2	6452.81	314.11	2.2	2.1	0.03		
79	2	6440.05	314.11	2.1	2.1	0.03		
80	2	6427.55	314.10	2.0	2.0	0.03		
81	2	6415.32	314.10	2.0	1.9	0.03		
82	2	6403.35	314.10	1.9	1.9	0.03		
83	2	6391.63	314.10	1.8	1.8	0.03		
84	2	6380.16	314.09	1.8	1.7	0.03		
85	2	6368.93	314.09	1.7	1.7	0.03		
86	1	6357.94	314.09	1.7	1.6	0.03		
87	1	6347.18	314.09	1.6	1.6	0.03		
88	1	6336.65	314.09	1.6	1.5	0.03		
89	1	6326.34	314.09	1.5	1.5	0.03		
90	1	6316.25	314.08	1.5	1.4	0.03		
91	1	6306.37	314.08	1.4	1.4	0.03		
92	1	6296.70	314.08	1.4	1.3	0.03		
93	1	6287.23	314.08	1.3	1.3	0.03		
94	1	6277.96	314.08	1.3	1.3	0.03		
95	1	6268.89	314.07	1.3	1.2	0.03		
96	1	6260.00	314.07	1.2	1.2	0.03		
97	1	6251.31	314.07	1.2	1.1	0.03		
98	1	6242.79	314.07	1.1	1.1	0.03		
99	1	6234.46	314.07	1.1	1.1	0.03		
100	1	6226.30	314.07	1.1	1.0	0.03		
101	1	6218.31	314.07	1.0	1.0	0.03		
102	1	6210.49	314.06	1.0	1.0	0.03		
103	1	6202.83	314.06	1.0	0.9	0.03		
104	1	6195.33	314.06	0.9	0.9	0.03		
105	1	6187.99	314.06	0.9	0.9	0.03		
106	1	6180.80	314.06	0.9	0.9	0.03		
107	1	6173.76	314.06	0.9	0.8	0.03		
108	1	6166.87	314.06	0.8	0.8	0.03		
109	1	6160.12	314.06	0.8	0.8	0.03		
110	1	6153.52	314.05	0.8	0.8	0.03		
111	1	6147.05	314.05	0.8	0.7	0.03		
112	1	6140.71	314.05	0.7	0.7	0.03		
113	1	6134.51	314.05	0.7	0.7	0.03		
114	1	6128.44	314.05	0.7	0.7	0.03		
115	1	6122.49	314.05	0.7	0.6	0.03		
116	1	6116.66	314.05	0.6	0.6	0.03		
117	1	6110.96	314.05	0.6	0.6	0.03		
118	1	6105.37	314.05	0.6	0.6	0.03		
119	1	6099.90	314.04	0.6	0.6	0.03		
120	0	6094.54	314.04	0.6	0.5	0.03		
121	0	6089.30	314.04	0.6	0.5	0.03		
122	0	6084.16	314.04	0.5	0.5	0.03		

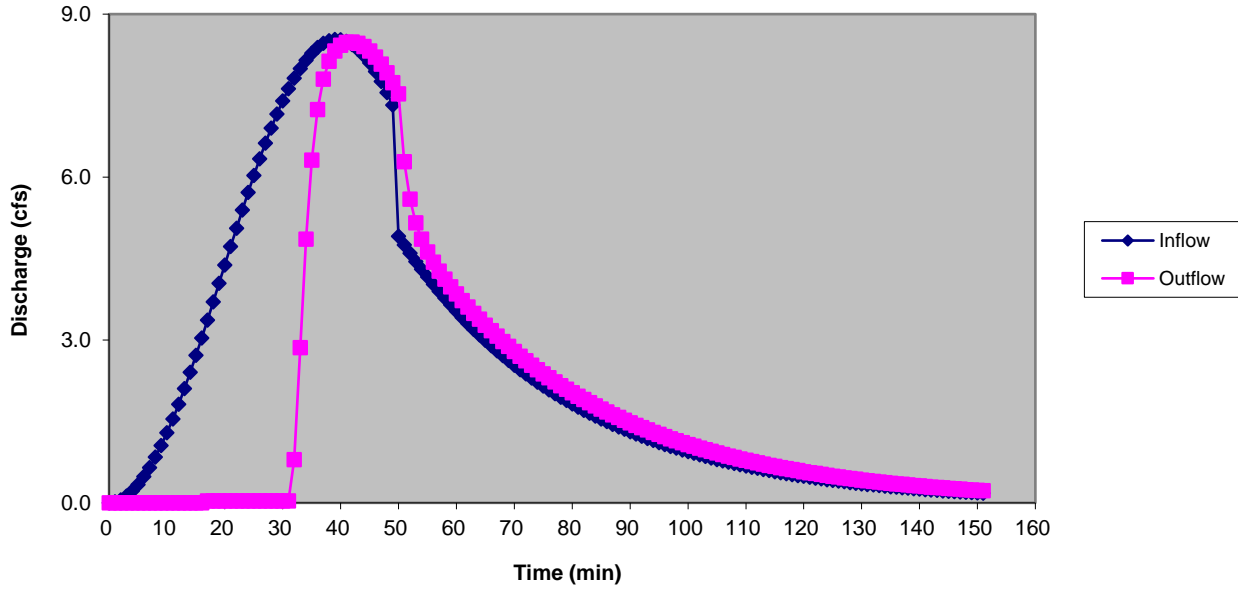
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	6079.12	314.04	0.5	0.5	0.03		
124	0	6074.19	314.04	0.5	0.5	0.03		
125	0	6069.36	314.04	0.5	0.5	0.03		
126	0	6064.63	314.04	0.5	0.4	0.03		
127	0	6060.00	314.04	0.5	0.4	0.03		
128	0	6055.46	314.04	0.4	0.4	0.03		
129	0	6051.01	314.04	0.4	0.4	0.03		
130	0	6046.66	314.03	0.4	0.4	0.03		
131	0	6042.39	314.03	0.4	0.4	0.03		
132	0	6038.21	314.03	0.4	0.4	0.03		
133	0	6034.12	314.03	0.4	0.3	0.03		
134	0	6030.11	314.03	0.4	0.3	0.03		
135	0	6026.18	314.03	0.4	0.3	0.03		
136	0	6022.32	314.03	0.3	0.3	0.03		
137	0	6018.55	314.03	0.3	0.3	0.03		
138	0	6014.85	314.03	0.3	0.3	0.03		
139	0	6011.23	314.03	0.3	0.3	0.03		
140	0	6007.68	314.03	0.3	0.3	0.03		
141	0	6004.20	314.03	0.3	0.3	0.03		
142	0	6000.79	314.03	0.3	0.3	0.03		
143	0	5997.45	314.03	0.3	0.2	0.03		
144	0	5994.17	314.03	0.3	0.2	0.03		
145	0	5990.96	314.02	0.3	0.2	0.03		
146	0	5987.81	314.02	0.3	0.2	0.03		
147	0	5984.73	314.02	0.2	0.2	0.03		
148	0	5981.71	314.02	0.2	0.2	0.03		
149	0	5978.74	314.02	0.2	0.2	0.03		
150	0	5975.83	314.02	0.2	0.2	0.03		
151	0	5972.99	314.02	0.2	0.2	0.03		



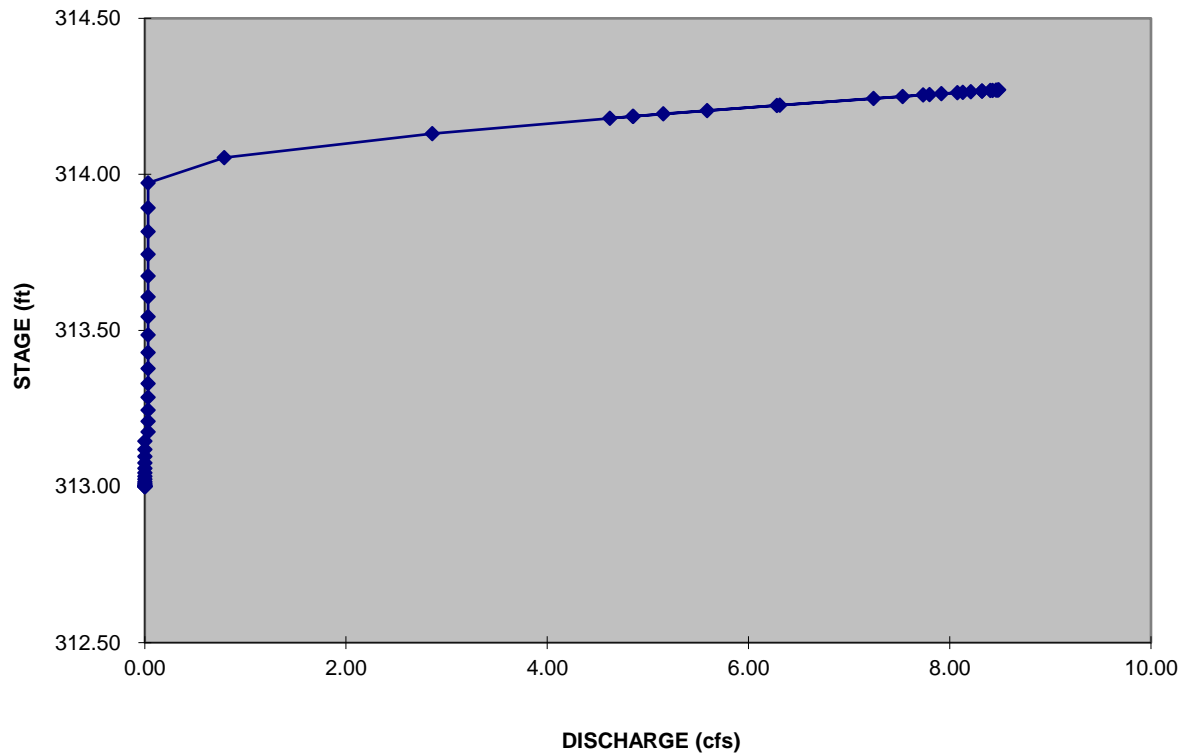
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-7  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID: ST-7

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	4.89 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	4.89 ac.	0.35
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>4.89 ac.</b>	<b>4.89 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	1064	feet	
Height of watershed =	12	feet	
Calculated t(c) =	9.4	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	9.4	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	4.82 in/hr
10	195	22	6.21 in/hr
25	232	23	7.16 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	4.82 in/hr	10.6 cfs	8.24 cfs
10-year storm	6.21 in/hr	13.7 cfs	10.63 cfs
25-year storm	7.16 in/hr	15.8 cfs	12.25 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	150
Bottom width =	75
Sediment depth =	1.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	1.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	3
Top of trap length =	168
Top of trap width =	93
Bottom elevation =	312

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	8802 cu. ft.	11940 cu. ft.
Sediment surface area =	4440 sq. ft.	12640 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	1
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.36 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.64 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 8,802 cubic feet

24 HOURS	Best Option =	1 - 3 inch Skimmers with a 2.8 inch orifice
2 DAYS	Best Option =	1 - 2.5 inch Skimmers with a 2.2 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.8 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.4 inch orifice

<<< USE


**McADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

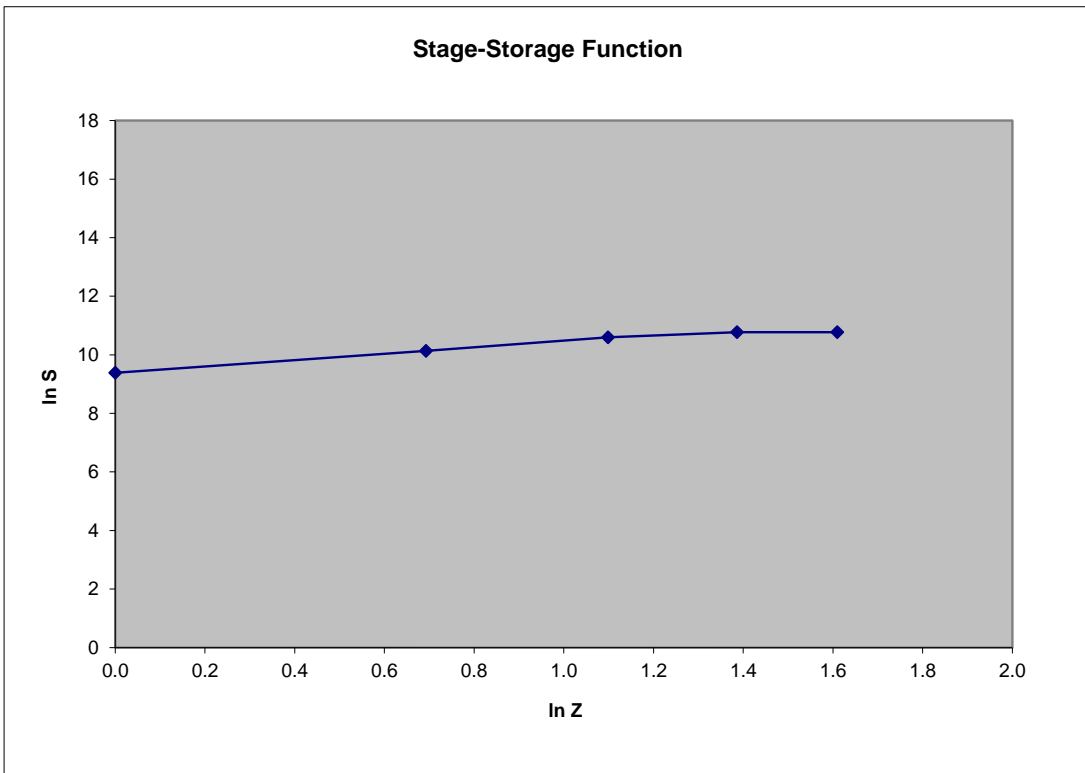
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
312	11229		0	0			
313	12581	11905	11905	1	9.3847	0.0000	0.92
314	13990	13285.5	25190.5	2	10.1342	0.6931	2.10
315	15456	14723	39913.5	3	10.5945	1.0986	3.48
316	0	7728	47641.5	4	10.7715	1.3863	4.22
317	0	0	47641.5	5	10.7715	1.6094	4.22

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 0.91$  and  $K_s = 12826$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	4.89 ac	<b>Use Kirpich Equation:</b> Tc = 9.4 min
Disturbed Area =	4.89 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 4.82 in/hr i <sub>10</sub> = 6.21 in/hr i <sub>25</sub> = 7.16 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	1064 ft	
Vertical Fall =	12 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 10.6 cfs Q <sub>10</sub> = 13.7 cfs Q <sub>25</sub> = 15.8 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 35.1 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 45.5 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 48.6 minutes



# McADAMS

## ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

### COMPUTED RESULTS

Freeboard =	1.75	ft	Peak Stage =	313.25	ft
			Rise =	1.25	ft
			Peak Outflow =	7.66	cfs

### INPUT

#### Hydrograph Results:

Qp = 10.60 cfs  
 Tp = 35.1 min  
 dT = 1.0 min

#### Weir:

N = 1  
 L = 20 ft  
 Cw = 3.00  
 Zcr = 313.00 ft

Top of Dam: 315.00 ft

#### Skimmer Orifice:

Number = 1.00 Ea  
 Diameter = 2.00 Inches  
 Head = 1.82 inches

#### Stage-Storage Results:

Ks = 12826  
 b = 0.91  
 Z<sub>0</sub> = 312.0 ft (inv)

#### Initial Water Level:

Z<sub>i</sub> = 312.00 ft

### OUTPUT

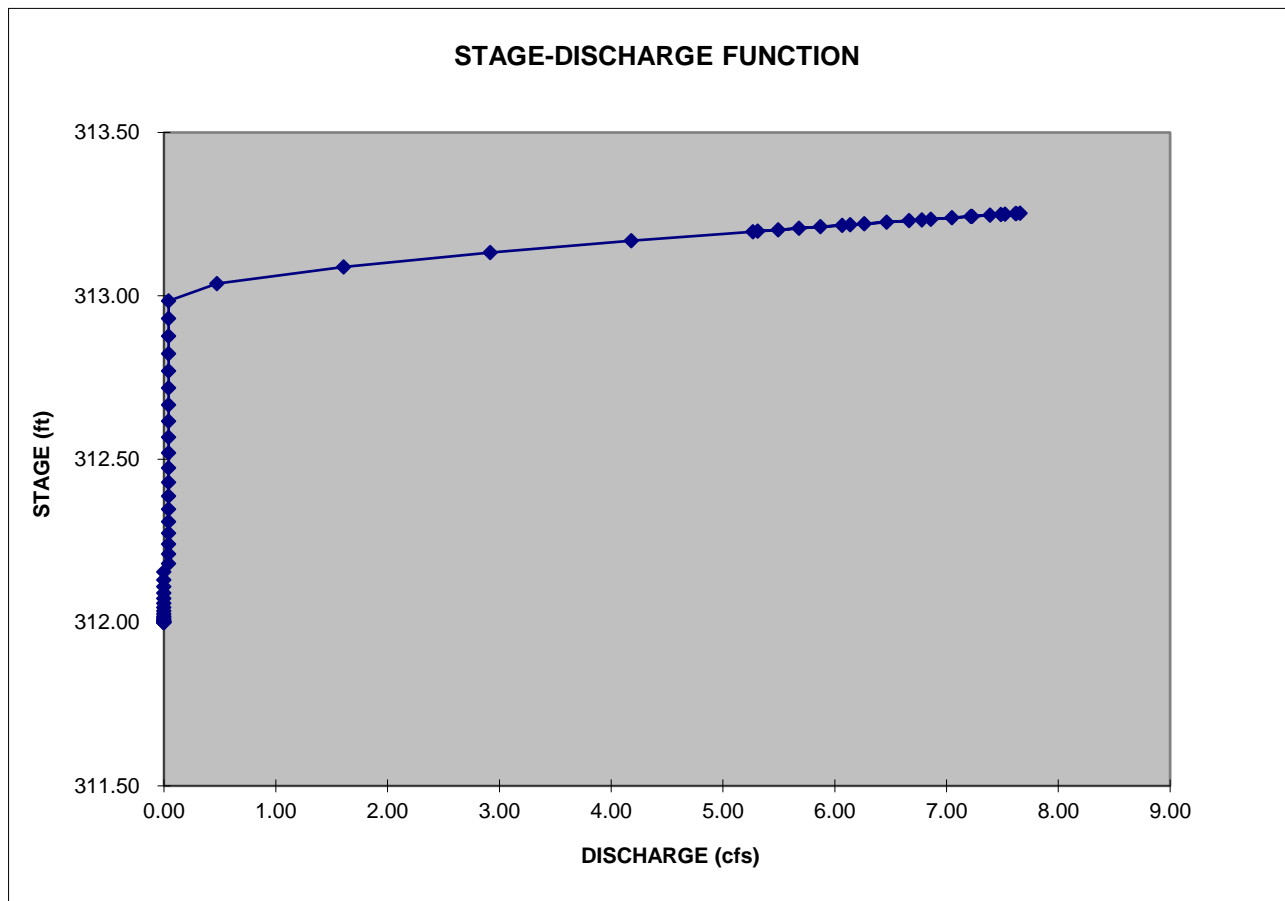
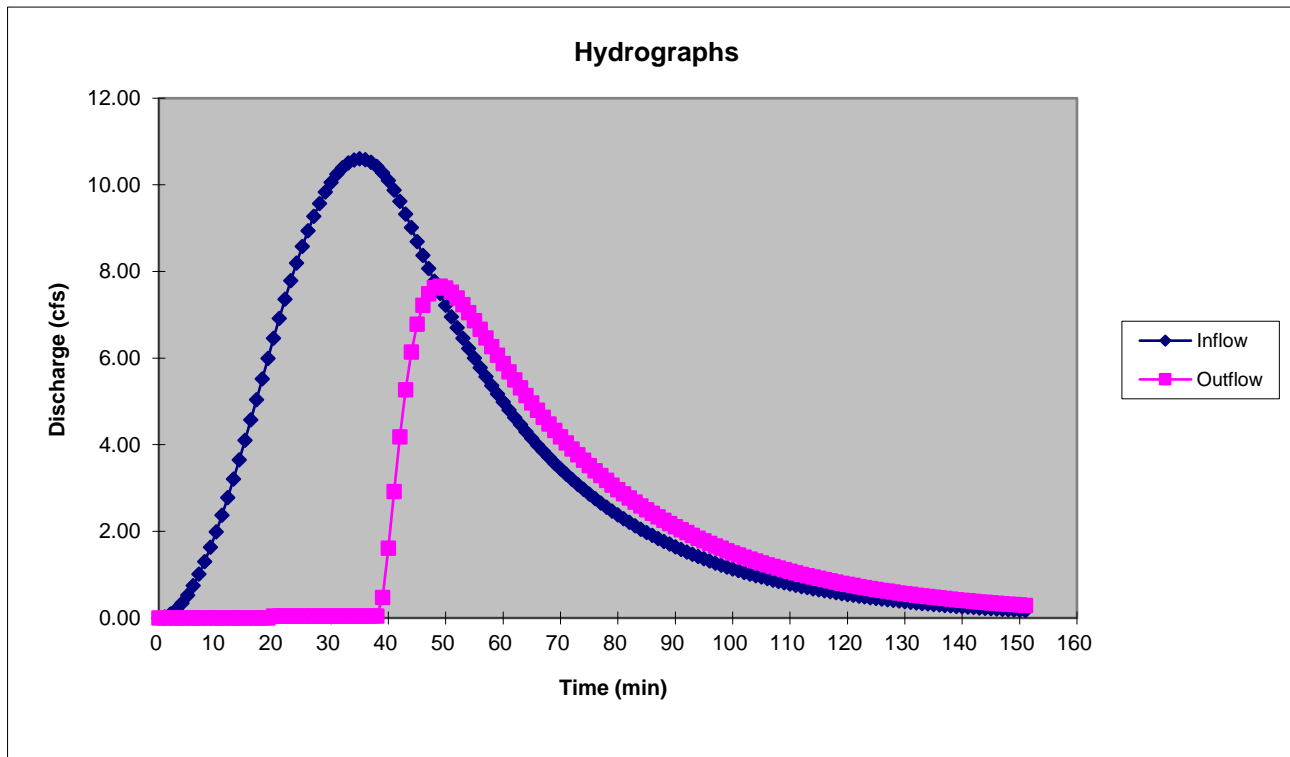
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.27	312.00	0.0	0.0	0.00		
3	0	6.36	312.00	0.0	0.0	0.00		
4	0	17.75	312.00	0.0	0.0	0.00		
5	1	37.92	312.00	0.0	0.0	0.00		
6	1	69.25	312.00	0.0	0.0	0.00		
7	1	114.03	312.01	0.0	0.0	0.00		
8	1	174.44	312.01	0.0	0.0	0.00		
9	2	252.56	312.01	0.0	0.0	0.00		
10	2	350.31	312.02	0.0	0.0	0.00		
11	2	469.44	312.03	0.0	0.0	0.00		
12	3	611.55	312.04	0.0	0.0	0.00		
13	3	778.04	312.05	0.0	0.0	0.00		
14	4	970.14	312.06	0.0	0.0	0.00		
15	4	1188.85	312.07	0.0	0.0	0.00		
16	5	1434.96	312.09	0.0	0.0	0.00		
17	5	1709.05	312.11	0.0	0.0	0.00		
18	6	2011.47	312.13	0.0	0.0	0.00		
19	6	2342.35	312.15	0.0	0.0	0.00		
20	6	2701.58	312.18	0.0	0.0	0.04		
21	7	3086.41	312.21	0.0	0.0	0.04		
22	7	3498.71	312.24	0.0	0.0	0.04		
23	8	3937.70	312.27	0.0	0.0	0.04		
24	8	4402.39	312.31	0.0	0.0	0.04		
25	9	4891.59	312.35	0.0	0.0	0.04		
26	9	5403.91	312.39	0.0	0.0	0.04		
27	9	5937.76	312.43	0.0	0.0	0.04		
28	10	6491.41	312.47	0.0	0.0	0.04		
29	10	7062.95	312.52	0.0	0.0	0.04		
30	10	7650.31	312.57	0.0	0.0	0.04		
31	10	8251.34	312.62	0.0	0.0	0.04		
32	10	8863.74	312.67	0.0	0.0	0.04		
33	11	9485.13	312.72	0.0	0.0	0.04		
34	11	10113.06	312.77	0.0	0.0	0.04		
35	11	10745.04	312.82	0.0	0.0	0.04		
36	11	11378.52	312.88	0.0	0.0	0.04		
37	11	12010.96	312.93	0.0	0.0	0.04		
38	10	12639.82	312.98	0.0	0.0	0.04		
39	10	13262.60	313.04	0.5	0.4	0.04		
40	10	13850.76	313.09	1.6	1.6	0.04		





# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.64	ft	
Peak Stage =	313.36	ft	
Rise =	1.36	ft	
Peak Outflow =	13.02	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 315.00 ft	
Qp = 13.66 cfs	N = 1		
Tp = 45.5 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
	Zcr = 313.00 ft	Number = 1	Ea
<b>Stage-Storage Results:</b>		Diameter = 2.00	Inches
Ks = 12826		Head = 1.82	inches
b = 0.91			
Z <sub>0</sub> = 312.0	ft (inv)		
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 312.00	ft		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	0.98	312.00	0.0	0.0	0.00		
3	0	4.88	312.00	0.0	0.0	0.00		
4	0	13.63	312.00	0.0	0.0	0.00		
5	0	29.16	312.00	0.0	0.0	0.00		
6	1	53.32	312.00	0.0	0.0	0.00		
7	1	87.98	312.00	0.0	0.0	0.00		
8	1	134.90	312.01	0.0	0.0	0.00		
9	1	195.82	312.01	0.0	0.0	0.00		
10	2	272.40	312.01	0.0	0.0	0.00		
11	2	366.22	312.02	0.0	0.0	0.00		
12	2	478.80	312.03	0.0	0.0	0.00		
13	3	611.55	312.04	0.0	0.0	0.00		
14	3	765.78	312.05	0.0	0.0	0.00		
15	3	942.73	312.06	0.0	0.0	0.00		
16	4	1143.48	312.07	0.0	0.0	0.00		
17	4	1369.05	312.09	0.0	0.0	0.00		
18	5	1620.31	312.10	0.0	0.0	0.00		
19	5	1898.01	312.12	0.0	0.0	0.00		
20	6	2202.79	312.14	0.0	0.0	0.00		
21	6	2535.14	312.17	0.0	0.0	0.04		
22	6	2893.01	312.19	0.0	0.0	0.04		
23	7	3279.07	312.22	0.0	0.0	0.04		
24	7	3693.41	312.25	0.0	0.0	0.04		
25	8	4136.00	312.29	0.0	0.0	0.04		
26	8	4606.67	312.32	0.0	0.0	0.04		
27	9	5105.12	312.36	0.0	0.0	0.04		
28	9	5630.92	312.40	0.0	0.0	0.04		
29	10	6183.50	312.45	0.0	0.0	0.04		
30	10	6762.18	312.50	0.0	0.0	0.04		
31	11	7366.13	312.54	0.0	0.0	0.04		
32	11	7994.43	312.60	0.0	0.0	0.04		
33	11	8646.01	312.65	0.0	0.0	0.04		
34	12	9319.72	312.70	0.0	0.0	0.04		
35	12	10014.30	312.76	0.0	0.0	0.04		
36	12	10728.36	312.82	0.0	0.0	0.04		
37	13	11460.46	312.88	0.0	0.0	0.04		
38	13	12209.04	312.95	0.0	0.0	0.04		
39	13	12972.48	313.01	0.1	0.1	0.04		
40	13	13744.04	313.08	1.4	1.3	0.04		
41	13	14452.34	313.14	3.2	3.1	0.04		
42	13	15061.26	313.19	5.1	5.1	0.04		
43	14	15561.60	313.24	6.9	6.9	0.04		
44	14	15958.78	313.27	8.5	8.5	0.04		
45	14	16265.51	313.30	9.8	9.8	0.04		
46	14	16496.86	313.32	10.8	10.8	0.04		

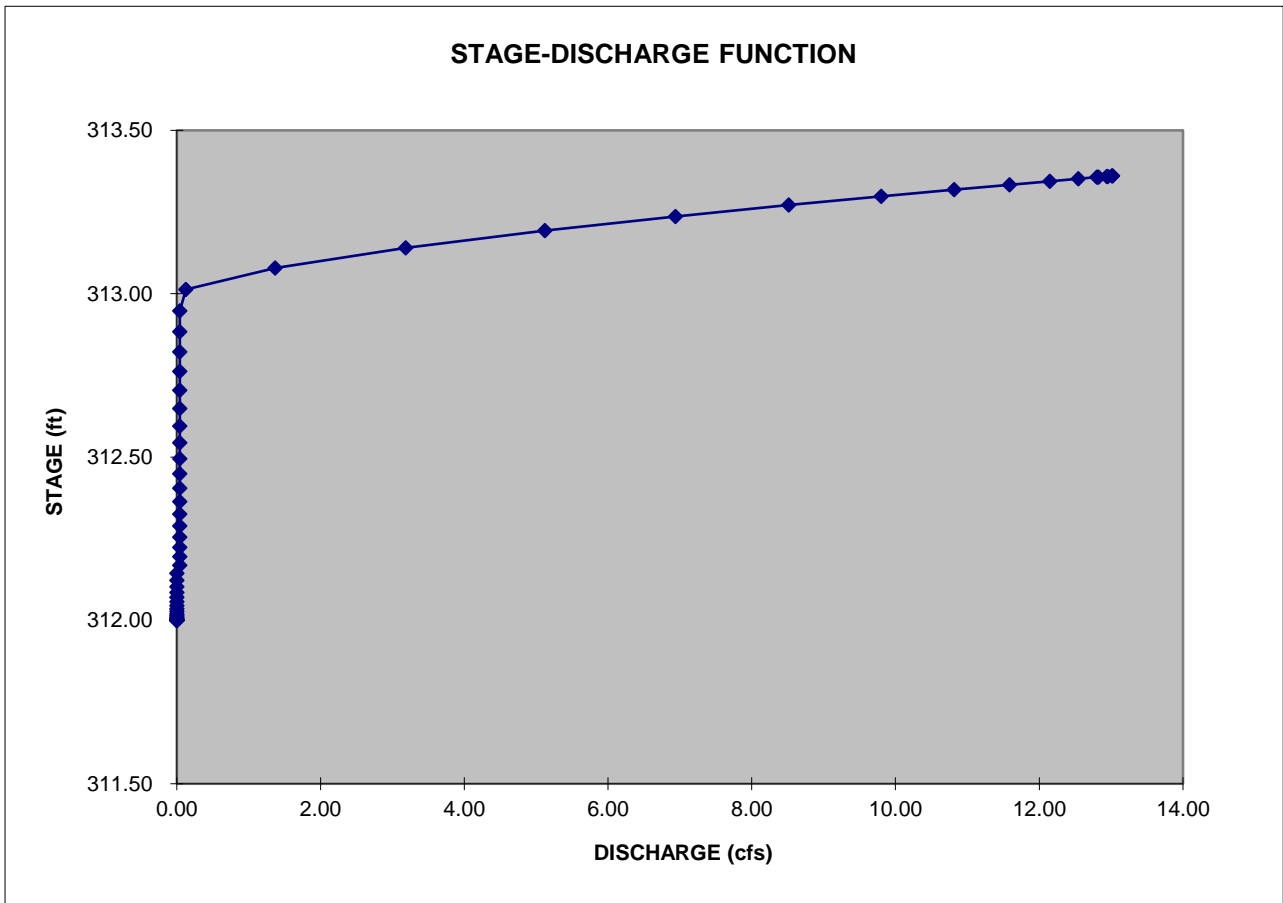
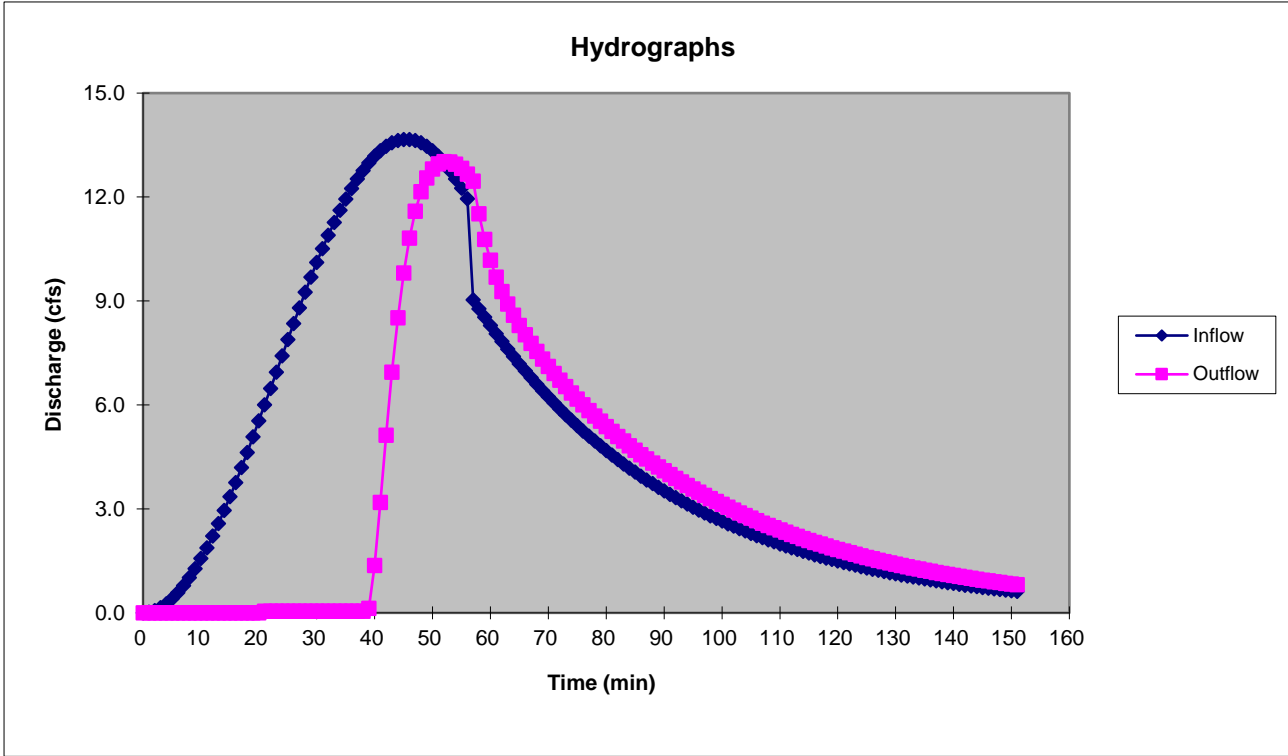
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	14	16667.44	313.33	11.6	11.5	0.04		
48	14	16789.93	313.34	12.1	12.1	0.04		
49	13	16874.74	313.35	12.5	12.5	0.04		
50	13	16930.01	313.36	12.8	12.8	0.04		
51	13	16961.98	313.36	13.0	12.9	0.04		
52	13	16975.32	313.36	13.0	13.0	0.04		
53	13	16973.50	313.36	13.0	13.0	0.04		
54	13	16959.09	313.36	12.9	12.9	0.04		
55	12	16933.96	313.36	12.8	12.8	0.04		
56	12	16899.54	313.35	12.7	12.6	0.04		
57	9	16856.87	313.35	12.5	12.4	0.04		
58	9	16650.99	313.33	11.5	11.5	0.04		
59	9	16486.90	313.32	10.8	10.7	0.04		
60	8	16352.30	313.31	10.2	10.1	0.04		
61	8	16238.87	313.30	9.7	9.6	0.04		
62	8	16140.88	313.29	9.3	9.2	0.04		
63	8	16054.29	313.28	8.9	8.9	0.04		
64	7	15976.26	313.27	8.6	8.5	0.04		
65	7	15904.76	313.27	8.3	8.3	0.04		
66	7	15838.29	313.26	8.0	8.0	0.04		
67	7	15775.79	313.26	7.8	7.7	0.04		
68	7	15716.47	313.25	7.5	7.5	0.04		
69	6	15659.75	313.25	7.3	7.3	0.04		
70	6	15605.19	313.24	7.1	7.1	0.04		
71	6	15552.47	313.24	6.9	6.9	0.04		
72	6	15501.34	313.23	6.7	6.7	0.04		
73	6	15451.63	313.23	6.5	6.5	0.04		
74	6	15403.17	313.22	6.3	6.3	0.04		
75	5	15355.87	313.22	6.2	6.1	0.04		
76	5	15309.64	313.21	6.0	6.0	0.04		
77	5	15264.41	313.21	5.8	5.8	0.04		
78	5	15220.11	313.21	5.7	5.6	0.04		
79	5	15176.72	313.20	5.5	5.5	0.04		
80	5	15134.18	313.20	5.4	5.3	0.04		
81	5	15092.46	313.20	5.2	5.2	0.04		
82	4	15051.55	313.19	5.1	5.1	0.04		
83	4	15011.41	313.19	5.0	4.9	0.04		
84	4	14972.02	313.19	4.8	4.8	0.04		
85	4	14933.36	313.18	4.7	4.7	0.04		
86	4	14895.42	313.18	4.6	4.5	0.04		
87	4	14858.18	313.18	4.4	4.4	0.04		
88	4	14821.62	313.17	4.3	4.3	0.04		
89	4	14785.72	313.17	4.2	4.2	0.04		
90	4	14750.49	313.17	4.1	4.1	0.04		
91	3	14715.89	313.16	4.0	3.9	0.04		
92	3	14681.93	313.16	3.9	3.8	0.04		
93	3	14648.58	313.16	3.8	3.7	0.04		
94	3	14615.84	313.15	3.7	3.6	0.04		
95	3	14583.69	313.15	3.6	3.5	0.04		
96	3	14552.12	313.15	3.5	3.4	0.04		
97	3	14521.12	313.15	3.4	3.3	0.04		
98	3	14490.68	313.14	3.3	3.3	0.04		
99	3	14460.80	313.14	3.2	3.2	0.04		
100	3	14431.45	313.14	3.1	3.1	0.04		
101	3	14402.64	313.14	3.0	3.0	0.04		
102	2	14374.34	313.13	3.0	2.9	0.04		
103	2	14346.55	313.13	2.9	2.8	0.04		
104	2	14319.27	313.13	2.8	2.8	0.04		
105	2	14292.48	313.13	2.7	2.7	0.04		
106	2	14266.17	313.12	2.7	2.6	0.04		
107	2	14240.33	313.12	2.6	2.5	0.04		
108	2	14214.96	313.12	2.5	2.5	0.04		
109	2	14190.05	313.12	2.5	2.4	0.04		
110	2	14165.59	313.12	2.4	2.3	0.04		
111	2	14141.56	313.11	2.3	2.3	0.04		
112	2	14117.97	313.11	2.3	2.2	0.04		
113	2	14094.80	313.11	2.2	2.2	0.04		
114	2	14072.05	313.11	2.1	2.1	0.04		
115	2	14049.71	313.11	2.1	2.0	0.04		
116	2	14027.77	313.10	2.0	2.0	0.04		
117	2	14006.22	313.10	2.0	1.9	0.04		
118	2	13985.06	313.10	1.9	1.9	0.04		
119	2	13964.28	313.10	1.9	1.8	0.04		
120	1	13943.87	313.10	1.8	1.8	0.04		
121	1	13923.83	313.09	1.8	1.7	0.04		
122	1	13904.15	313.09	1.7	1.7	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	13884.82	313.09	1.7	1.6	0.04		
124	1	13865.84	313.09	1.6	1.6	0.04		
125	1	13847.19	313.09	1.6	1.6	0.04		
126	1	13828.88	313.09	1.6	1.5	0.04		
127	1	13810.90	313.08	1.5	1.5	0.04		
128	1	13793.24	313.08	1.5	1.4	0.04		
129	1	13775.89	313.08	1.4	1.4	0.04		
130	1	13758.85	313.08	1.4	1.4	0.04		
131	1	13742.12	313.08	1.4	1.3	0.04		
132	1	13725.69	313.08	1.3	1.3	0.04		
133	1	13709.54	313.08	1.3	1.3	0.04		
134	1	13693.69	313.07	1.3	1.2	0.04		
135	1	13678.12	313.07	1.2	1.2	0.04		
136	1	13662.82	313.07	1.2	1.2	0.04		
137	1	13647.80	313.07	1.2	1.1	0.04		
138	1	13633.05	313.07	1.1	1.1	0.04		
139	1	13618.55	313.07	1.1	1.1	0.04		
140	1	13604.32	313.07	1.1	1.0	0.04		
141	1	13590.33	313.07	1.0	1.0	0.04		
142	1	13576.60	313.06	1.0	1.0	0.04		
143	1	13563.10	313.06	1.0	1.0	0.04		
144	1	13549.85	313.06	1.0	0.9	0.04		
145	1	13536.83	313.06	0.9	0.9	0.04		
146	1	13524.04	313.06	0.9	0.9	0.04		
147	1	13511.48	313.06	0.9	0.9	0.04		
148	1	13499.14	313.06	0.9	0.8	0.04		
149	1	13487.01	313.06	0.9	0.8	0.04		
150	1	13475.10	313.06	0.8	0.8	0.04		
151	1	13463.40	313.05	0.8	0.8	0.04		

# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.60	ft	
Peak Stage =	313.40	ft	
Rise =	1.40	ft	
Peak Outflow =	15.32	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 315.00 ft	
Qp = 15.75 cfs	N = 1		
Tp = 45.5 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00	<b>Skimmer Orifice:</b>	
<b>Stage-Storage Results:</b>	Zcr = 313.00 ft	Number = 1.00 Ea	
Ks = 12826		Diameter = 2.00 Inches	
b = 0.91		Head = 1.82 inches	
Z <sub>0</sub> = 312.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 312.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cfs	Storage S cu-ft	Stage Z ft	Outflow O cfs	Weir Flow cfs	Skimmer Flow cfs		
0	0	0.00	312.00	0.0	0.0	0.0		
1	0	0.00	312.00	0.0	0.0	0.00		
2	0	1.13	312.00	0.0	0.0	0.00		
3	0	5.62	312.00	0.0	0.0	0.00		
4	0	15.72	312.00	0.0	0.0	0.00		
5	0	33.62	312.00	0.0	0.0	0.00		
6	1	61.49	312.00	0.0	0.0	0.00		
7	1	101.44	312.00	0.0	0.0	0.00		
8	1	155.54	312.01	0.0	0.0	0.00		
9	1	225.78	312.01	0.0	0.0	0.00		
10	2	314.08	312.02	0.0	0.0	0.00		
11	2	422.27	312.02	0.0	0.0	0.00		
12	3	552.07	312.03	0.0	0.0	0.00		
13	3	705.13	312.04	0.0	0.0	0.00		
14	3	882.97	312.05	0.0	0.0	0.00		
15	4	1086.99	312.07	0.0	0.0	0.00		
16	4	1318.47	312.08	0.0	0.0	0.00		
17	5	1578.56	312.10	0.0	0.0	0.00		
18	5	1868.26	312.12	0.0	0.0	0.00		
19	6	2188.46	312.14	0.0	0.0	0.00		
20	6	2539.88	312.17	0.0	0.0	0.04		
21	7	2920.67	312.20	0.0	0.0	0.04		
22	7	3333.69	312.23	0.0	0.0	0.04		
23	8	3779.19	312.26	0.0	0.0	0.04		
24	9	4257.31	312.30	0.0	0.0	0.04		
25	9	4767.99	312.34	0.0	0.0	0.04		
26	10	5311.06	312.38	0.0	0.0	0.04		
27	10	5886.16	312.43	0.0	0.0	0.04		
28	11	6492.79	312.47	0.0	0.0	0.04		
29	11	7130.31	312.52	0.0	0.0	0.04		
30	12	7797.91	312.58	0.0	0.0	0.04		
31	12	8494.66	312.64	0.0	0.0	0.04		
32	13	9219.47	312.70	0.0	0.0	0.04		
33	13	9971.14	312.76	0.0	0.0	0.04		
34	13	10748.32	312.82	0.0	0.0	0.04		
35	14	11549.55	312.89	0.0	0.0	0.04		
36	14	12373.26	312.96	0.0	0.0	0.04		
37	14	13217.75	313.03	0.4	0.4	0.04		
38	15	14059.11	313.11	2.1	2.1	0.04		
39	15	14815.44	313.17	4.3	4.3	0.04		
40	15	15455.49	313.23	6.5	6.5	0.04		
41	15	15974.52	313.27	8.6	8.5	0.04		
42	16	16382.30	313.31	10.3	10.3	0.04		
43	16	16695.09	313.34	11.7	11.7	0.04		
44	16	16930.50	313.36	12.8	12.8	0.04		
45	16	17104.70	313.37	13.6	13.6	0.04		
46	16	17231.22	313.38	14.3	14.2	0.04		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	16	17320.80	313.39	14.7	14.7	0.04		
48	16	17381.65	313.40	15.0	15.0	0.04		
49	16	17419.86	313.40	15.2	15.2	0.04		
50	15	17439.92	313.40	15.3	15.3	0.04		
51	15	17445.07	313.40	15.3	15.3	0.04		
52	15	17437.66	313.40	15.3	15.2	0.04		
53	15	17419.40	313.40	15.2	15.1	0.04		
54	14	17391.51	313.40	15.1	15.0	0.04		
55	14	17354.93	313.39	14.9	14.8	0.04		
56	14	17310.34	313.39	14.6	14.6	0.04		
57	9	17258.30	313.39	14.4	14.3	0.04		
58	9	16936.73	313.36	12.8	12.8	0.04		
59	9	16693.05	313.34	11.7	11.7	0.04		
60	8	16502.60	313.32	10.8	10.8	0.04		
61	8	16349.40	313.31	10.2	10.1	0.04		
62	8	16222.73	313.29	9.6	9.6	0.04		
63	8	16115.27	313.28	9.2	9.1	0.04		
64	7	16021.93	313.28	8.8	8.7	0.04		
65	7	15939.12	313.27	8.4	8.4	0.04		
66	7	15864.25	313.26	8.1	8.1	0.04		
67	7	15795.49	313.26	7.9	7.8	0.04		
68	7	15731.46	313.25	7.6	7.6	0.04		
69	6	15671.20	313.25	7.4	7.3	0.04		
70	6	15613.97	313.24	7.1	7.1	0.04		
71	6	15559.22	313.24	6.9	6.9	0.04		
72	6	15506.55	313.23	6.7	6.7	0.04		
73	6	15455.65	313.23	6.5	6.5	0.04		
74	6	15406.29	313.22	6.4	6.3	0.04		
75	5	15358.30	313.22	6.2	6.1	0.04		
76	5	15311.54	313.21	6.0	6.0	0.04		
77	5	15265.89	313.21	5.8	5.8	0.04		
78	5	15221.28	313.21	5.7	5.6	0.04		
79	5	15177.63	313.20	5.5	5.5	0.04		
80	5	15134.90	313.20	5.4	5.3	0.04		
81	5	15093.03	313.20	5.2	5.2	0.04		
82	4	15052.00	313.19	5.1	5.1	0.04		
83	4	15011.76	313.19	5.0	4.9	0.04		
84	4	14972.30	313.19	4.8	4.8	0.04		
85	4	14933.59	313.18	4.7	4.7	0.04		
86	4	14895.60	313.18	4.6	4.5	0.04		
87	4	14858.32	313.18	4.4	4.4	0.04		
88	4	14821.73	313.17	4.3	4.3	0.04		
89	4	14785.82	313.17	4.2	4.2	0.04		
90	4	14750.56	313.17	4.1	4.1	0.04		
91	3	14715.96	313.16	4.0	3.9	0.04		
92	3	14681.98	313.16	3.9	3.8	0.04		
93	3	14648.62	313.16	3.8	3.7	0.04		
94	3	14615.87	313.15	3.7	3.6	0.04		
95	3	14583.71	313.15	3.6	3.5	0.04		
96	3	14552.14	313.15	3.5	3.4	0.04		
97	3	14521.14	313.15	3.4	3.3	0.04		
98	3	14490.70	313.14	3.3	3.3	0.04		
99	3	14460.81	313.14	3.2	3.2	0.04		
100	3	14431.46	313.14	3.1	3.1	0.04		
101	3	14402.64	313.14	3.0	3.0	0.04		
102	2	14374.35	313.13	3.0	2.9	0.04		
103	2	14346.56	313.13	2.9	2.8	0.04		
104	2	14319.27	313.13	2.8	2.8	0.04		
105	2	14292.48	313.13	2.7	2.7	0.04		
106	2	14266.17	313.12	2.7	2.6	0.04		
107	2	14240.34	313.12	2.6	2.5	0.04		
108	2	14214.97	313.12	2.5	2.5	0.04		
109	2	14190.05	313.12	2.5	2.4	0.04		
110	2	14165.59	313.12	2.4	2.3	0.04		
111	2	14141.56	313.11	2.3	2.3	0.04		
112	2	14117.97	313.11	2.3	2.2	0.04		
113	2	14094.80	313.11	2.2	2.2	0.04		
114	2	14072.05	313.11	2.1	2.1	0.04		
115	2	14049.71	313.11	2.1	2.0	0.04		
116	2	14027.77	313.10	2.0	2.0	0.04		
117	2	14006.22	313.10	2.0	1.9	0.04		
118	2	13985.06	313.10	1.9	1.9	0.04		
119	2	13964.28	313.10	1.9	1.8	0.04		
120	1	13943.87	313.10	1.8	1.8	0.04		
121	1	13923.83	313.09	1.8	1.7	0.04		
122	1	13904.15	313.09	1.7	1.7	0.04		

**OUTPUT**

1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	1	13884.82	313.09	1.7	1.6	0.04		
124	1	13865.84	313.09	1.6	1.6	0.04		
125	1	13847.19	313.09	1.6	1.6	0.04		
126	1	13828.88	313.09	1.6	1.5	0.04		
127	1	13810.90	313.08	1.5	1.5	0.04		
128	1	13793.24	313.08	1.5	1.4	0.04		
129	1	13775.89	313.08	1.4	1.4	0.04		
130	1	13758.85	313.08	1.4	1.4	0.04		
131	1	13742.12	313.08	1.4	1.3	0.04		
132	1	13725.69	313.08	1.3	1.3	0.04		
133	1	13709.54	313.08	1.3	1.3	0.04		
134	1	13693.69	313.07	1.3	1.2	0.04		
135	1	13678.12	313.07	1.2	1.2	0.04		
136	1	13662.82	313.07	1.2	1.2	0.04		
137	1	13647.80	313.07	1.2	1.1	0.04		
138	1	13633.05	313.07	1.1	1.1	0.04		
139	1	13618.55	313.07	1.1	1.1	0.04		
140	1	13604.32	313.07	1.1	1.0	0.04		
141	1	13590.33	313.07	1.0	1.0	0.04		
142	1	13576.60	313.06	1.0	1.0	0.04		
143	1	13563.10	313.06	1.0	1.0	0.04		
144	1	13549.85	313.06	1.0	0.9	0.04		
145	1	13536.83	313.06	0.9	0.9	0.04		
146	1	13524.04	313.06	0.9	0.9	0.04		
147	1	13511.48	313.06	0.9	0.9	0.04		
148	1	13499.14	313.06	0.9	0.8	0.04		
149	1	13487.01	313.06	0.9	0.8	0.04		
150	1	13475.10	313.06	0.8	0.8	0.04		
151	1	13463.40	313.05	0.8	0.8	0.04		

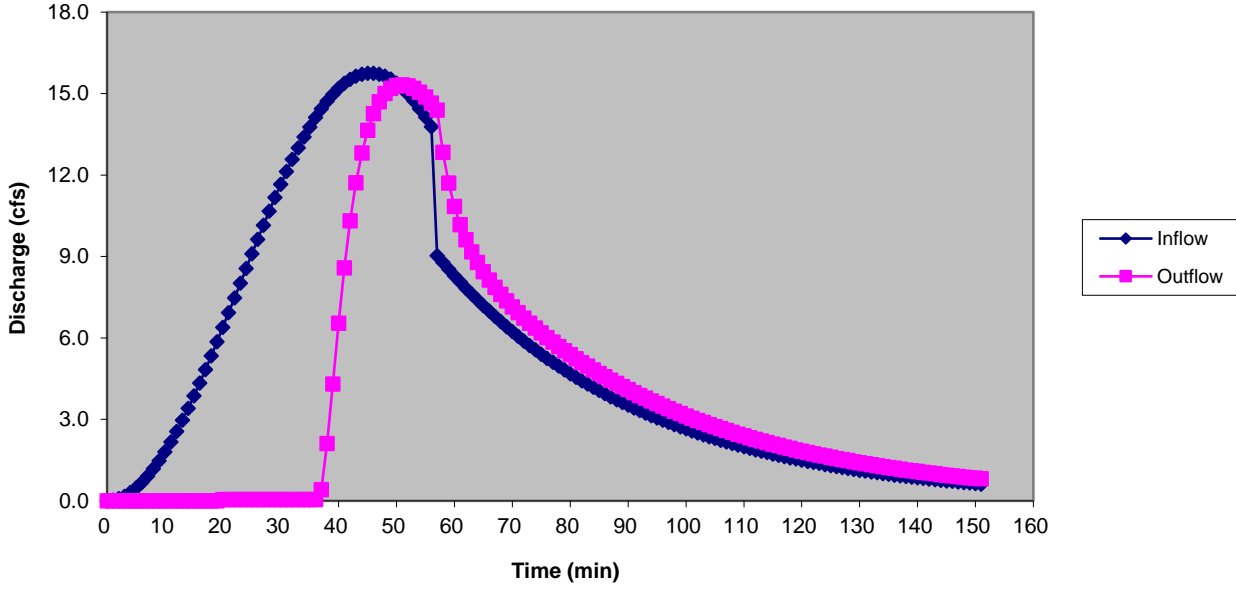




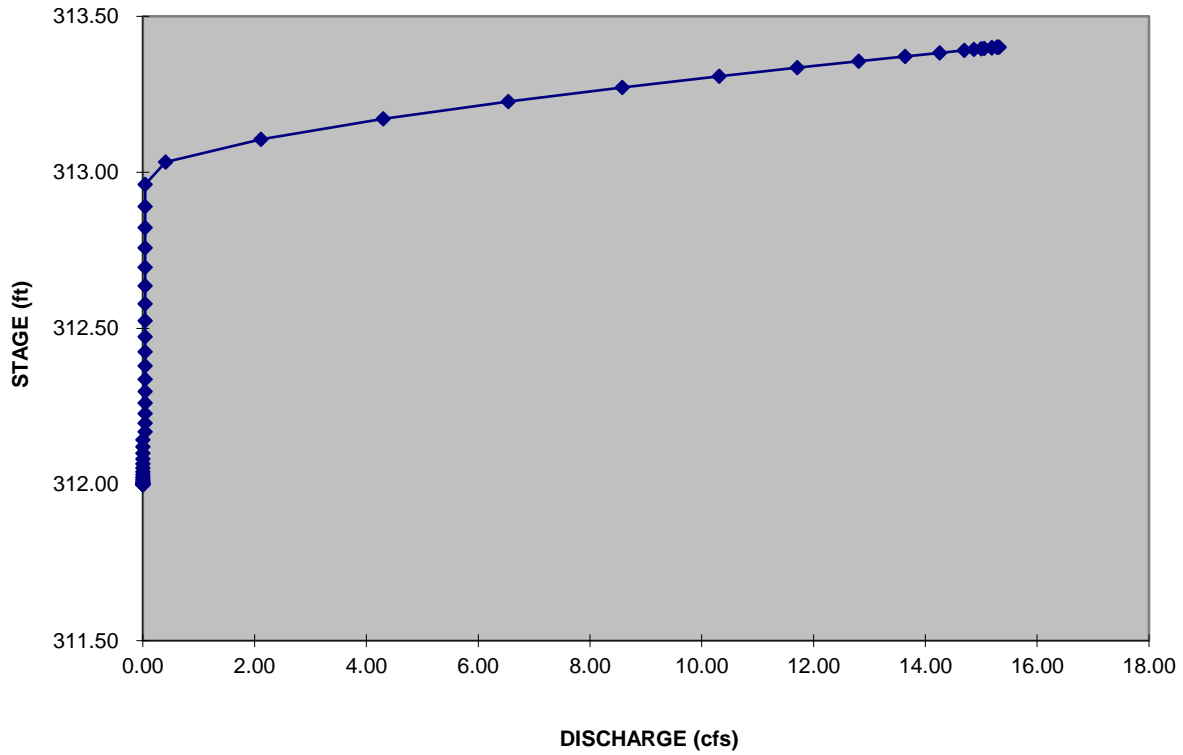
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION





**ST-8  
SKIMMER BASIN  
DESIGN**

**CHAMBLEE LAKE  
ZEBULON, NC  
DRH22004**

Skimmer Basin Sizing  
Stage-Storage Function  
Hydrograph Formulation  
Routing for the 2-year storm  
Graphed Data for the 2-year storm  
Routing for the 10-year storm  
Graphed Data for the 10-year storm

# McADAMS

## SKIMMER BASIN SIZING

Project Name: CHAMBLEE LAKE  
 Project Number: DRH22004  
 Sediment Trap ID ST-8

Date: 2/27/2024  
 Calculated By: BJK

### Drainage Area Data

	Disturbed Area	Pre-Developed Area	Rational "C"
Disturbed area	1.41 ac.	0.00 ac.	0.45
Undisturbed wooded area	0.00 ac.	1.41 ac.	0.35
Undisturbed grassy area	0.00 ac.	0.00 ac.	0.25
Other (Impervious)	0.00 ac.	0.00 ac.	0.95
<b>Total drainage area to trap</b>	<b>1.41 ac.</b>	<b>1.41 ac.</b>	
<b>Composite Rational 'c'</b>	<b>0.45</b>	<b>0.35</b>	

### Time of Concentration

Use Kerpich (USBR, 1974) equation to calculate time of concentration - t(c)

K =	1		
Length of flow path =	466	feet	
Height of watershed =	13	feet	
Calculated t(c) =	3.5	minutes	
Minimum t(c) =	5.0	minutes	
Time of concentration =	5.0	minutes	

K Values	
Overland on grassed surfaces:	2.0
Overland on paved surfaces:	0.4
Channel in natural channels:	1.0
Channel in mixed urban setting:	1.1
Channel in paved pipes or channels:	0.2

### Calculate Rainfall Intensity

IDF curves for Raleigh, NC

Return Period	B	D	Intensity
2	132	18	5.74 in/hr
10	195	22	7.22 in/hr
25	232	23	8.29 in/hr

### Runoff Calculations

Return Period	Rainfall	Runoff	Pre-Developed Runoff
2-year storm	5.74 in/hr	3.6 cfs	2.83 cfs
10-year storm	7.22 in/hr	4.6 cfs	3.56 cfs
25-year storm	8.29 in/hr	5.3 cfs	4.09 cfs

### Sediment Trap Design Data and Constraints

Required sediment trap volume	1,800	cubic feet per disturbed acre
Required sediment trap surface area	325	square feet per 1 cfs of Q
Design Return Period	10	Year

### Sediment Trap Dimensions

Bottom length =	60
Bottom width =	30
Sediment depth =	2.0
Freeboard (from sediment depth) =	2.0
Depth to crest of Emergency Spillway =	2.0
Side slopes =	3.0 H:1V
Emergency Spillway length =	20
Height of berm =	4
Top of trap length =	84
Top of trap width =	54
Bottom elevation =	311

### Sediment Trap Data

	Required	Provided
Sediment storage volume =	2538 cu. ft.	4770 cu. ft.
Sediment surface area =	1490 sq. ft.	3020 sq. ft.
Sediment storage depth =	3.5 ft. (max.)	2
Trap bottom length to width ratio =	2.0L:1W (min)	2.0L:1W
Spillway length =	10.0 ft. (min)	20
10-Year flow depth over spillway =	0.50 ft. (max)	0.18 ft
Freeboard at 10-Year discharge =	1.00 ft. (min)	1.82 ft

1. Reference for Rational Coefficients:  
 Malcom, H. Rooney, Elements of Urban Stormwater Design, NCSU  
 1999, Chapter 2 (Hydrologic Estimates)

### SIZING FAIRCLOTH SKIMMER AND ORIFICE

Volume Provided for Sediment Basin = 2,538 cubic feet

24 HOURS	Best Option =	1 - 2 inch Skimmers with a 1.7 inch orifice
2 DAYS	Best Option =	1 - 2 inch Skimmers with a 1.2 inch orifice
3 DAYS	Best Option =	1 - 2 inch Skimmers with a 1 inch orifice
4 DAYS	Best Option =	1 - 2 inch Skimmers with a 0.8 inch orifice

<<< USE


**MCADAMS**  
**STAGE-STORAGE FUNCTION**  
 CHAMBLEE LAKE

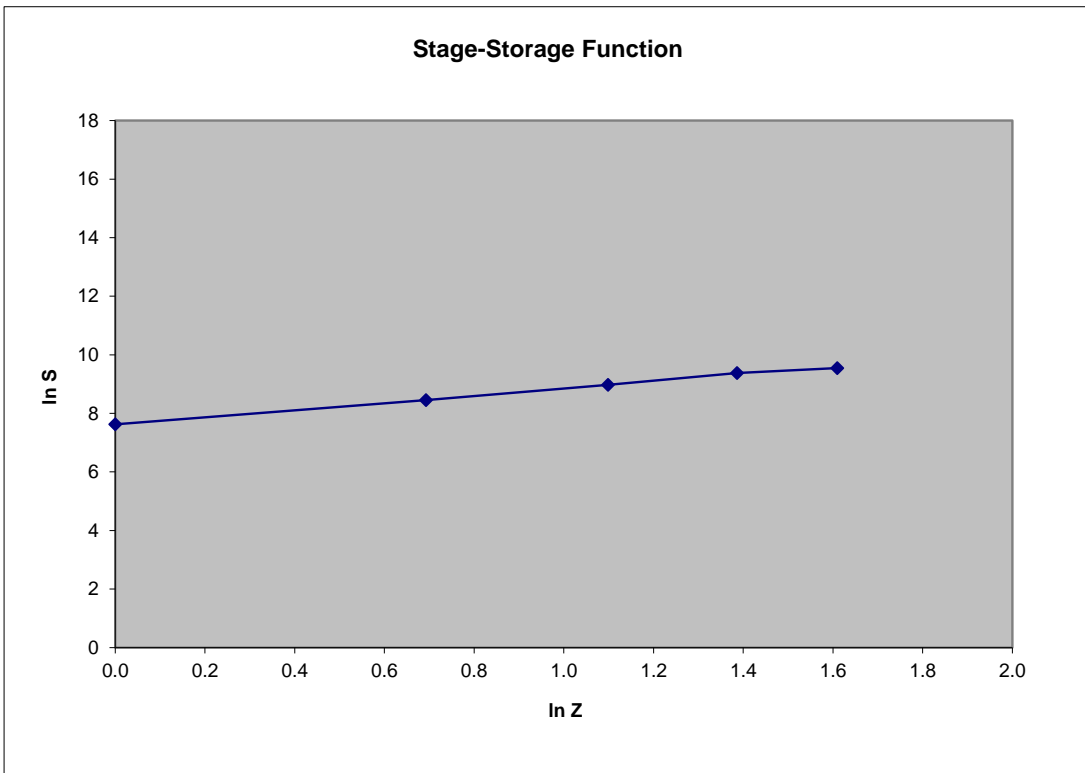
**AVERAGE-END CALCULATIONS:**

1	2	3	4	5	6	7	8
CONTOUR	CONTOUR AREA sq ft	INCR VOLUME cu ft	ACCUM VOLUME cu ft	Z STAGE ft	Y ln S	X ln Z	Z est ft
311	1779		0	0			
312	2321	2050	2050	1	7.6256	0.0000	1.00
313	2920	2620.5	4670.5	2	8.4490	0.6931	1.96
314	3578	3249	7919.5	3	8.9771	1.0986	3.02
315	4288	3933	11852.5	4	9.3803	1.3863	4.20
316	0	2144	13996.5	5	9.5466	1.6094	4.81

**STAGE-STORAGE FUNCTION:**

$$S = K_s * Z^b$$

where:  $b = 1.22$  and  $K_s = 2046$





# MCADAMS

## HYDROGRAPH FORMULATION

### CHAMBLEE LAKE

REFERENCES: ( Malcom, 1991; Wanielista, 1990 )

**SOLUTION - Qp:** Use the Kirpich Equation & the Rational Method to compute Qp for Post-Development Condition.

INPUT		OUTPUT
Drainage Area, DA =	1.41 ac	<b>Use Kirpich Equation:</b> Tc = 5.0 min
Disturbed Area =	1.41 ac	
Undisturbed Woods =	0.00 ac	<b>Use Malcom Method:</b> i <sub>1</sub> = 5.74 in/hr i <sub>10</sub> = 7.22 in/hr i <sub>25</sub> = 8.29 in/hr
Undisturbed Grass =	0.00 ac	
Hydraulic Length =	466 ft	
Vertical Fall =	13 ft	<b>Use Rational Method:</b> Q <sub>2</sub> = 3.6 cfs Q <sub>10</sub> = 4.6 cfs Q <sub>25</sub> = 5.3 cfs
For 1 yr Storm, g =	132	
h =	18	
For 10 yr Storm, g =	195	
h =	22	
For 25 yr Storm, g =	232	
h =	23	
"C" CALCULATION - See Sediment Trap Sizing		
"C" = <u>0.45</u>		

**SOLUTION - Tp:**

INPUT		OUTPUT
P <sub>n6</sub> =	2.7 in (2 Year Storm)	S = 0.99 in
CN =	91 (newly graded areas)	Q* = 1.75 in
		T <sub>p2</sub> = 29.4 minutes
P <sub>n6</sub> =	3.9 in (10 Year Storm)	Q* = 2.92 in
		T <sub>p10</sub> = 39.1 minutes
P <sub>n6</sub> =	4.6 in (25 Year Storm)	Q* = 3.59 in
		T <sub>p25</sub> = 42.0 minutes



# ROUTING FOR 2-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.88	ft	
Peak Stage =	313.12	ft	
Rise =	2.12	ft	
Peak Outflow =	2.54	cfs	

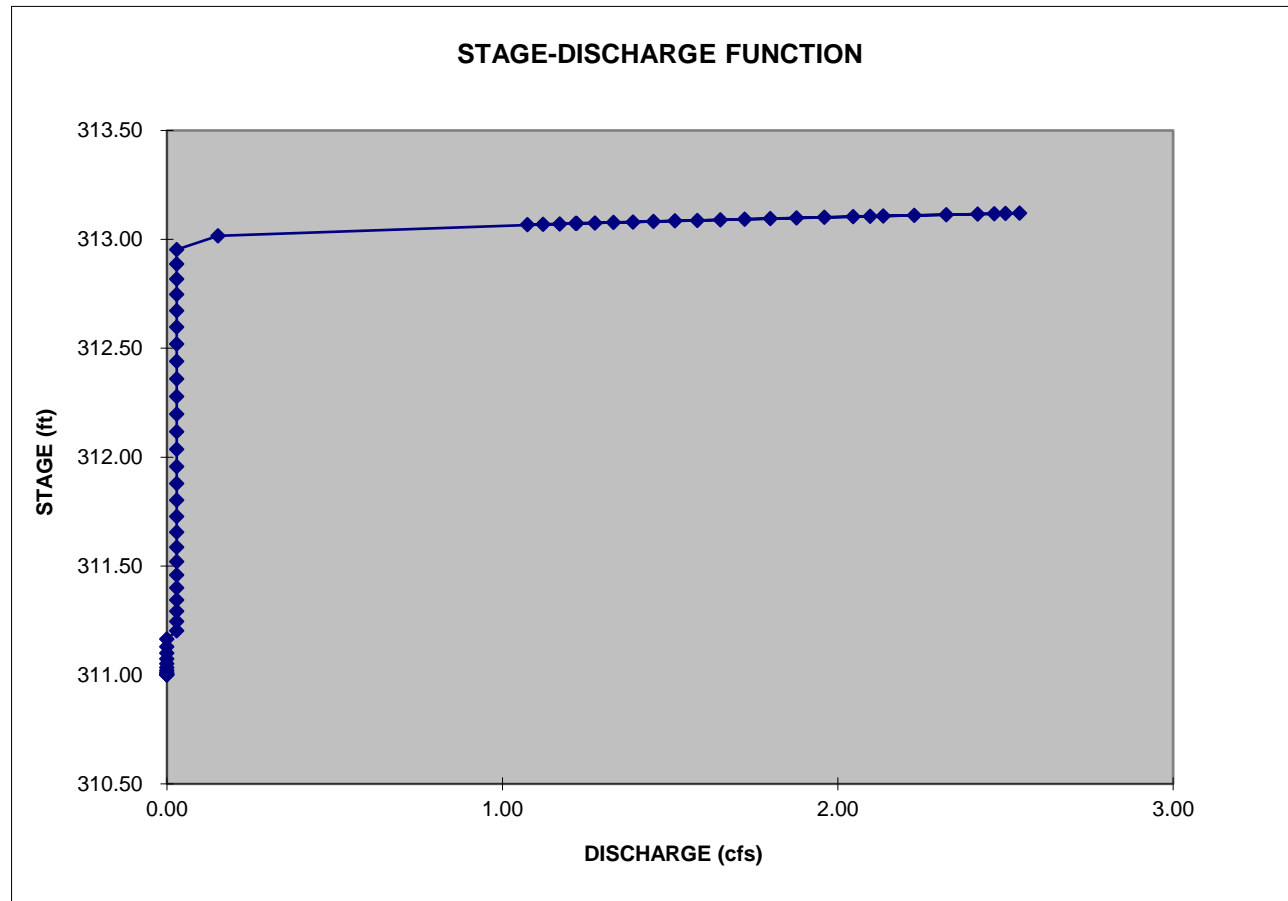
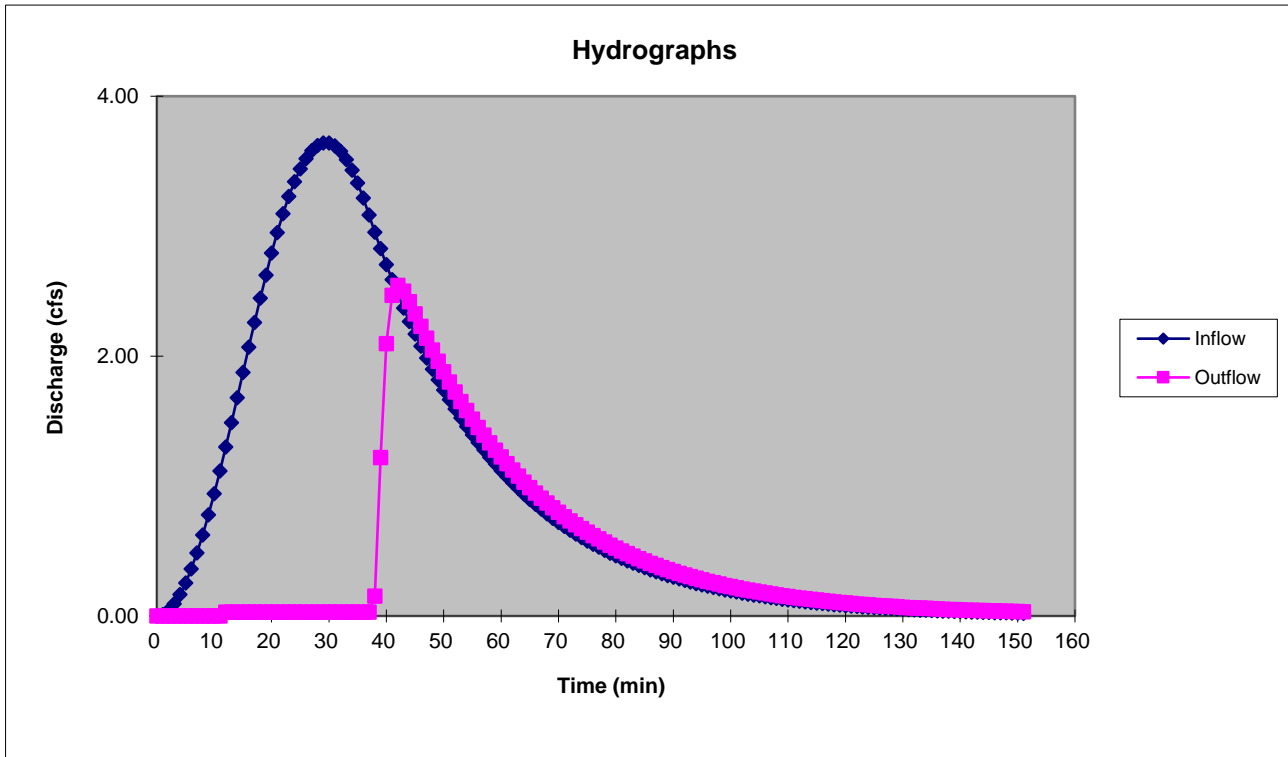
INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b> 315.00 ft	
Qp = 3.64 cfs	N = 1		
Tp = 29.4 min	L = 20 ft		
dT = 1.0 min	Cw = 3.00		
<b>Stage-Storage Results:</b>	Zcr = 313.00 ft	<b>Skimmer Orifice:</b>	
Ks = 2046		Number = 1.00	Ea
b = 1.22		Diameter = 2.00	Inches
Z <sub>0</sub> = 311.0 ft (inv)		Head = 0.98	inches
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 311.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	311.00	0.0	0.0	0.0		
1	0	0.00	311.00	0.0	0.0	0.00		
2	0	0.62	311.00	0.0	0.0	0.00		
3	0	3.10	311.00	0.0	0.0	0.00		
4	0	8.64	311.01	0.0	0.0	0.00		
5	0	18.44	311.02	0.0	0.0	0.00		
6	0	33.61	311.03	0.0	0.0	0.00		
7	0	55.24	311.05	0.0	0.0	0.00		
8	1	84.31	311.07	0.0	0.0	0.00		
9	1	121.73	311.10	0.0	0.0	0.00		
10	1	168.33	311.13	0.0	0.0	0.00		
11	1	224.81	311.16	0.0	0.0	0.00		
12	1	291.78	311.20	0.0	0.0	0.03		
13	1	367.95	311.25	0.0	0.0	0.03		
14	2	455.43	311.29	0.0	0.0	0.03		
15	2	554.47	311.34	0.0	0.0	0.03		
16	2	665.15	311.40	0.0	0.0	0.03		
17	2	787.43	311.46	0.0	0.0	0.03		
18	2	921.16	311.52	0.0	0.0	0.03		
19	3	1066.03	311.59	0.0	0.0	0.03		
20	3	1221.62	311.66	0.0	0.0	0.03		
21	3	1387.37	311.73	0.0	0.0	0.03		
22	3	1562.63	311.80	0.0	0.0	0.03		
23	3	1746.63	311.88	0.0	0.0	0.03		
24	3	1938.50	311.96	0.0	0.0	0.03		
25	3	2137.27	312.04	0.0	0.0	0.03		
26	4	2341.91	312.12	0.0	0.0	0.03		
27	4	2551.31	312.20	0.0	0.0	0.03		
28	4	2764.32	312.28	0.0	0.0	0.03		
29	4	2979.73	312.36	0.0	0.0	0.03		
30	4	3196.33	312.44	0.0	0.0	0.03		
31	4	3412.86	312.52	0.0	0.0	0.03		
32	4	3628.08	312.60	0.0	0.0	0.03		
33	4	3840.79	312.67	0.0	0.0	0.03		
34	3	4049.76	312.75	0.0	0.0	0.03		
35	3	4253.86	312.82	0.0	0.0	0.03		
36	3	4451.98	312.89	0.0	0.0	0.03		
37	3	4643.10	312.95	0.0	0.0	0.03		
38	3	4826.51	313.02	0.2	0.1	0.03		
39	3	4994.58	313.07	1.2	1.2	0.03		
40	3	5090.97	313.11	2.1	2.1	0.03		



# McADAMS

## GRAPHED DATA FOR 2 YEAR STORM





# ROUTING FOR 10-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.82	ft	
Peak Stage =	313.18	ft	
Rise =	2.18	ft	
Peak Outflow =	4.52	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	315.00 ft
Qp = 4.58 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.1 min	L = 20 ft	Number = 1 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 313.00 ft	Head = 0.98 inches	
Ks = 2046			
b = 1.22			
Z <sub>0</sub> = 311.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 311.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	311.00	0.0	0.0	0.0		
1	0	0.00	311.00	0.0	0.0	0.0		
2	0	0.44	311.00	0.0	0.0	0.0		
3	0	2.21	311.00	0.0	0.0	0.0		
4	0	6.18	311.01	0.0	0.0	0.0		
5	0	13.21	311.02	0.0	0.0	0.0		
6	0	24.14	311.03	0.0	0.0	0.0		
7	0	39.78	311.04	0.0	0.0	0.0		
8	0	60.93	311.06	0.0	0.0	0.0		
9	1	88.33	311.08	0.0	0.0	0.0		
10	1	122.69	311.10	0.0	0.0	0.0		
11	1	164.67	311.13	0.0	0.0	0.0		
12	1	214.89	311.16	0.0	0.0	0.0		
13	1	273.91	311.19	0.0	0.0	0.0	0.03	
14	1	340.47	311.23	0.0	0.0	0.0	0.03	
15	1	416.78	311.27	0.0	0.0	0.0	0.03	
16	2	503.22	311.32	0.0	0.0	0.0	0.03	
17	2	600.12	311.37	0.0	0.0	0.0	0.03	
18	2	707.72	311.42	0.0	0.0	0.0	0.03	
19	2	826.21	311.48	0.0	0.0	0.0	0.03	
20	2	955.69	311.54	0.0	0.0	0.0	0.03	
21	3	1096.20	311.60	0.0	0.0	0.0	0.03	
22	3	1247.72	311.67	0.0	0.0	0.0	0.03	
23	3	1410.14	311.74	0.0	0.0	0.0	0.03	
24	3	1583.29	311.81	0.0	0.0	0.0	0.03	
25	3	1766.92	311.89	0.0	0.0	0.0	0.03	
26	3	1960.74	311.97	0.0	0.0	0.0	0.03	
27	4	2164.36	312.05	0.0	0.0	0.0	0.03	
28	4	2377.35	312.13	0.0	0.0	0.0	0.03	
29	4	2599.21	312.22	0.0	0.0	0.0	0.03	
30	4	2829.39	312.30	0.0	0.0	0.0	0.03	
31	4	3067.27	312.39	0.0	0.0	0.0	0.03	
32	4	3312.20	312.48	0.0	0.0	0.0	0.03	
33	4	3563.47	312.57	0.0	0.0	0.0	0.03	
34	4	3820.34	312.67	0.0	0.0	0.0	0.03	
35	4	4082.03	312.76	0.0	0.0	0.0	0.03	
36	5	4347.72	312.85	0.0	0.0	0.0	0.03	
37	5	4616.59	312.94	0.0	0.0	0.0	0.03	
38	5	4887.77	313.04	0.5	0.4	0.0	0.03	
39	5	5134.80	313.12	2.5	2.5	0.0	0.03	
40	5	5257.07	313.16	3.9	3.9	0.0	0.03	
41	5	5295.56	313.17	4.4	4.4	0.0	0.03	
42	5	5304.21	313.18	4.5	4.5	0.0	0.03	
43	4	5304.20	313.18	4.5	4.5	0.0	0.03	
44	4	5301.25	313.18	4.5	4.5	0.0	0.03	
45	4	5296.75	313.18	4.4	4.4	0.0	0.03	
46	4	5291.06	313.17	4.4	4.3	0.0	0.03	

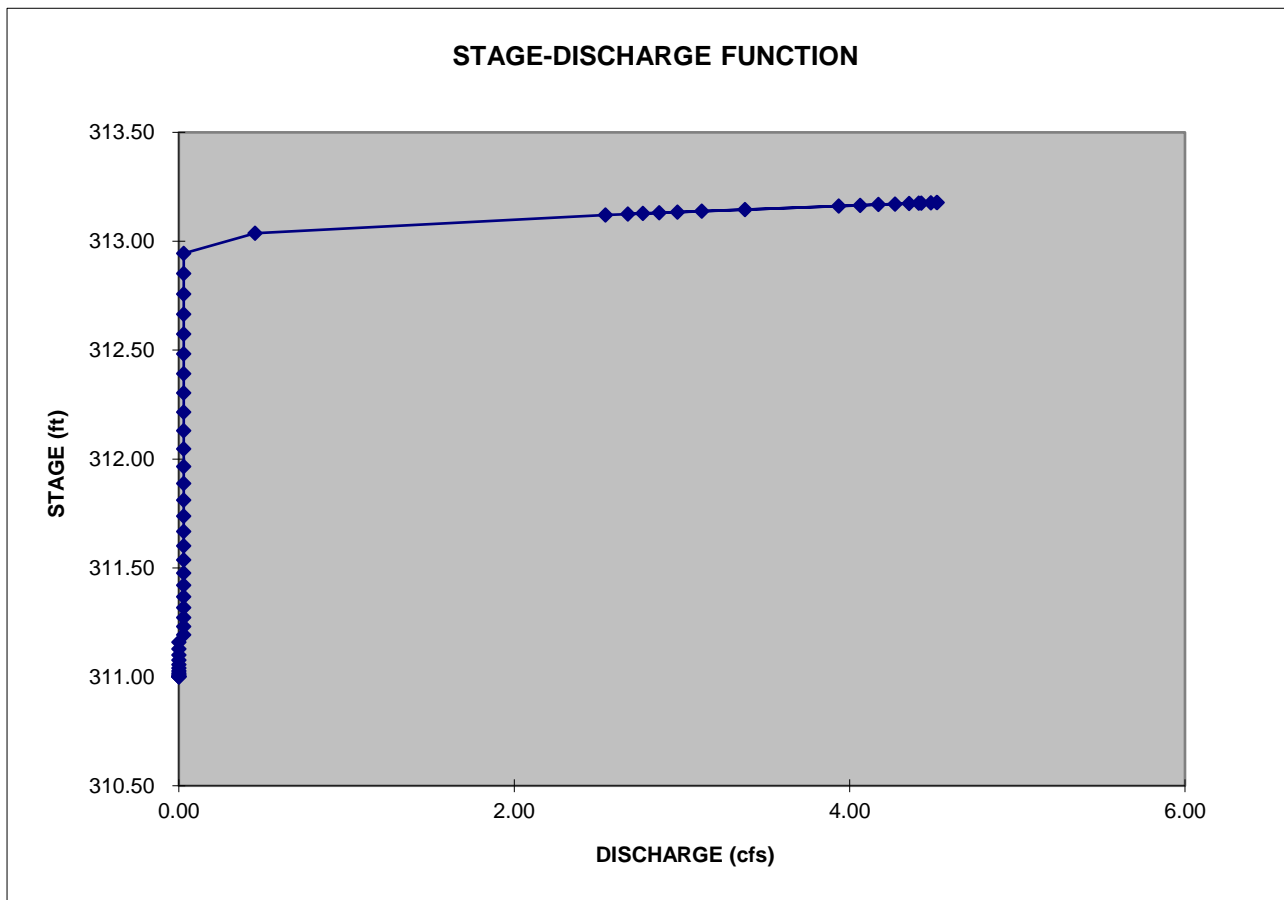
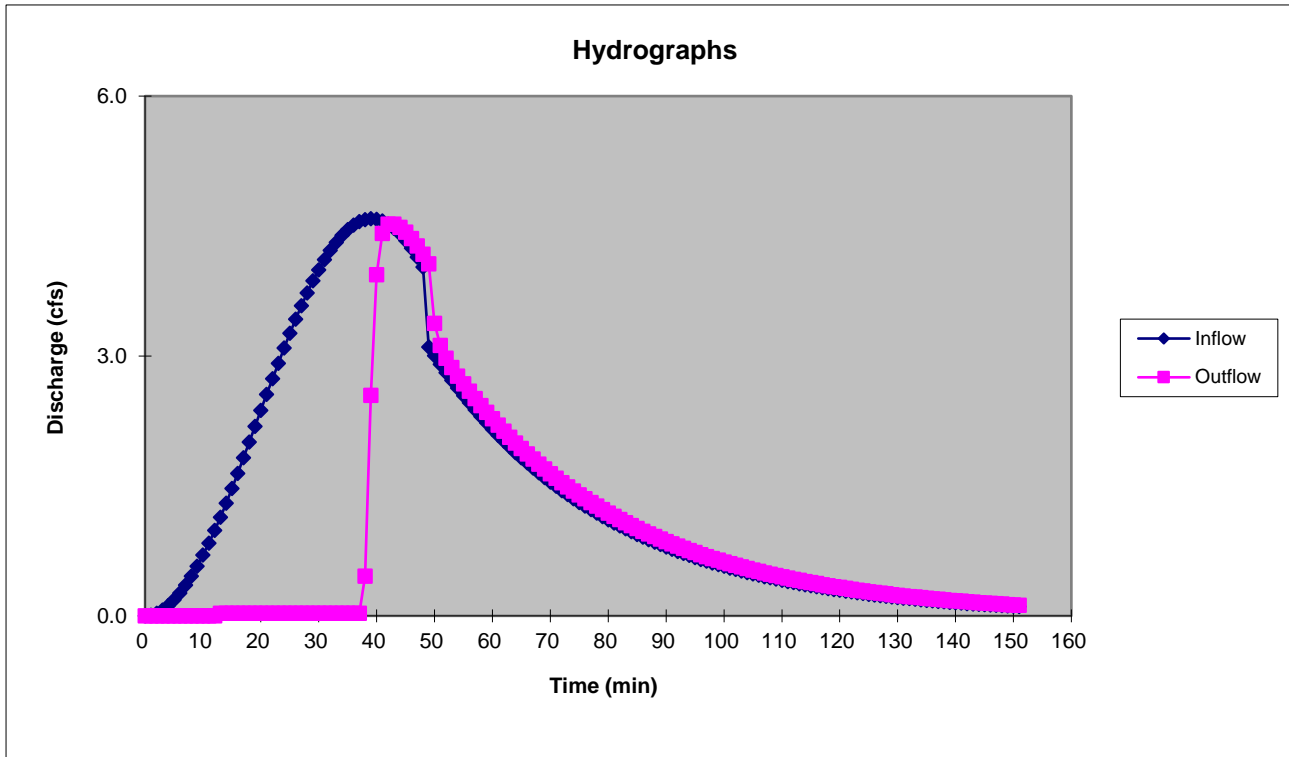


OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	4	5284.27	313.17	4.3	4.2	0.03		
48	4	5276.42	313.17	4.2	4.1	0.03		
49	3	5267.56	313.17	4.1	4.0	0.03		
50	3	5209.90	313.15	3.4	3.3	0.03		
51	3	5187.44	313.14	3.1	3.1	0.03		
52	3	5174.49	313.13	3.0	2.9	0.03		
53	3	5164.54	313.13	2.9	2.8	0.03		
54	3	5155.67	313.13	2.8	2.7	0.03		
55	3	5147.28	313.12	2.7	2.6	0.03		
56	2	5139.16	313.12	2.6	2.6	0.03		
57	2	5131.26	313.12	2.5	2.5	0.03		
58	2	5123.53	313.12	2.4	2.4	0.03		
59	2	5115.99	313.11	2.3	2.3	0.03		
60	2	5108.60	313.11	2.3	2.2	0.03		
61	2	5101.38	313.11	2.2	2.2	0.03		
62	2	5094.32	313.11	2.1	2.1	0.03		
63	2	5087.41	313.10	2.1	2.0	0.03		
64	2	5080.65	313.10	2.0	2.0	0.03		
65	2	5074.04	313.10	1.9	1.9	0.03		
66	2	5067.58	313.10	1.9	1.8	0.03		
67	2	5061.25	313.10	1.8	1.8	0.03		
68	2	5055.06	313.09	1.8	1.7	0.03		
69	2	5049.01	313.09	1.7	1.7	0.03		
70	2	5043.09	313.09	1.6	1.6	0.03		
71	1	5037.30	313.09	1.6	1.6	0.03		
72	1	5031.63	313.09	1.5	1.5	0.03		
73	1	5026.08	313.08	1.5	1.5	0.03		
74	1	5020.66	313.08	1.4	1.4	0.03		
75	1	5015.35	313.08	1.4	1.4	0.03		
76	1	5010.16	313.08	1.3	1.3	0.03		
77	1	5005.08	313.08	1.3	1.3	0.03		
78	1	5000.11	313.08	1.3	1.2	0.03		
79	1	4995.25	313.07	1.2	1.2	0.03		
80	1	4990.49	313.07	1.2	1.2	0.03		
81	1	4985.84	313.07	1.1	1.1	0.03		
82	1	4981.28	313.07	1.1	1.1	0.03		
83	1	4976.83	313.07	1.1	1.0	0.03		
84	1	4972.47	313.07	1.0	1.0	0.03		
85	1	4968.20	313.06	1.0	1.0	0.03		
86	1	4964.03	313.06	1.0	0.9	0.03		
87	1	4959.94	313.06	0.9	0.9	0.03		
88	1	4955.95	313.06	0.9	0.9	0.03		
89	1	4952.04	313.06	0.9	0.9	0.03		
90	1	4948.21	313.06	0.9	0.8	0.03		
91	1	4944.46	313.06	0.8	0.8	0.03		
92	1	4940.80	313.05	0.8	0.8	0.03		
93	1	4937.21	313.05	0.8	0.7	0.03		
94	1	4933.70	313.05	0.8	0.7	0.03		
95	1	4930.27	313.05	0.7	0.7	0.03		
96	1	4926.91	313.05	0.7	0.7	0.03		
97	1	4923.62	313.05	0.7	0.7	0.03		
98	1	4920.40	313.05	0.7	0.6	0.03		
99	1	4917.24	313.05	0.6	0.6	0.03		
100	1	4914.16	313.05	0.6	0.6	0.03		
101	1	4911.14	313.04	0.6	0.6	0.03		
102	1	4908.19	313.04	0.6	0.6	0.03		
103	1	4905.29	313.04	0.6	0.5	0.03		
104	0	4902.46	313.04	0.5	0.5	0.03		
105	0	4899.69	313.04	0.5	0.5	0.03		
106	0	4896.98	313.04	0.5	0.5	0.03		
107	0	4894.32	313.04	0.5	0.5	0.03		
108	0	4891.73	313.04	0.5	0.4	0.03		
109	0	4889.18	313.04	0.5	0.4	0.03		
110	0	4886.69	313.04	0.4	0.4	0.03		
111	0	4884.25	313.04	0.4	0.4	0.03		
112	0	4881.86	313.03	0.4	0.4	0.03		
113	0	4879.53	313.03	0.4	0.4	0.03		
114	0	4877.24	313.03	0.4	0.4	0.03		
115	0	4875.00	313.03	0.4	0.4	0.03		
116	0	4872.80	313.03	0.4	0.3	0.03		
117	0	4870.65	313.03	0.4	0.3	0.03		
118	0	4868.55	313.03	0.3	0.3	0.03		
119	0	4866.49	313.03	0.3	0.3	0.03		
120	0	4864.47	313.03	0.3	0.3	0.03		
121	0	4862.49	313.03	0.3	0.3	0.03		
122	0	4860.56	313.03	0.3	0.3	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	4858.66	313.03	0.3	0.3	0.03		
124	0	4856.81	313.03	0.3	0.3	0.03		
125	0	4854.99	313.03	0.3	0.2	0.03		
126	0	4853.21	313.03	0.3	0.2	0.03		
127	0	4851.46	313.02	0.3	0.2	0.03		
128	0	4849.75	313.02	0.3	0.2	0.03		
129	0	4848.08	313.02	0.2	0.2	0.03		
130	0	4846.44	313.02	0.2	0.2	0.03		
131	0	4844.83	313.02	0.2	0.2	0.03		
132	0	4843.26	313.02	0.2	0.2	0.03		
133	0	4841.71	313.02	0.2	0.2	0.03		
134	0	4840.20	313.02	0.2	0.2	0.03		
135	0	4838.72	313.02	0.2	0.2	0.03		
136	0	4837.27	313.02	0.2	0.2	0.03		
137	0	4835.84	313.02	0.2	0.2	0.03		
138	0	4834.45	313.02	0.2	0.2	0.03		
139	0	4833.08	313.02	0.2	0.1	0.03		
140	0	4831.74	313.02	0.2	0.1	0.03		
141	0	4830.42	313.02	0.2	0.1	0.03		
142	0	4829.14	313.02	0.2	0.1	0.03		
143	0	4827.87	313.02	0.2	0.1	0.03		
144	0	4826.63	313.02	0.2	0.1	0.03		
145	0	4825.42	313.02	0.1	0.1	0.03		
146	0	4824.23	313.02	0.1	0.1	0.03		
147	0	4823.06	313.01	0.1	0.1	0.03		
148	0	4821.91	313.01	0.1	0.1	0.03		
149	0	4820.79	313.01	0.1	0.1	0.03		
150	0	4819.68	313.01	0.1	0.1	0.03		
151	0	4818.60	313.01	0.1	0.1	0.03		

# McADAMS

## GRAPHED DATA FOR 10 YEAR STORM





# ROUTING FOR 25-YEAR STORM

(REF: Malcom, 1991)

COMPUTED RESULTS			
Freeboard =	1.80	ft	
Peak Stage =	313.20	ft	
Rise =	2.20	ft	
Peak Outflow =	5.25	cfs	

INPUT			
<b>Hydrograph Results:</b>	<b>Weir:</b>	<b>Top of Dam:</b>	315.00 ft
Qp = 5.26 cfs	N = 1	<b>Skimmer Orifice:</b>	
Tp = 39.1 min	L = 20 ft	Number = 1.00 Ea	
dT = 1.0 min	Cw = 3.00	Diameter = 2.00 Inches	
<b>Stage-Storage Results:</b>	Zcr = 313.00 ft	Head = 0.98 inches	
Ks = 2046			
b = 1.22			
Z <sub>0</sub> = 311.0 ft (inv)			
<b>Initial Water Level:</b>			
Z <sub>i</sub> = 311.00 ft			

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
0	0	0.00	311.00	0.0	0.0	0.0		
1	0	0.00	311.00	0.0	0.0	0.00		
2	0	0.51	311.00	0.0	0.0	0.00		
3	0	2.54	311.00	0.0	0.0	0.00		
4	0	7.09	311.01	0.0	0.0	0.00		
5	0	15.15	311.02	0.0	0.0	0.00		
6	0	27.69	311.03	0.0	0.0	0.00		
7	0	45.64	311.04	0.0	0.0	0.00		
8	1	69.90	311.06	0.0	0.0	0.00		
9	1	101.34	311.09	0.0	0.0	0.00		
10	1	140.75	311.11	0.0	0.0	0.00		
11	1	188.91	311.14	0.0	0.0	0.00		
12	1	246.53	311.18	0.0	0.0	0.03		
13	1	312.47	311.22	0.0	0.0	0.03		
14	1	389.09	311.26	0.0	0.0	0.03		
15	2	476.90	311.30	0.0	0.0	0.03		
16	2	576.34	311.36	0.0	0.0	0.03		
17	2	687.76	311.41	0.0	0.0	0.03		
18	2	811.47	311.47	0.0	0.0	0.03		
19	3	947.66	311.53	0.0	0.0	0.03		
20	3	1096.47	311.60	0.0	0.0	0.03		
21	3	1257.94	311.67	0.0	0.0	0.03		
22	3	1432.02	311.75	0.0	0.0	0.03		
23	3	1618.62	311.83	0.0	0.0	0.03		
24	4	1817.53	311.91	0.0	0.0	0.03		
25	4	2028.47	311.99	0.0	0.0	0.03		
26	4	2251.09	312.08	0.0	0.0	0.03		
27	4	2484.95	312.17	0.0	0.0	0.03		
28	4	2729.57	312.27	0.0	0.0	0.03		
29	4	2984.36	312.36	0.0	0.0	0.03		
30	5	3248.68	312.46	0.0	0.0	0.03		
31	5	3521.85	312.56	0.0	0.0	0.03		
32	5	3803.11	312.66	0.0	0.0	0.03		
33	5	4091.64	312.76	0.0	0.0	0.03		
34	5	4386.60	312.86	0.0	0.0	0.03		
35	5	4687.08	312.97	0.0	0.0	0.03		
36	5	4992.16	313.07	1.2	1.2	0.03		
37	5	5230.71	313.15	3.6	3.6	0.03		
38	5	5326.70	313.19	4.8	4.8	0.03		
39	5	5352.83	313.19	5.2	5.1	0.03		
40	5	5359.02	313.20	5.2	5.2	0.03		
41	5	5359.89	313.20	5.2	5.2	0.03		
42	5	5358.68	313.20	5.2	5.2	0.03		
43	5	5356.04	313.20	5.2	5.2	0.03		
44	5	5352.12	313.19	5.1	5.1	0.03		
45	5	5346.97	313.19	5.1	5.0	0.03		
46	5	5340.62	313.19	5.0	5.0	0.03		

OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
47	5	5333.09	313.19	4.9	4.9	0.03		
48	5	5324.41	313.18	4.8	4.8	0.03		
49	3	5314.62	313.18	4.7	4.6	0.03		
50	3	5221.48	313.15	3.5	3.5	0.03		
51	3	5190.93	313.14	3.2	3.1	0.03		
52	3	5175.62	313.13	3.0	3.0	0.03		
53	3	5164.92	313.13	2.9	2.8	0.03		
54	3	5155.80	313.13	2.8	2.7	0.03		
55	3	5147.32	313.12	2.7	2.6	0.03		
56	2	5139.18	313.12	2.6	2.6	0.03		
57	2	5131.26	313.12	2.5	2.5	0.03		
58	2	5123.54	313.12	2.4	2.4	0.03		
59	2	5115.99	313.11	2.3	2.3	0.03		
60	2	5108.60	313.11	2.3	2.2	0.03		
61	2	5101.38	313.11	2.2	2.2	0.03		
62	2	5094.32	313.11	2.1	2.1	0.03		
63	2	5087.41	313.10	2.1	2.0	0.03		
64	2	5080.65	313.10	2.0	2.0	0.03		
65	2	5074.04	313.10	1.9	1.9	0.03		
66	2	5067.58	313.10	1.9	1.8	0.03		
67	2	5061.25	313.10	1.8	1.8	0.03		
68	2	5055.06	313.09	1.8	1.7	0.03		
69	2	5049.01	313.09	1.7	1.7	0.03		
70	2	5043.09	313.09	1.6	1.6	0.03		
71	1	5037.30	313.09	1.6	1.6	0.03		
72	1	5031.63	313.09	1.5	1.5	0.03		
73	1	5026.08	313.08	1.5	1.5	0.03		
74	1	5020.66	313.08	1.4	1.4	0.03		
75	1	5015.35	313.08	1.4	1.4	0.03		
76	1	5010.16	313.08	1.3	1.3	0.03		
77	1	5005.08	313.08	1.3	1.3	0.03		
78	1	5000.11	313.08	1.3	1.2	0.03		
79	1	4995.25	313.07	1.2	1.2	0.03		
80	1	4990.49	313.07	1.2	1.2	0.03		
81	1	4985.84	313.07	1.1	1.1	0.03		
82	1	4981.28	313.07	1.1	1.1	0.03		
83	1	4976.83	313.07	1.1	1.0	0.03		
84	1	4972.47	313.07	1.0	1.0	0.03		
85	1	4968.20	313.06	1.0	1.0	0.03		
86	1	4964.03	313.06	1.0	0.9	0.03		
87	1	4959.94	313.06	0.9	0.9	0.03		
88	1	4955.95	313.06	0.9	0.9	0.03		
89	1	4952.04	313.06	0.9	0.9	0.03		
90	1	4948.21	313.06	0.9	0.8	0.03		
91	1	4944.46	313.06	0.8	0.8	0.03		
92	1	4940.80	313.05	0.8	0.8	0.03		
93	1	4937.21	313.05	0.8	0.7	0.03		
94	1	4933.70	313.05	0.8	0.7	0.03		
95	1	4930.27	313.05	0.7	0.7	0.03		
96	1	4926.91	313.05	0.7	0.7	0.03		
97	1	4923.62	313.05	0.7	0.7	0.03		
98	1	4920.40	313.05	0.7	0.6	0.03		
99	1	4917.24	313.05	0.6	0.6	0.03		
100	1	4914.16	313.05	0.6	0.6	0.03		
101	1	4911.14	313.04	0.6	0.6	0.03		
102	1	4908.19	313.04	0.6	0.6	0.03		
103	1	4905.29	313.04	0.6	0.5	0.03		
104	0	4902.46	313.04	0.5	0.5	0.03		
105	0	4899.69	313.04	0.5	0.5	0.03		
106	0	4896.98	313.04	0.5	0.5	0.03		
107	0	4894.32	313.04	0.5	0.5	0.03		
108	0	4891.73	313.04	0.5	0.4	0.03		
109	0	4889.18	313.04	0.5	0.4	0.03		
110	0	4886.69	313.04	0.4	0.4	0.03		
111	0	4884.25	313.04	0.4	0.4	0.03		
112	0	4881.86	313.03	0.4	0.4	0.03		
113	0	4879.53	313.03	0.4	0.4	0.03		
114	0	4877.24	313.03	0.4	0.4	0.03		
115	0	4875.00	313.03	0.4	0.4	0.03		
116	0	4872.80	313.03	0.4	0.3	0.03		
117	0	4870.65	313.03	0.4	0.3	0.03		
118	0	4868.55	313.03	0.3	0.3	0.03		
119	0	4866.49	313.03	0.3	0.3	0.03		
120	0	4864.47	313.03	0.3	0.3	0.03		
121	0	4862.49	313.03	0.3	0.3	0.03		
122	0	4860.56	313.03	0.3	0.3	0.03		

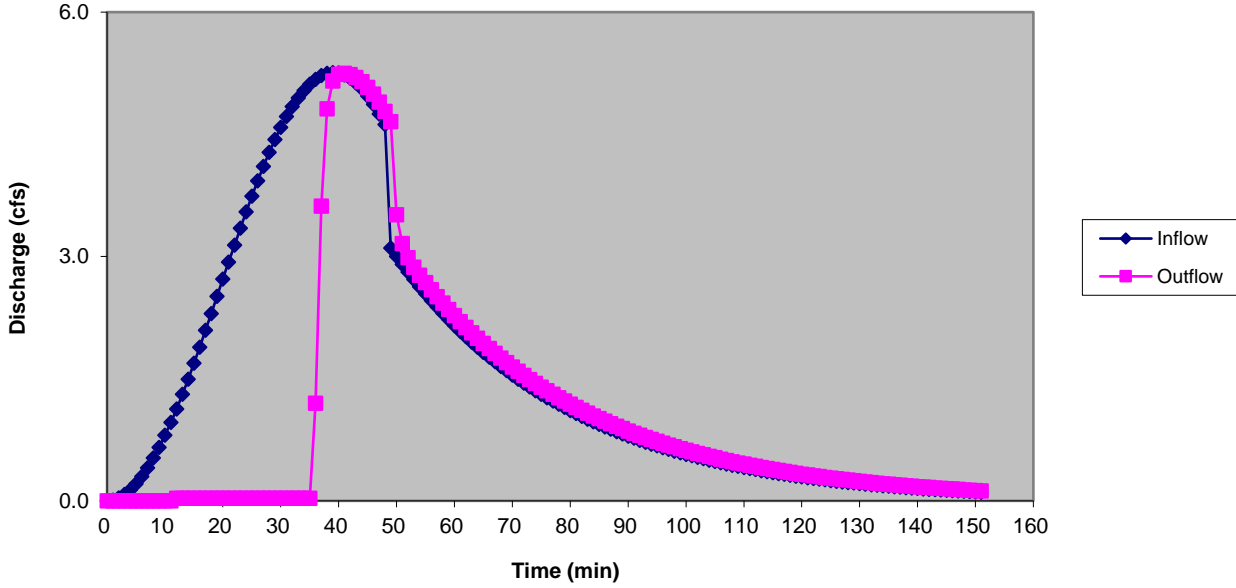
OUTPUT								
1	2	3	4	5	6	7	8	9
Time T min	Inflow I cts	Storage S cu-ft	Stage Z ft	Outflow O cts	Weir Flow cts	Skimmer Flow cts		
123	0	4858.66	313.03	0.3	0.3	0.03		
124	0	4856.81	313.03	0.3	0.3	0.03		
125	0	4854.99	313.03	0.3	0.2	0.03		
126	0	4853.21	313.03	0.3	0.2	0.03		
127	0	4851.46	313.02	0.3	0.2	0.03		
128	0	4849.75	313.02	0.3	0.2	0.03		
129	0	4848.08	313.02	0.2	0.2	0.03		
130	0	4846.44	313.02	0.2	0.2	0.03		
131	0	4844.83	313.02	0.2	0.2	0.03		
132	0	4843.26	313.02	0.2	0.2	0.03		
133	0	4841.71	313.02	0.2	0.2	0.03		
134	0	4840.20	313.02	0.2	0.2	0.03		
135	0	4838.72	313.02	0.2	0.2	0.03		
136	0	4837.27	313.02	0.2	0.2	0.03		
137	0	4835.84	313.02	0.2	0.2	0.03		
138	0	4834.45	313.02	0.2	0.2	0.03		
139	0	4833.08	313.02	0.2	0.1	0.03		
140	0	4831.74	313.02	0.2	0.1	0.03		
141	0	4830.42	313.02	0.2	0.1	0.03		
142	0	4829.14	313.02	0.2	0.1	0.03		
143	0	4827.87	313.02	0.2	0.1	0.03		
144	0	4826.63	313.02	0.2	0.1	0.03		
145	0	4825.42	313.02	0.1	0.1	0.03		
146	0	4824.23	313.02	0.1	0.1	0.03		
147	0	4823.06	313.01	0.1	0.1	0.03		
148	0	4821.91	313.01	0.1	0.1	0.03		
149	0	4820.79	313.01	0.1	0.1	0.03		
150	0	4819.68	313.01	0.1	0.1	0.03		
151	0	4818.60	313.01	0.1	0.1	0.03		



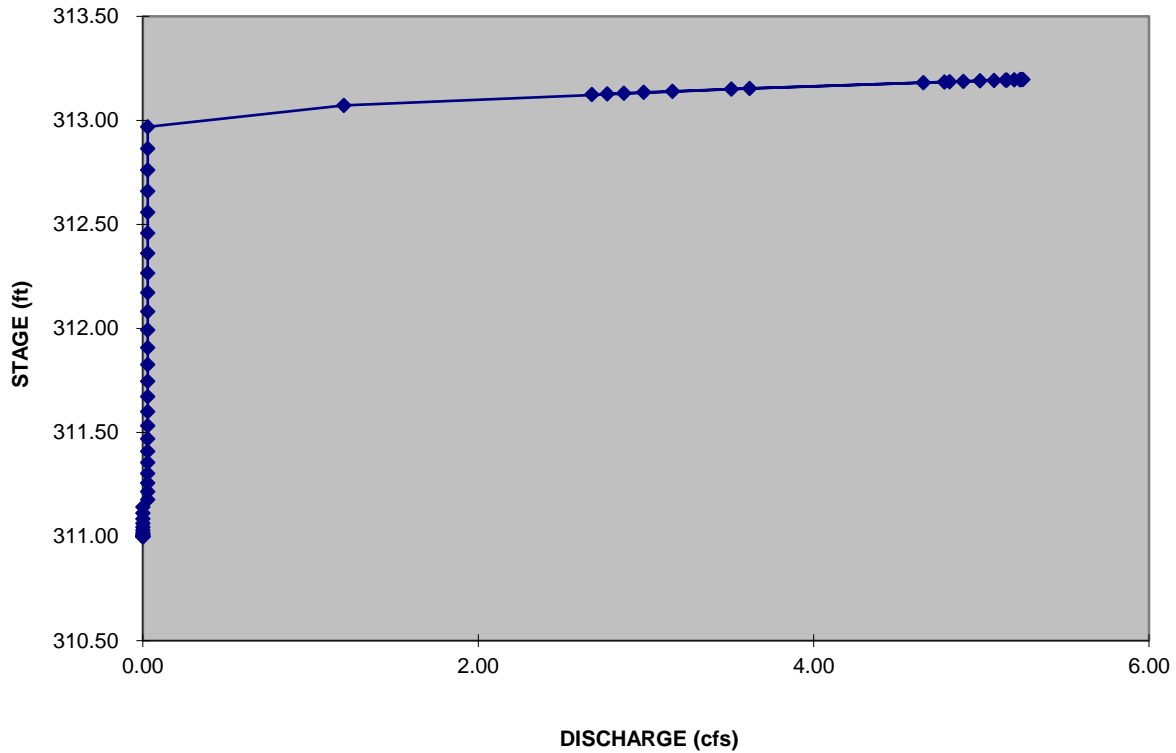
McADAMS

# GRAPHED DATA FOR 25 YEAR STORM

### Hydrographs



### STAGE-DISCHARGE FUNCTION



---

## **Drainage Area Maps**

---





**MCADAMS**

The John R. McAdams Company, Inc.  
621 Hillsborough Street  
Suite 500  
Raleigh, NC 27603  
phone 919. 361. 5000  
fax 919. 361. 2269  
license number: C-0293, C-187

www.mcadamsco.com

**CLIENT**

D.R. HORTON, INC.  
7208 FALLS OF NEUSE ROAD, SUITE 201  
RALEIGH, NC 27615  
PHONE: 919. 809. 4207



**CHAMBLEE LAKE  
CONSTRUCTION DRAWINGS  
1509 CHAMBLEE ROAD  
ZEBULON, NORTH CAROLINA**



M:\Projects\DRH\DRH22004-Production\Engineering\Design Files\Erosion Control\EC Drainage Areas.dwg, 2/26/2024, 5:56:13 PM, Benjamin Kotner

**REVISIONS**

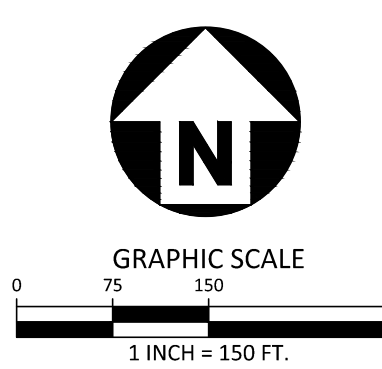
NO.	DATE

**PLAN INFORMATION**

PROJECT NO.	DRH22004
FILENAME	DRH22004-OAS1
CHECKED BY	RKB
DRAWN BY	
SCALE	1"=150'
DATE	02.27.2024

**SHEET**

**EROSION CONTROL  
DRAINAGE AREAS**



Town Certification. This design has been reviewed by the Engineer for the Town of Zebulon, and to the best of my knowledge and belief, it conforms to the requirements established in the Standard Specifications of the Town of Zebulon.  
By: \_\_\_\_\_ Date: \_\_\_\_\_  
Town Engineer  
These plans are approved by the Town of Zebulon and serve as construction plans for this project.  
By: \_\_\_\_\_ Date: \_\_\_\_\_  
Administrator

FINAL DRAWING - NOT RELEASED FOR CONSTRUCTION



---

## Slope Drain and Diversion Ditch Calculations

---

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-1**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.34	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	1.46	cfs	
Q <sub>10</sub> =	1.86	cfs	
Q <sub>25</sub> =	2.05		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	3.50%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.090	0.66	1.31	4.17	0.31	1.42	0.45	1.86	1.86	1.44	Tall Fescue	0.090
0.025	1.42	6.05	8.98	0.67	8.54	n/a	1.86	51.69	3.10	ERONET P300	0.025

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.355	0.38	2.25	0.17	3.85	n/a	1.46	1.46	0.78	Bare Earth	-
0.055	0.5	0.75	3.16	0.24	1.94	n/a	1.46	1.45	1.09	ERONET DS75	0.055

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-2**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.25	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	5.36	cfs	
Q <sub>10</sub> =	6.84	cfs	
Q <sub>25</sub> =	7.55		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	4.50%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.059	0.875	2.30	5.53	0.42	2.97	1.23	6.84	6.83	2.46	Tall Fescue	0.059
0.025	0.875	2.30	5.53	0.42	7.02	n/a	6.84	16.11	2.46	ERONET P300	0.031

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.552	0.91	3.49	0.26	5.86	n/a	5.36	5.36	1.55	Bare Earth	-
0.022	0.552	0.91	3.49	0.26	5.86	n/a	5.36	5.36	1.55	ERONET C125	0.022

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 36 Months

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET C125

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Coconut Fiber - 0.5lb/sy	Heavyweight photodegradable w/ UV additives - 3lb/1000sf	10.0	2.25

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-3**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.77	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	3.30	cfs	
Q <sub>10</sub> =	4.21	cfs	
Q <sub>25</sub> =	4.65		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.02%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.073	1.045	3.28	6.61	0.50	1.29	0.64	4.21	4.22	0.67	Tall Fescue	0.073
0.025	0.875	2.30	5.53	0.42	3.34	n/a	4.21	7.67	0.56	ERONET P300	0.031

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.608	1.11	3.85	0.29	2.98	n/a	3.30	3.30	0.39	Bare Earth	-
0.048	0.813	1.98	5.14	0.39	1.66	n/a	3.30	3.28	0.52	ERONET DS75	0.048

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-4**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.55	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	2.36	cfs	
Q <sub>10</sub> =	3.01	cfs	
Q <sub>25</sub> =	3.32		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.02%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.090	0.997	2.98	6.31	0.47	1.01	0.48	3.01	3.02	0.63	Tall Fescue	0.090
0.025	0.875	2.30	5.53	0.42	3.34	n/a	3.01	7.67	0.56	ERONET P300	0.031

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.535	0.86	3.38	0.25	2.73	n/a	2.36	2.35	0.34	Bare Earth	-
0.050	0.73	1.60	4.62	0.35	1.48	n/a	2.36	2.37	0.46	ERONET DS75	0.050

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/1000bsf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-5**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.82	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	3.51	cfs	
Q <sub>10</sub> =	4.49	cfs	
Q <sub>25</sub> =	4.96		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.17%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.073	1.043	3.26	6.60	0.49	1.38	0.68	4.49	4.49	0.76	Tall Fescue	0.073
0.025	0.875	2.30	5.53	0.42	3.58	n/a	4.49	8.22	0.64	ERONET P300	0.031

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.607	1.11	3.84	0.29	3.18	n/a	3.51	3.52	0.44	Bare Earth	-
0.048	0.812	1.98	5.14	0.39	1.77	n/a	3.51	3.51	0.59	ERONET DS75	0.048

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-6**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	7.47	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	9.80	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	4.57	in/hr	
I <sub>10</sub> =	5.92	in/hr	
I <sub>25</sub> =	6.56		
Q <sub>2</sub> =	25.95	cfs	
Q <sub>10</sub> =	33.61	cfs	
Q <sub>25</sub> =	37.22		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.72%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.040	1.647	8.14	10.42	0.78	4.13	3.23	33.61	33.63	1.77	Tall Fescue	0.040
0.025	0.875	2.30	5.53	0.42	4.34	n/a	33.61	9.96	0.94	ERONET P300	0.031

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	1.195	4.28	7.56	0.57	6.07	n/a	25.95	25.99	1.28	Bare Earth	-
0.034	1.406	5.93	8.89	0.67	4.37	n/a	25.95	25.95	1.51	ERONET DS75	0.034

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle



# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-7**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.35	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	1.50	cfs	
Q <sub>10</sub> =	1.92	cfs	
Q <sub>25</sub> =	2.12		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	3.35%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.090	0.67	1.35	4.24	0.32	1.41	0.45	1.92	1.90	1.40	Tall Fescue	0.090
0.025	0.875	2.30	5.53	0.42	6.05	n/a	1.92	13.90	1.83	ERONET P300	0.031

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.362	0.39	2.29	0.17	3.82	n/a	1.50	1.50	0.76	Bare Earth	-
0.055	0.51	0.78	3.23	0.24	1.92	n/a	1.50	1.50	1.07	ERONET DS75	0.055

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-8**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	4.14	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	7.70	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	4.98	in/hr	
I <sub>10</sub> =	6.41	in/hr	
I <sub>25</sub> =	7.09		
Q <sub>2</sub> =	15.67	cfs	
Q <sub>10</sub> =	20.17	cfs	
Q <sub>25</sub> =	22.31		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.17%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.046	1.54	7.11	9.74	0.73	2.83	2.07	20.17	20.16	1.12	Tall Fescue	0.046
0.025	1.1	3.63	6.96	0.52	4.17	n/a	20.17	15.13	0.80	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	1.063	3.39	6.72	0.50	4.63	n/a	15.67	15.69	0.78	Bare Earth	-
0.037	1.292	5.01	8.17	0.61	3.13	n/a	15.67	15.69	0.94	ERONET DS75	0.037

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/1000bsf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-9**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	3.28	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.80	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.43	in/hr	
I <sub>10</sub> =	6.94	in/hr	
I <sub>25</sub> =	7.67		
Q <sub>2</sub> =	13.52	cfs	
Q <sub>10</sub> =	17.31	cfs	
Q <sub>25</sub> =	19.12		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.19%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.050	1.496	6.71	9.46	0.71	2.58	1.83	17.31	17.32	1.11	Tall Fescue	0.050
0.025	1.1	3.63	6.96	0.52	4.20	n/a	17.31	15.25	0.82	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	1.003	3.02	6.34	0.48	4.49	n/a	13.52	13.55	0.74	Bare Earth	-
0.038	1.23	4.54	7.78	0.58	2.98	n/a	13.52	13.52	0.91	ERONET DS75	0.038

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-10**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.86	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.80	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.43	in/hr	
I <sub>10</sub> =	6.94	in/hr	
I <sub>25</sub> =	7.67		
Q <sub>2</sub> =	7.67	cfs	
Q <sub>10</sub> =	9.82	cfs	
Q <sub>25</sub> =	10.85		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.11%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.059	1.304	5.10	8.25	0.62	1.93	1.19	9.82	9.83	0.90	Tall Fescue	0.059
0.025	1.1	3.63	6.96	0.52	4.06	n/a	9.82	14.73	0.76	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.821	2.02	5.19	0.39	3.79	n/a	7.67	7.67	0.57	Bare Earth	-
0.043	1.055	3.34	6.67	0.50	2.29	n/a	7.67	7.66	0.73	ERONET DS75	0.042

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-11**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.83	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	3.54	cfs	
Q <sub>10</sub> =	4.52	cfs	
Q <sub>25</sub> =	4.99		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.67%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.068	0.952	2.72	6.02	0.45	1.66	0.75	4.52	4.52	0.99	Tall Fescue	0.068
0.025	1.1	3.63	6.96	0.52	4.98	n/a	4.52	18.07	1.15	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.57	0.97	3.60	0.27	3.65	n/a	3.54	3.56	0.59	Bare Earth	-
0.049	0.77	1.78	4.87	0.37	2.00	n/a	3.54	3.56	0.80	ERONET DS75	0.049

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/1000bsf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-12**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.25	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	5.38	cfs	
Q <sub>10</sub> =	6.86	cfs	
Q <sub>25</sub> =	7.58		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.79%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.059	1.042	3.26	6.59	0.49	2.11	1.04	6.86	6.86	1.16	Tall Fescue	0.059
0.025	1.1	3.63	6.96	0.52	5.15	n/a	6.86	18.71	1.23	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.657	1.29	4.16	0.31	4.15	n/a	5.38	5.38	0.73	Bare Earth	-
0.047	0.874	2.29	5.53	0.41	2.35	n/a	5.38	5.39	0.98	ERONET DS75	0.047

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-13**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.17	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	5.01	cfs	
Q <sub>10</sub> =	6.40	cfs	
Q <sub>25</sub> =	7.07		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.49%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.061	1.064	3.40	6.73	0.50	1.88	0.95	6.40	6.40	0.99	Tall Fescue	0.061
0.025	1.1	3.63	6.96	0.52	4.70	n/a	6.40	17.07	1.02	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.662	1.31	4.19	0.31	3.81	n/a	5.01	5.01	0.62	Bare Earth	-
0.046	0.876	2.30	5.54	0.42	2.20	n/a	5.01	5.05	0.81	ERONET DS75	0.046

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-14**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.96	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	4.11	cfs	
Q <sub>10</sub> =	5.24	cfs	
Q <sub>25</sub> =	5.79		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.00%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.068	1.108	3.68	7.01	0.53	1.42	0.75	5.24	5.24	0.69	Tall Fescue	0.068
0.025	1.1	3.63	6.96	0.52	3.85	n/a	5.24	13.98	0.69	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.662	1.31	4.19	0.31	3.12	n/a	4.11	4.10	0.41	Bare Earth	-
0.046	0.876	2.30	5.54	0.42	1.80	n/a	4.11	4.14	0.55	ERONET DS75	0.046

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle



# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-15**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	3.50	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.80	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.43	in/hr	
I <sub>10</sub> =	6.94	in/hr	
I <sub>25</sub> =	7.67		
Q <sub>2</sub> =	14.41	cfs	
Q <sub>10</sub> =	18.45	cfs	
Q <sub>25</sub> =	20.38		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.03%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.050	1.574	7.43	9.95	0.75	2.48	1.85	18.45	18.45	1.01	Tall Fescue	0.050
0.025	1.1	3.63	6.96	0.52	3.91	n/a	18.45	14.19	0.71	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	1.055	3.34	6.67	0.50	4.32	n/a	14.41	14.42	0.68	Bare Earth	-
0.037	1.282	4.93	8.11	0.61	2.93	n/a	14.41	14.42	0.82	ERONET DS75	0.037

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-16**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	2.43	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	10.41	cfs	
Q <sub>10</sub> =	13.30	cfs	
Q <sub>25</sub> =	14.69		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	3.26%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.050	1.122	3.78	7.10	0.53	3.52	1.88	13.30	13.31	2.28	Tall Fescue	0.050
0.025	1.1	3.63	6.96	0.52	6.96	n/a	13.30	25.25	2.24	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.752	1.70	4.76	0.36	6.13	n/a	10.41	10.40	1.53	Bare Earth	-
0.040	0.946	2.68	5.98	0.45	3.93	n/a	10.41	10.55	1.92	ERONET SC150	0.040

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 24 Months

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET SC150

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
70% Straw Fiber - 0.35lb/sy 30% Coconut Fiber - 0.15lb/sy	T: Heavyweight photodegradable w/ UV additives - 3lb/1000sf B: Lightweight	8.0	2.00

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-17**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.18	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	5.06	cfs	
Q <sub>10</sub> =	6.46	cfs	
Q <sub>25</sub> =	7.13		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	3.10%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.059	0.919	2.53	5.81	0.44	2.55	1.11	6.46	6.46	1.78	Tall Fescue	0.059
0.025	1.1	3.63	6.96	0.52	6.78	n/a	6.46	24.62	2.13	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.58	1.01	3.67	0.28	5.03	n/a	5.06	5.07	1.12	Bare Earth	-
0.049	0.782	1.83	4.95	0.37	2.76	n/a	5.06	5.06	1.51	ERONET DS75	0.049

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-18**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	4.39	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	6.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.37	in/hr	
I <sub>10</sub> =	6.88	in/hr	
I <sub>25</sub> =	7.61		
Q <sub>2</sub> =	17.93	cfs	
Q <sub>10</sub> =	22.97	cfs	
Q <sub>25</sub> =	25.38		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	3.59%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.043	1.278	4.90	8.08	0.61	4.69	2.84	22.97	22.98	2.86	Tall Fescue	0.043
0.028	1.09	3.56	6.89	0.52	6.48	n/a	22.97	23.09	2.44	ERONET P300	0.028

-----> CHANNEL UNSTABLE - PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.906	2.46	5.73	0.43	7.29	n/a	17.93	17.94	2.03	Bare Earth	-
0.020	0.874	2.29	5.53	0.41	7.82	n/a	17.93	17.93	1.96	ERONET C125	0.020

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 36 Months

#### Permanent Liner Specifications - ERONET P300

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% UV Stable Polypropylene Fiber - 0.7lb/sy	T: UV-Stabilized Polypropylene - 5lb/1000sf B: UV-Stabilized Polypropylene - 3lb/1000sf	9.0	8.0

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET C125

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Coconut Fiber - 0.5lb/sy	Heavyweight photodegradable w/ UV additives - 3lb/1000sf	10.0	2.25

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-19**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.71	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	3.03	cfs	
Q <sub>10</sub> =	3.87	cfs	
Q <sub>25</sub> =	4.28		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.53%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.073	0.938	2.64	5.93	0.44	1.47	0.65	3.87	3.87	0.90	Tall Fescue	0.073
0.028	1.09	3.56	6.89	0.52	4.23	n/a	3.87	15.07	1.04	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.546	0.89	3.45	0.26	3.39	n/a	3.03	3.03	0.52	Bare Earth	-
0.050	0.742	1.65	4.69	0.35	1.83	n/a	3.03	3.03	0.71	ERONET DS75	0.050

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/1000bsf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-20**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	2.14	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	7.70	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	4.98	in/hr	
I <sub>10</sub> =	6.41	in/hr	
I <sub>25</sub> =	7.09		
Q <sub>2</sub> =	8.10	cfs	
Q <sub>10</sub> =	10.43	cfs	
Q <sub>25</sub> =	11.53		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.02%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.059	1.355	5.51	8.57	0.64	1.89	1.22	10.43	10.43	0.86	Tall Fescue	0.059
0.028	1.09	3.56	6.89	0.52	3.45	n/a	10.43	12.31	0.69	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.852	2.18	5.39	0.40	3.73	n/a	8.10	8.12	0.54	Bare Earth	-
0.042	1.085	3.53	6.86	0.51	2.29	n/a	8.10	8.10	0.69	ERONET DS75	0.042

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-21**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.73	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.70	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.45	in/hr	
I <sub>10</sub> =	6.98	in/hr	
I <sub>25</sub> =	7.71		
Q <sub>2</sub> =	3.02	cfs	
Q <sub>10</sub> =	3.87	cfs	
Q <sub>25</sub> =	4.27		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.26%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.073	0.974	2.85	6.16	0.46	1.37	0.63	3.87	3.89	0.77	Tall Fescue	0.073
0.028	1.09	3.56	6.89	0.52	3.84		3.87	13.68	0.86	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.565	0.96	3.57	0.27	3.15	n/a	3.02	3.02	0.44	Bare Earth	-
0.049	0.764	1.75	4.83	0.36	1.73	n/a	3.02	3.03	0.60	ERONET DS75	0.049

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/1000bsf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-22**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	0.92	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.70	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.45	in/hr	
I <sub>10</sub> =	6.98	in/hr	
I <sub>25</sub> =	7.71		
Q <sub>2</sub> =	3.82	cfs	
Q <sub>10</sub> =	4.89	cfs	
Q <sub>25</sub> =	5.40		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.19%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.068	1.045	3.28	6.61	0.50	1.49	0.74	4.89	4.89	0.78	Tall Fescue	0.068
0.028	1.09	3.56	6.89	0.52	3.73	n/a	4.89	13.29	0.81	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.624	1.17	3.95	0.30	3.27	n/a	3.82	3.82	0.46	Bare Earth	-
0.047	0.831	2.07	5.26	0.39	1.85	n/a	3.82	3.84	0.62	ERONET DS75	0.047

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle



# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-23**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.57	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.70	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.45	in/hr	
I <sub>10</sub> =	6.98	in/hr	
I <sub>25</sub> =	7.71		
Q <sub>2</sub> =	6.51	cfs	
Q <sub>10</sub> =	8.33	cfs	
Q <sub>25</sub> =	9.20		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.15%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.059	1.217	4.44	7.70	0.58	1.87	1.08	8.33	8.32	0.87	Tall Fescue	0.059
0.028	1.09	3.56	6.89	0.52	3.67	n/a	8.33	13.07	0.78	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.767	1.76	4.85	0.36	3.69	n/a	6.51	6.51	0.55	Bare Earth	-
0.044	0.995	2.97	6.29	0.47	2.19	n/a	6.51	6.52	0.71	ERONET DS75	0.044

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-24**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	4.13	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	9.40	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	4.64	in/hr	
I <sub>10</sub> =	6.01	in/hr	
I <sub>25</sub> =	6.65		
Q <sub>2</sub> =	14.57	cfs	
Q <sub>10</sub> =	18.85	cfs	
Q <sub>25</sub> =	20.87		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.08%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.050	1.573	7.42	9.95	0.75	2.54	1.90	18.85	18.86	1.06	Tall Fescue	0.050
0.028	1.09	3.56	6.89	0.52	3.55	n/a	18.85	12.66	0.73	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	1.05	3.31	6.64	0.50	4.41	n/a	14.57	14.58	0.71	Bare Earth	-
0.037	1.276	4.88	8.07	0.61	2.99	n/a	14.57	14.59	0.86	ERONET DS75	0.037

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-25**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	2.79	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	8.40	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	4.83	in/hr	
I <sub>10</sub> =	6.24	in/hr	
I <sub>25</sub> =	6.90		
Q <sub>2</sub> =	10.25	cfs	
Q <sub>10</sub> =	13.23	cfs	
Q <sub>25</sub> =	14.64		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.04%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.050	1.387	5.77	8.77	0.66	2.29	1.51	13.23	13.23	0.90	Tall Fescue	0.050
0.028	1.09	3.56	6.89	0.52	3.49	n/a	13.23	12.43	0.71	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.927	2.58	5.86	0.44	3.98	n/a	10.25	10.27	0.60	Bare Earth	-
0.040	1.16	4.04	7.34	0.55	2.54	n/a	10.25	10.27	0.75	ERONET DS75	0.040

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# McADAMS

## EROSION CONTROL CHANNEL DESIGN

Project Name: **CHAMBLEE LAKE**  
 Project Number: **DRH22004**  
 Channel Number: **TD-26**

Date: **2/27/2024**  
 Calculated By: **BJK**

### I. FLOW CALCULATIONS

Drainage Area =	1.14	acres	For newly graded areas, HSG 'A' soils C = 0.020 x CN - 1.0 Conservative assumption
SCS CN =	88		
Rational C =	0.76		
T <sub>c</sub> =	5.00	min	
Return Storm for Temp Liners	2	year	
Return Storm for Permanent Liners	10	year	
I <sub>2</sub> =	5.64	in/hr	
I <sub>10</sub> =	7.20	in/hr	
I <sub>25</sub> =	7.95		
Q <sub>2</sub> =	4.88	cfs	
Q <sub>10</sub> =	6.23	cfs	
Q <sub>25</sub> =	6.89		

### II. SWALE CALCULATIONS

#### Proposed Channel Section

Longitudinal Slope =	1.15%		For established grass lined channel
Left Sideslope =	3.00	H:V	
Right Sideslope =	3.00	H:V	
Maximum Vel. =	4.50	ft/s	
Bottom Width =	0.00	ft	

#### 10-Year Storm Calculation (for Permanent Liner)

Match Manning's "n" to Calculated "n" and adjust Depth until Capacity matches Flow.

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Radius [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Liner Description/ Selection	Calculated "n"
0.064	1.126	3.80	7.12	0.53	1.64	0.88	6.23	6.23	0.81	Tall Fescue	0.064
0.028	1.09	3.56	6.89	0.52	3.67	n/a	6.23	13.07	0.78	ERONET P300	0.028

-----> NO PERMANENT LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY

#### 2-Year Storm Calculation (for Temporary Liner)

Manning's "n"	Depth [ft]	Flow Area [ft <sup>2</sup> ]	Wetted Perimeter [ft]	Hydraulic Rad. [ft]	Velocity [ft/sec]	VR [ftz/sec]	Flow (Q2) [cfs]	Capacity [cfs]	Shear Stress [lb/ft <sup>2</sup> ]	Description	Calculated "n"
0.022	0.689	1.42	4.36	0.33	3.44	n/a	4.88	4.89	0.49	Bare Earth	-
0.046	0.908	2.47	5.74	0.43	1.98	n/a	4.88	4.89	0.65	ERONET DS75	0.046

-----> CHANNEL UNSTABLE - TEMPORARY LINER REQUIRED  
 -----> VELOCITY IS OKAY  
 -----> SHEAR STRESS IS OKAY  
 Liner Longevity -> 45 Days

#### Permanent Liner Specifications - NO LINER REQUIRED

Matrix	Netting	Allowable Partially Vegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
#N/A	#N/A	#N/A	#N/A

T = Top; B = Bottom; M = Middle

#### Temporary Liner Specifications - ERONET DS75

Matrix	Netting	Allowable Unvegetated Velocity (f/s)	Allowable Partially Vegetated Shear Stress [lb/ft <sup>2</sup> ]
100% Straw Fiber - 0.5lb/sy	T(only): Lightweight photodegradable with photo accelerators - 1.5lb/10000sf	5.0	1.55

T = Top; B = Bottom; M = Middle

# Channel Report

## TD-1

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 314.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 1.86

### Highlighted

Depth (ft) = 0.28

Q (cfs) = 1.860

Area (sqft) = 0.18

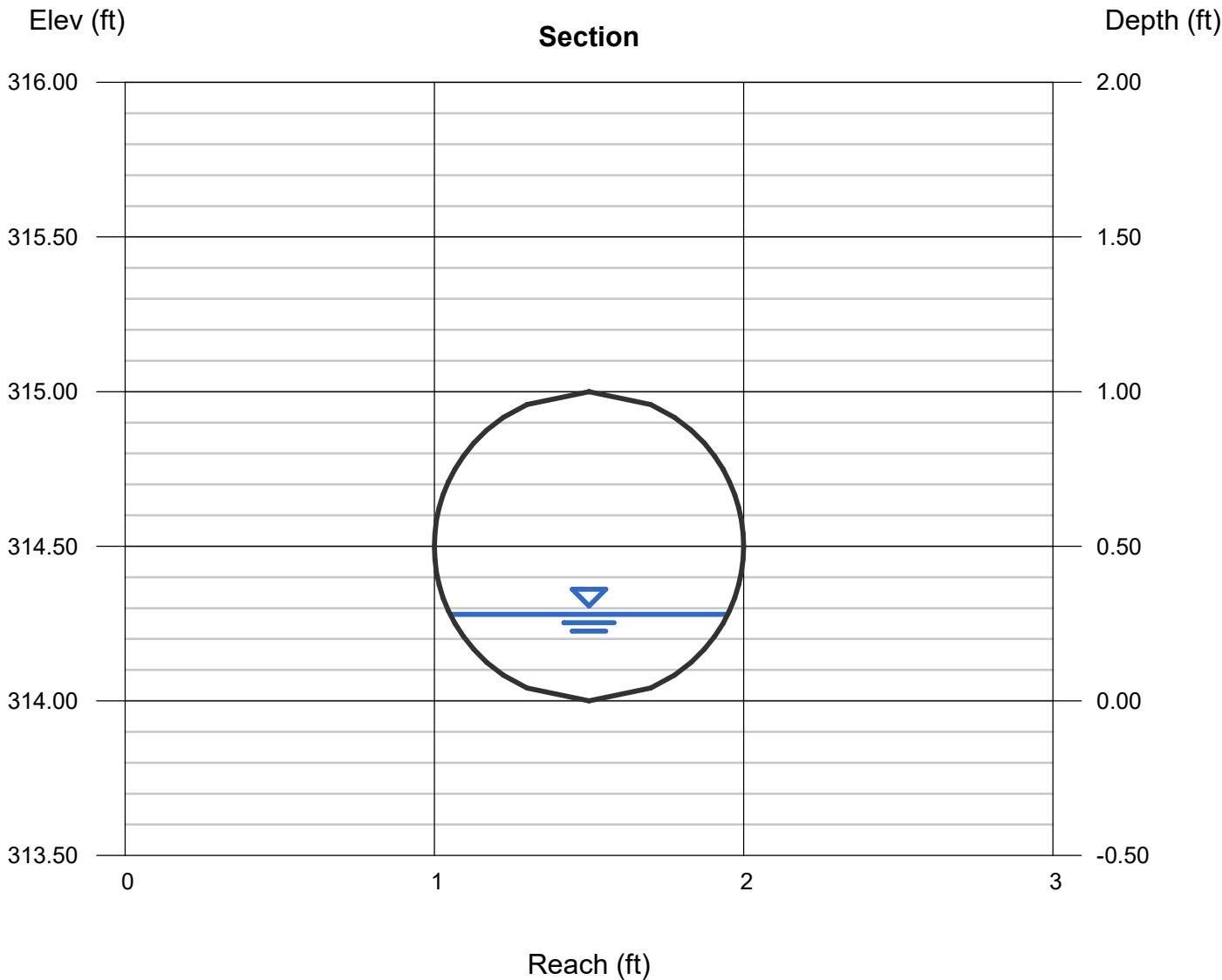
Velocity (ft/s) = 10.22

Wetted Perim (ft) = 1.12

Crit Depth, Yc (ft) = 0.58

Top Width (ft) = 0.90

EGL (ft) = 1.90



# Channel Report

## TD-2

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 314.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 6.84

### Highlighted

Depth (ft) = 0.57

Q (cfs) = 6.840

Area (sqft) = 0.46

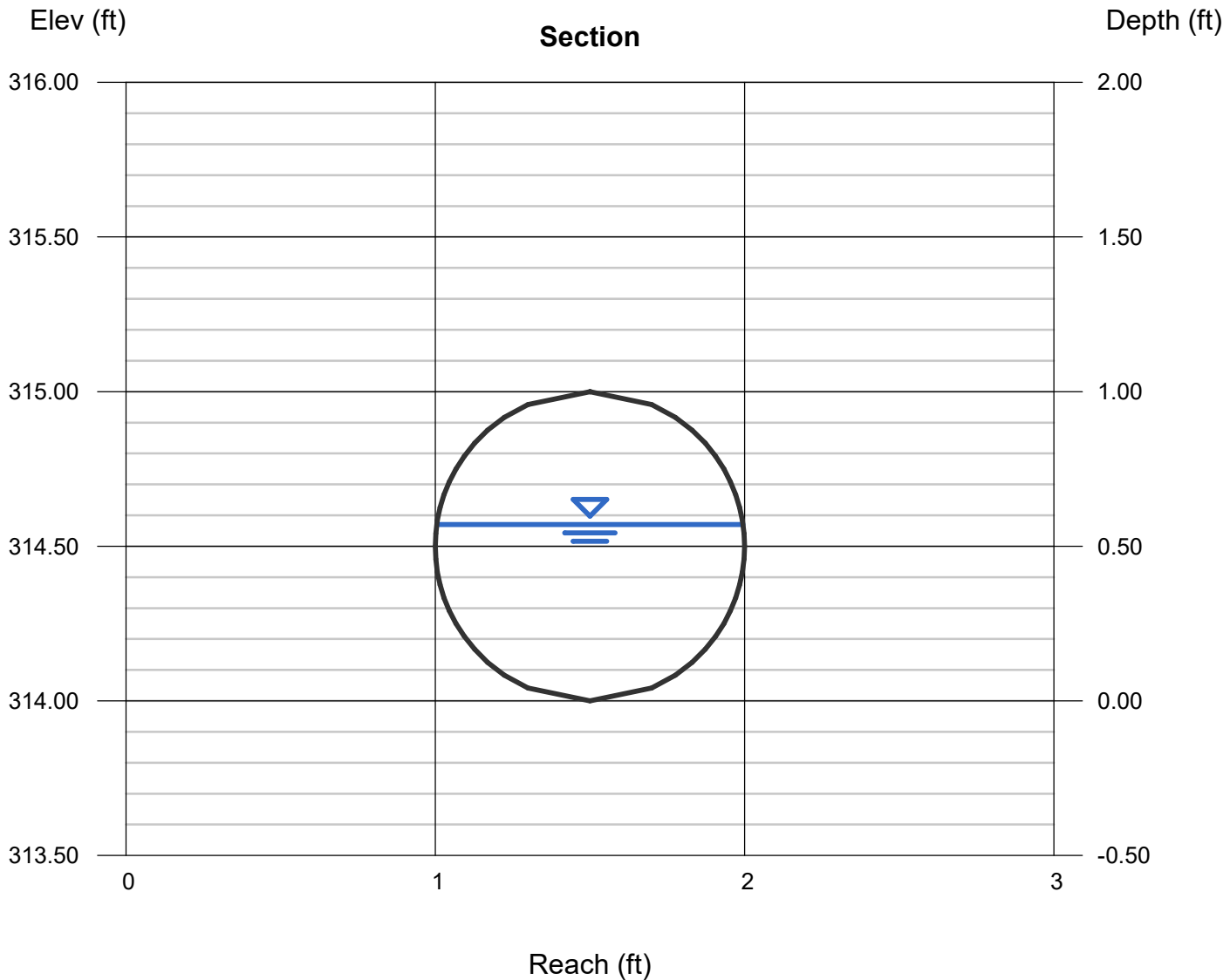
Velocity (ft/s) = 14.73

Wetted Perim (ft) = 1.71

Crit Depth, Yc (ft) = 0.98

Top Width (ft) = 0.99

EGL (ft) = 3.94



# Channel Report

## TD-3

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 3.30

### Highlighted

Depth (ft) = 0.38

Q (cfs) = 3.300

Area (sqft) = 0.27

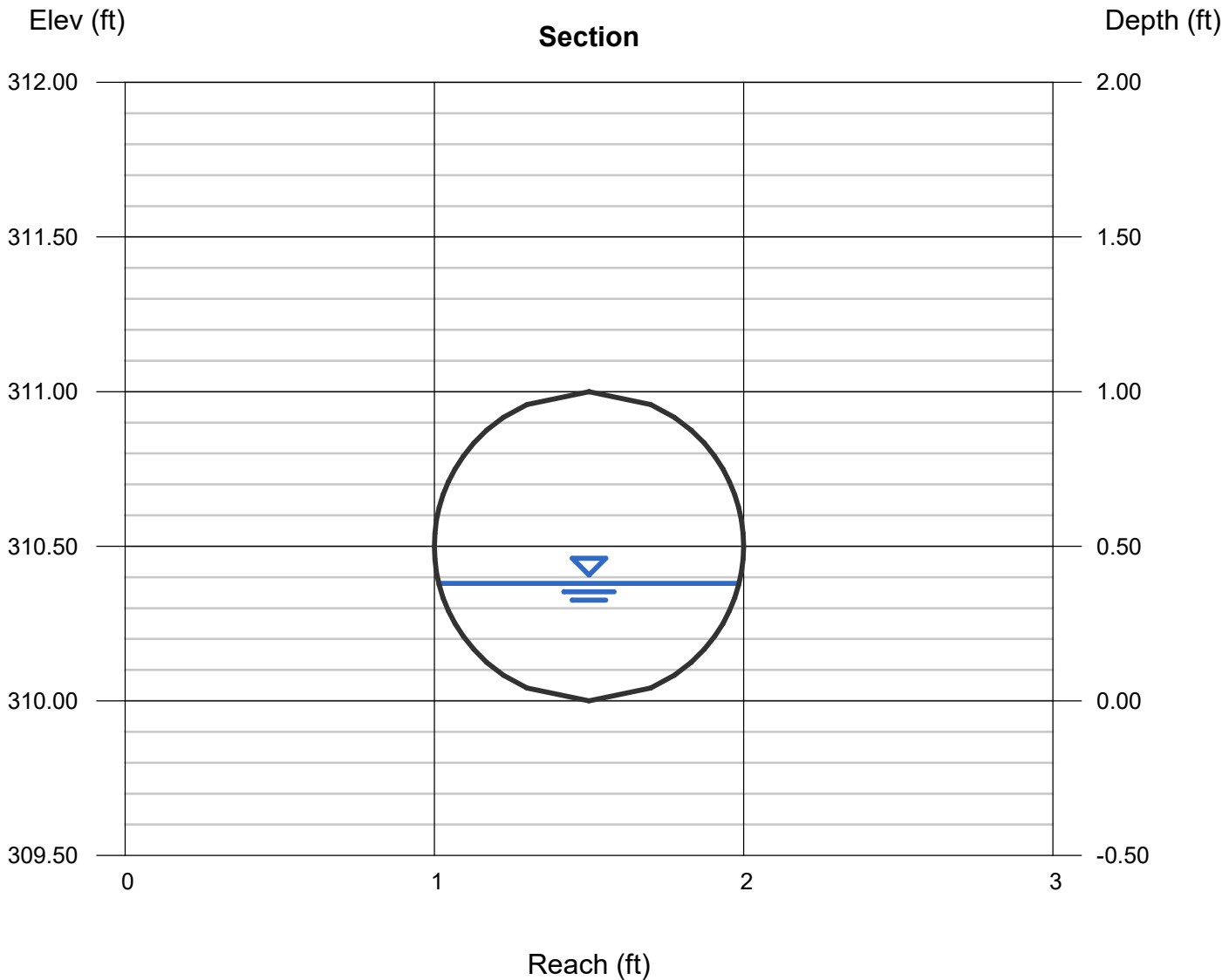
Velocity (ft/s) = 12.02

Wetted Perim (ft) = 1.33

Crit Depth,  $Y_c$  (ft) = 0.78

Top Width (ft) = 0.97

EGL (ft) = 2.63



# Channel Report

## TD-4

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 2.36

### Highlighted

Depth (ft) = 0.32

Q (cfs) = 2.360

Area (sqft) = 0.22

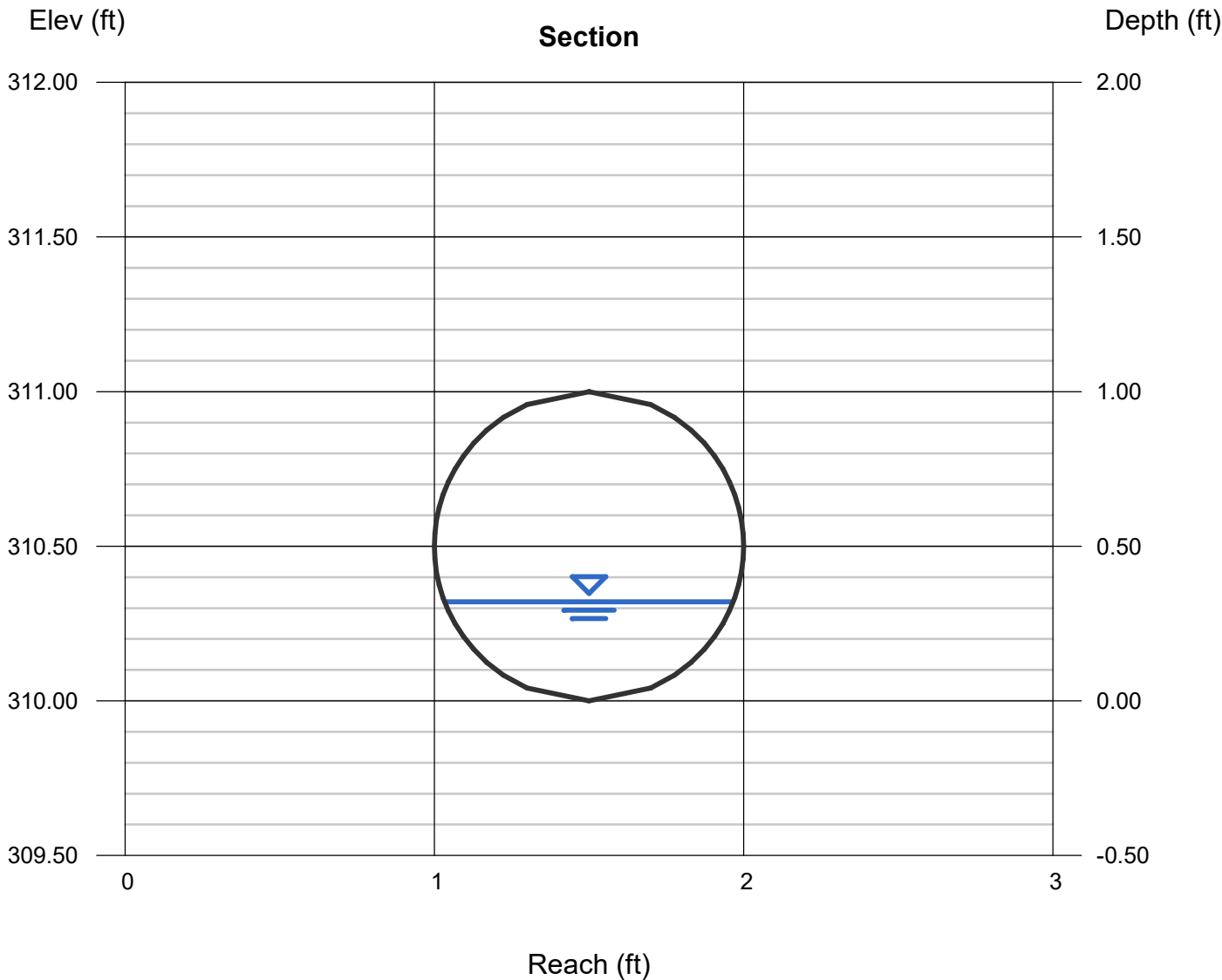
Velocity (ft/s) = 10.84

Wetted Perim (ft) = 1.20

Crit Depth,  $Y_c$  (ft) = 0.66

Top Width (ft) = 0.93

EGL (ft) = 2.15





# Channel Report

## TD-5

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 4.49

### Highlighted

Depth (ft) = 0.44

Q (cfs) = 4.490

Area (sqft) = 0.34

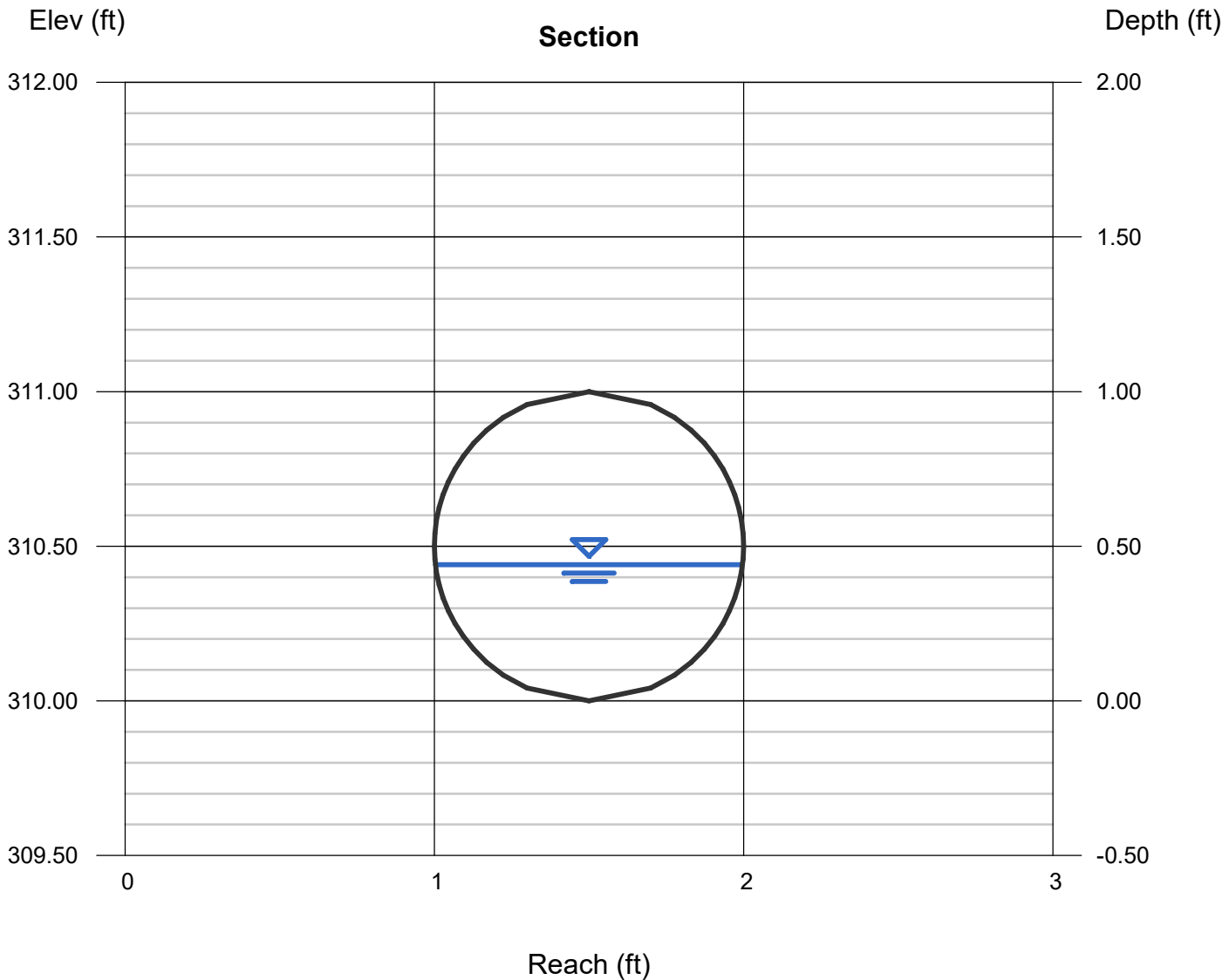
Velocity (ft/s) = 13.40

Wetted Perim (ft) = 1.45

Crit Depth, Yc (ft) = 0.89

Top Width (ft) = 0.99

EGL (ft) = 3.23



# Channel Report

## TD-6

### Circular

Diameter (ft) = 1.50

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 33.61

### Highlighted

Depth (ft) = 1.27

Q (cfs) = 33.61

Area (sqft) = 1.60

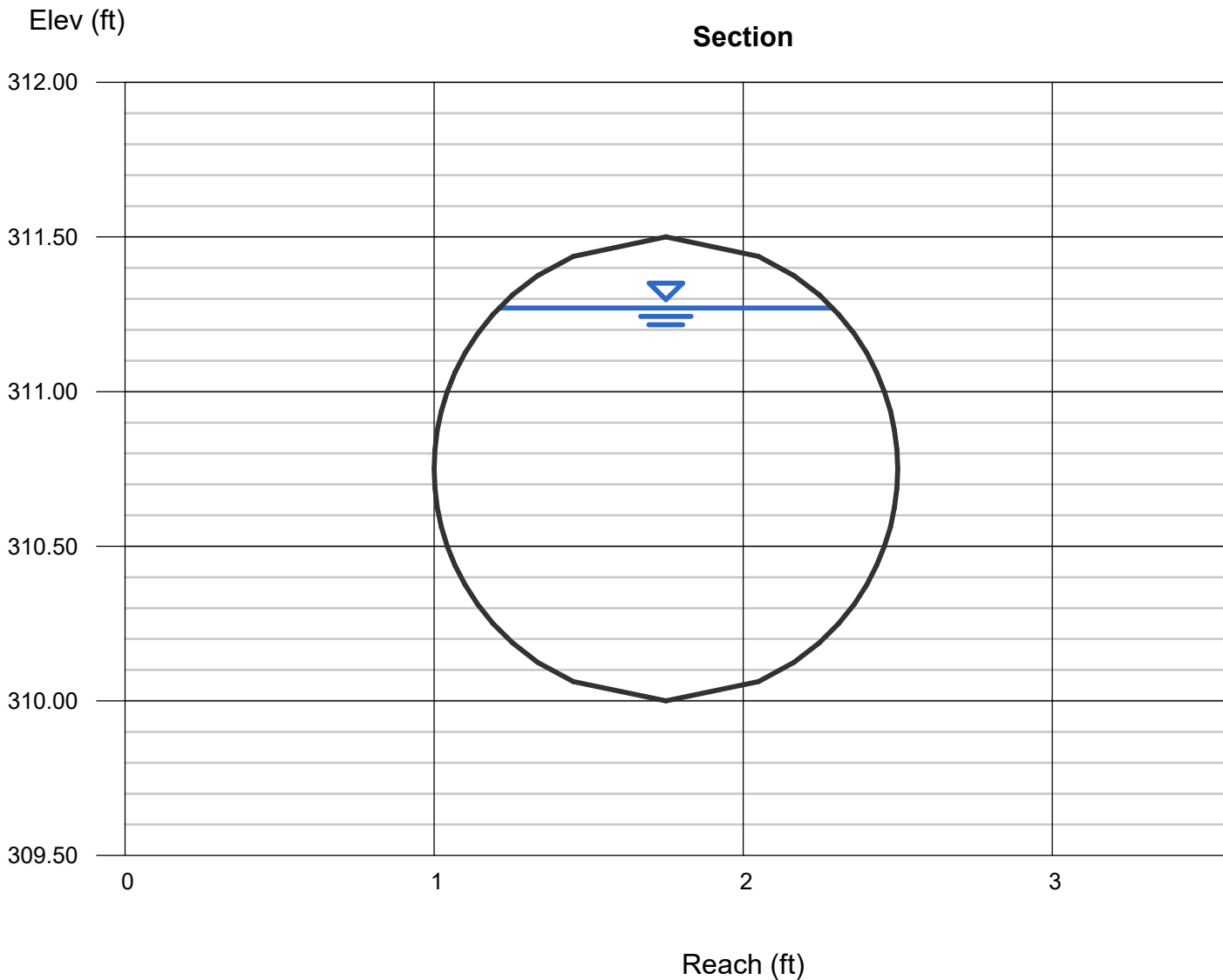
Velocity (ft/s) = 21.04

Wetted Perim (ft) = 3.51

Crit Depth,  $Y_c$  (ft) = 1.50

Top Width (ft) = 1.08

EGL (ft) = 8.15



# Channel Report

## TD-7

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 1.92

### Highlighted

Depth (ft) = 0.28

Q (cfs) = 1.920

Area (sqft) = 0.18

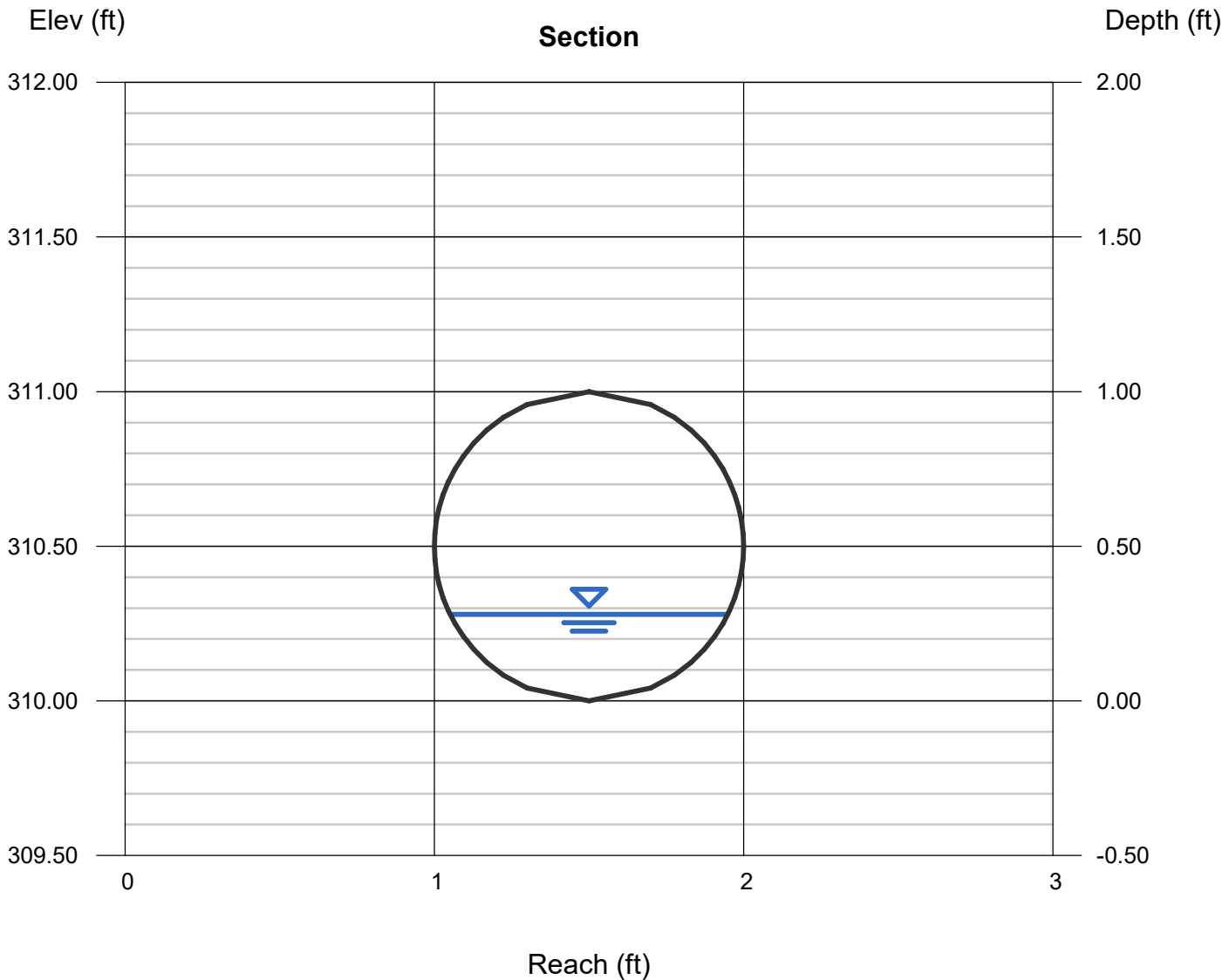
Velocity (ft/s) = 10.55

Wetted Perim (ft) = 1.12

Crit Depth,  $Y_c$  (ft) = 0.59

Top Width (ft) = 0.90

EGL (ft) = 2.01



# Channel Report

## TD-8

### Circular

Diameter (ft) = 1.25

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 20.17

### Highlighted

Depth (ft) = 1.03

Q (cfs) = 20.17

Area (sqft) = 1.08

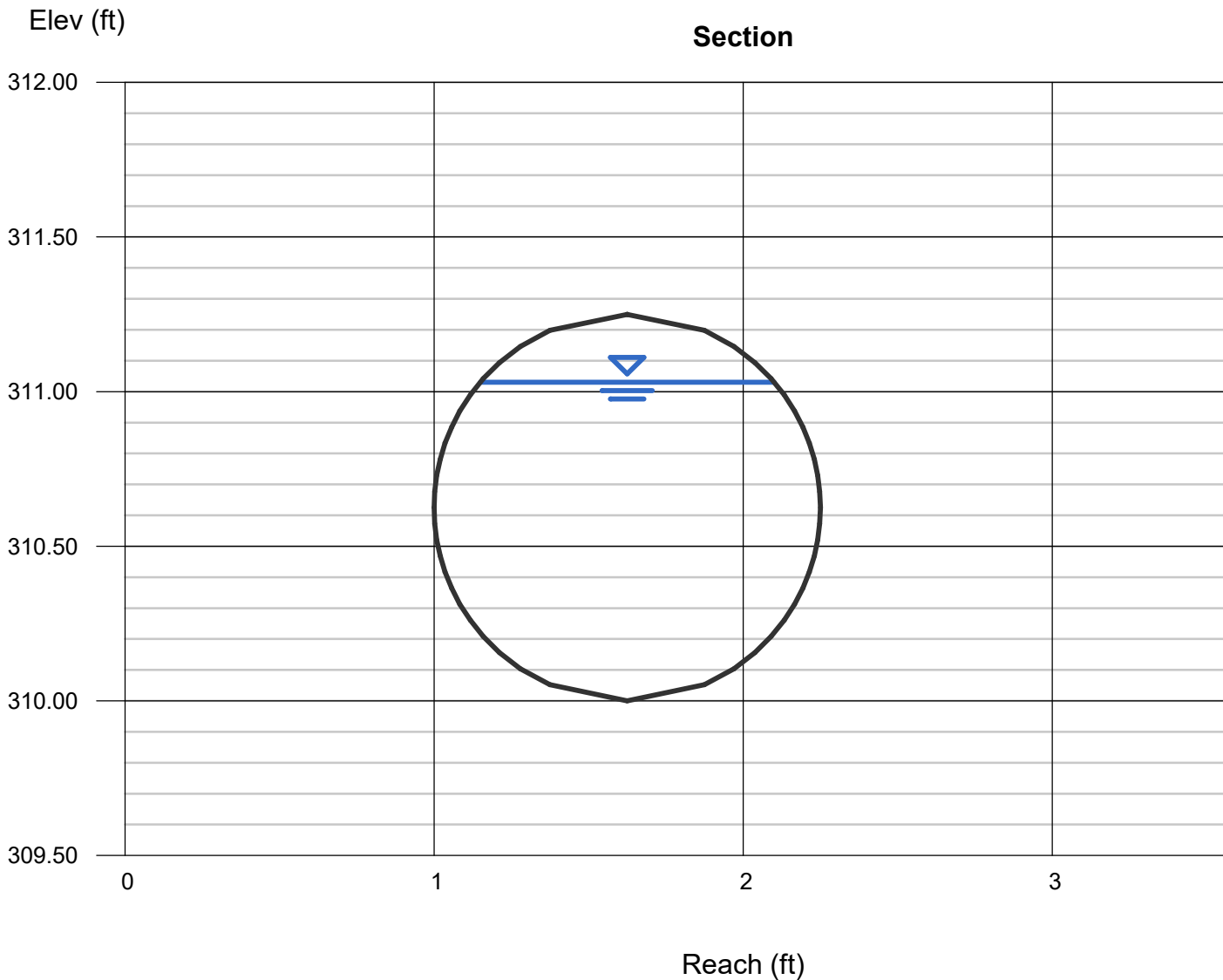
Velocity (ft/s) = 18.61

Wetted Perim (ft) = 2.85

Crit Depth,  $Y_c$  (ft) = 1.25

Top Width (ft) = 0.95

EGL (ft) = 6.42



# Channel Report

## TD-9

### Circular

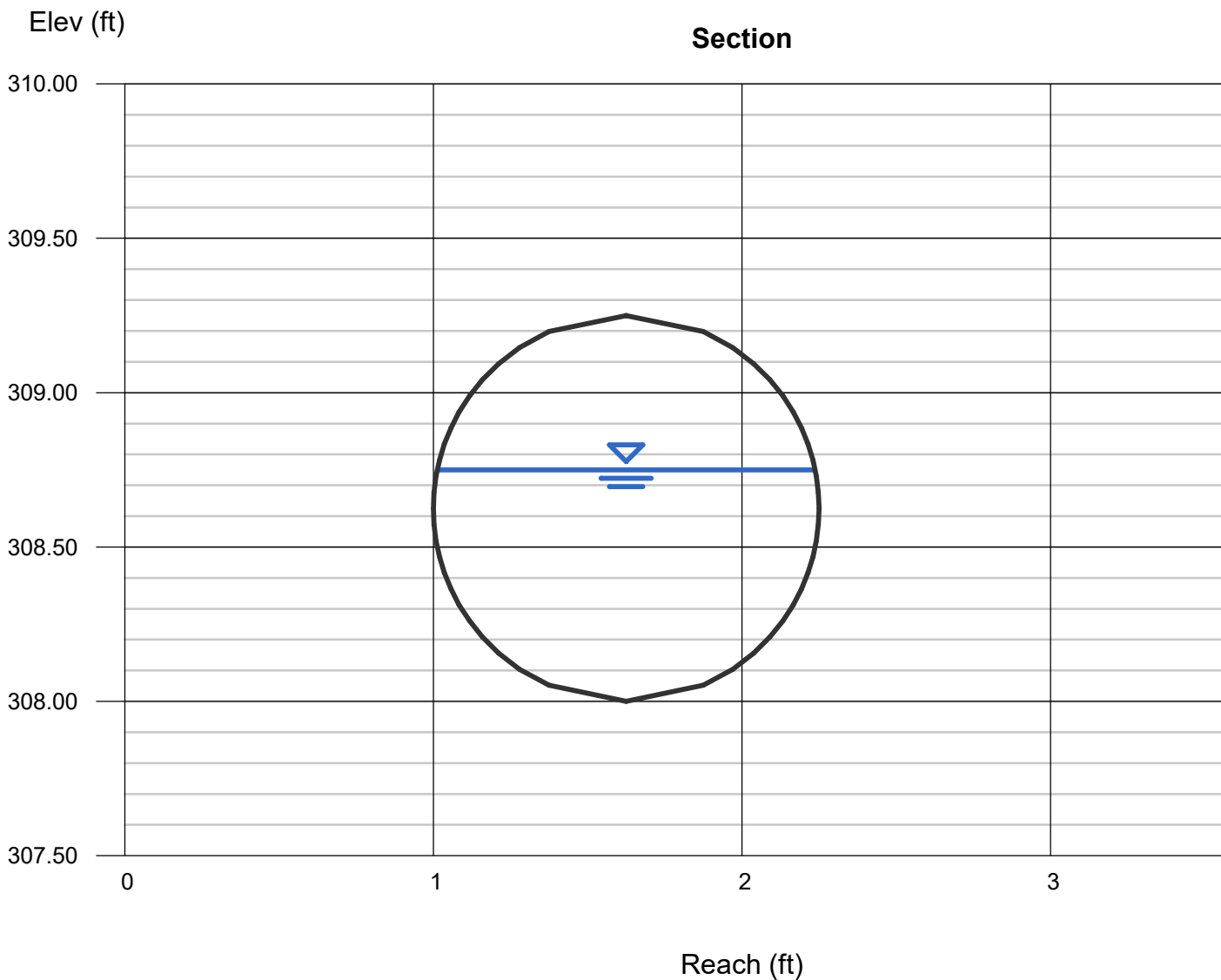
Diameter (ft) = 1.25  
  
Invert Elev (ft) = 308.00  
Slope (%) = 33.33  
N-Value = 0.024

### Highlighted

Depth (ft) = 0.75  
Q (cfs) = 13.52  
Area (sqft) = 0.77  
Velocity (ft/s) = 17.54  
Wetted Perim (ft) = 2.22  
Crit Depth,  $Y_c$  (ft) = 1.23  
Top Width (ft) = 1.22  
EGL (ft) = 5.53

### Calculations

Compute by: Known Q  
Known Q (cfs) = 13.52



# Channel Report

## TD-10

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 7.67

### Highlighted

Depth (ft) = 0.61

Q (cfs) = 7.670

Area (sqft) = 0.50

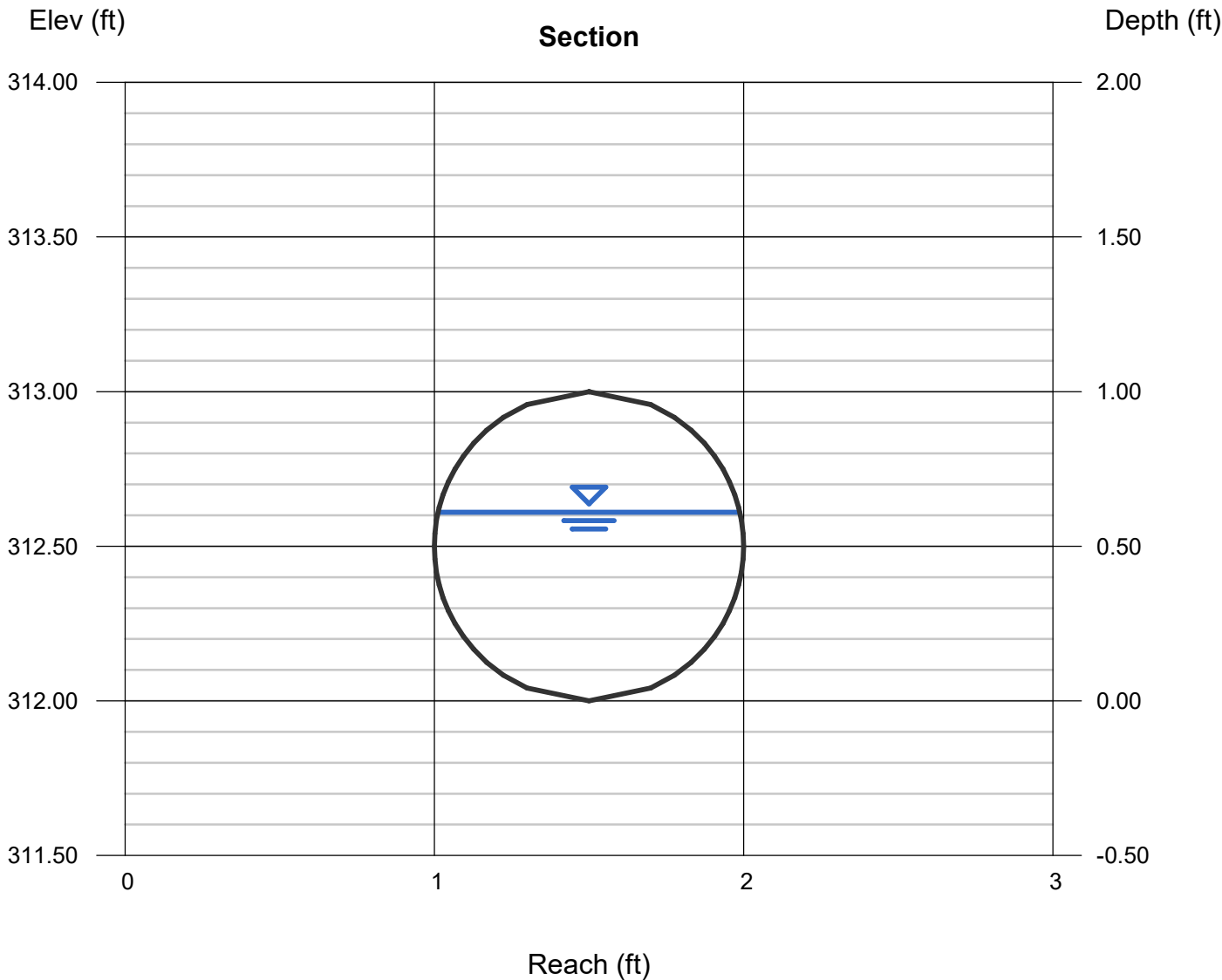
Velocity (ft/s) = 15.25

Wetted Perim (ft) = 1.79

Crit Depth,  $Y_c$  (ft) = 0.99

Top Width (ft) = 0.97

EGL (ft) = 4.23



# Channel Report

## TD-10

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 3.54

### Highlighted

Depth (ft) = 0.39

Q (cfs) = 3.540

Area (sqft) = 0.28

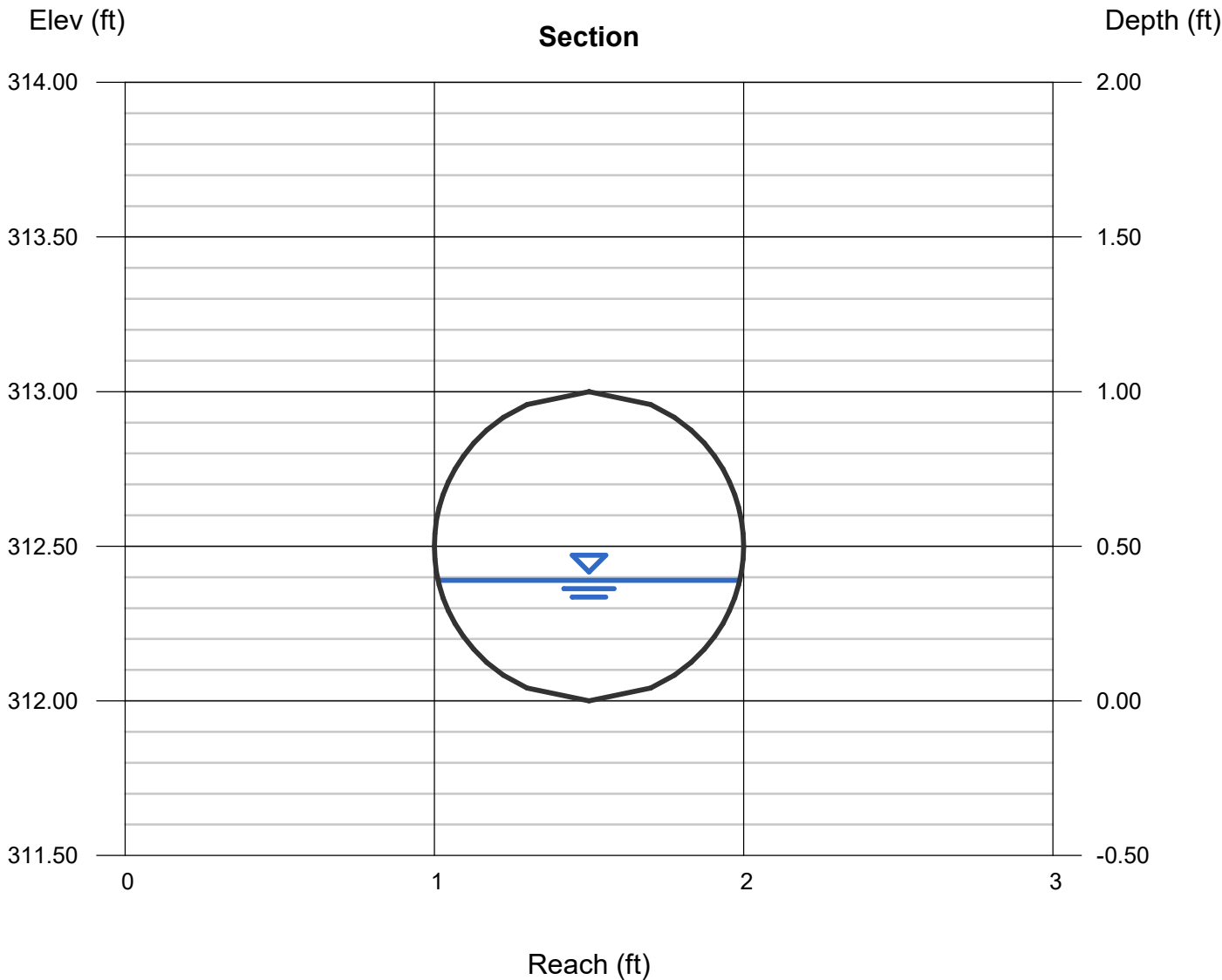
Velocity (ft/s) = 12.46

Wetted Perim (ft) = 1.35

Crit Depth,  $Y_c$  (ft) = 0.81

Top Width (ft) = 0.98

EGL (ft) = 2.80



# Channel Report

## TD-12

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 6.86

### Highlighted

Depth (ft) = 0.57

Q (cfs) = 6.860

Area (sqft) = 0.46

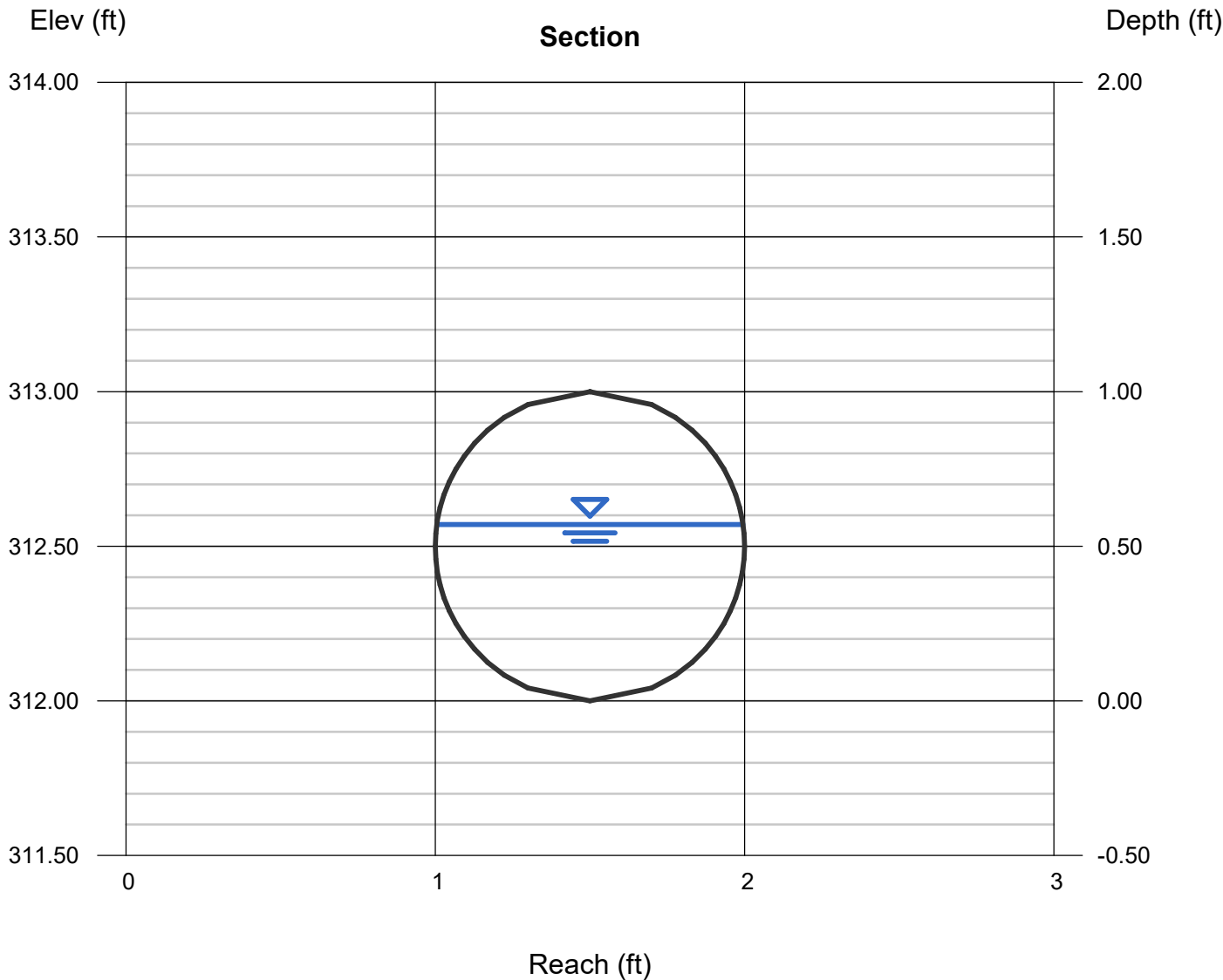
Velocity (ft/s) = 14.77

Wetted Perim (ft) = 1.71

Crit Depth, Yc (ft) = 0.98

Top Width (ft) = 0.99

EGL (ft) = 3.96





# Channel Report

## TD-13

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 6.40

### Highlighted

Depth (ft) = 0.55

Q (cfs) = 6.400

Area (sqft) = 0.44

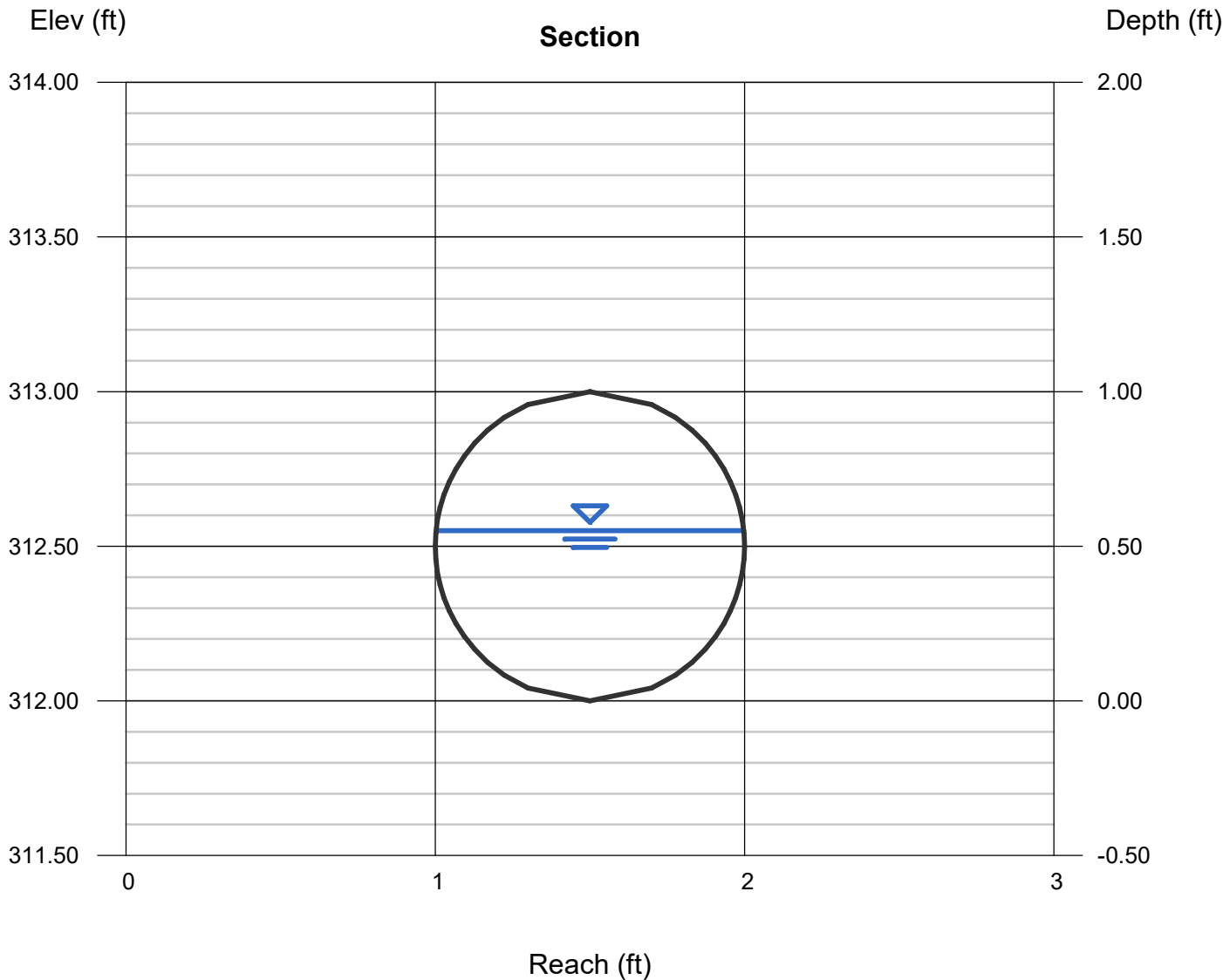
Velocity (ft/s) = 14.39

Wetted Perim (ft) = 1.67

Crit Depth,  $Y_c$  (ft) = 0.97

Top Width (ft) = 0.99

EGL (ft) = 3.77



# Channel Report

## TD-14

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 311.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 5.24

### Highlighted

Depth (ft) = 0.49

Q (cfs) = 5.240

Area (sqft) = 0.38

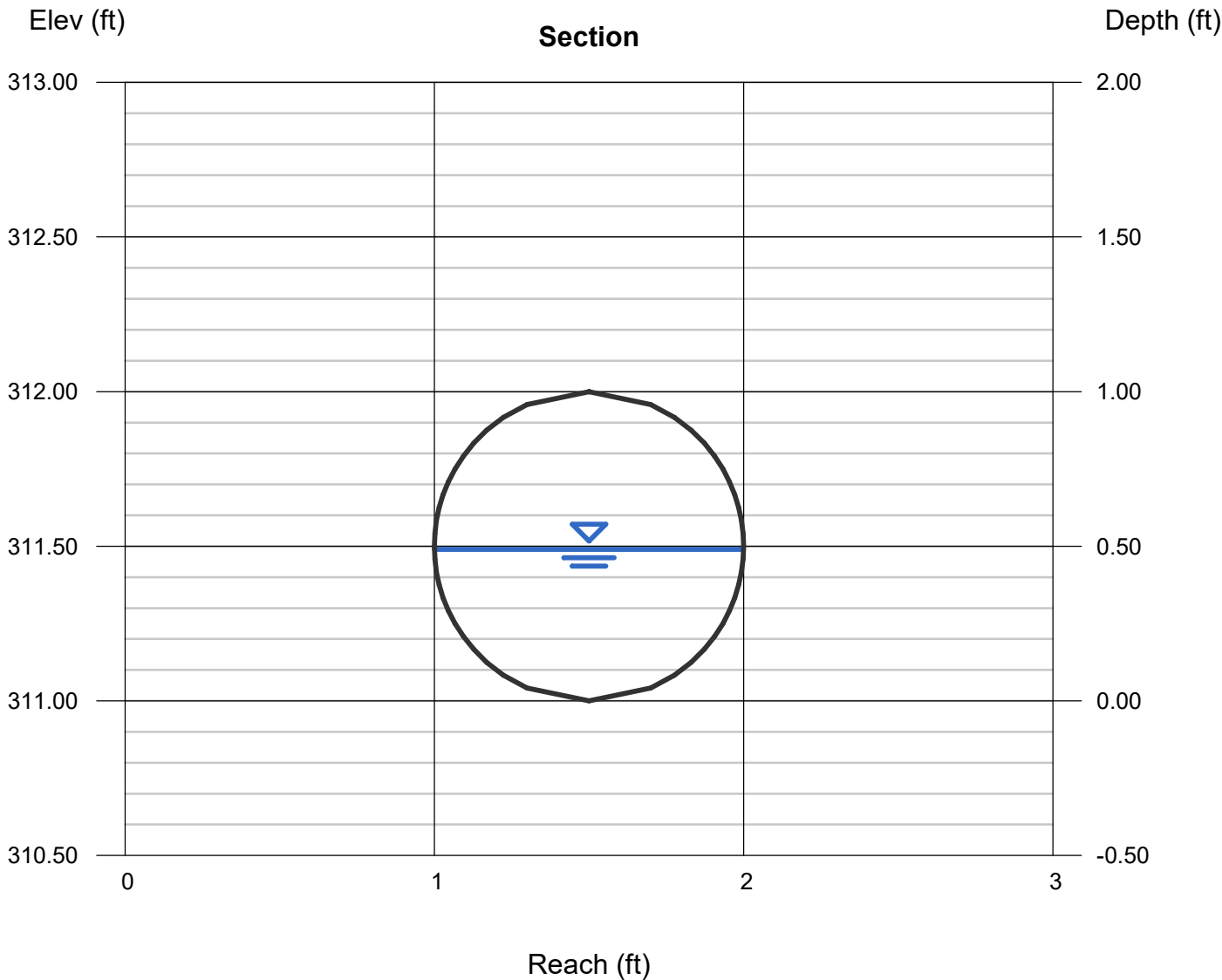
Velocity (ft/s) = 13.62

Wetted Perim (ft) = 1.55

Crit Depth,  $Y_c$  (ft) = 0.93

Top Width (ft) = 1.00

EGL (ft) = 3.37



# Channel Report

## TD-15

### Circular

Diameter (ft) = 1.25

Invert Elev (ft) = 302.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 18.45

### Highlighted

Depth (ft) = 0.94

Q (cfs) = 18.45

Area (sqft) = 0.99

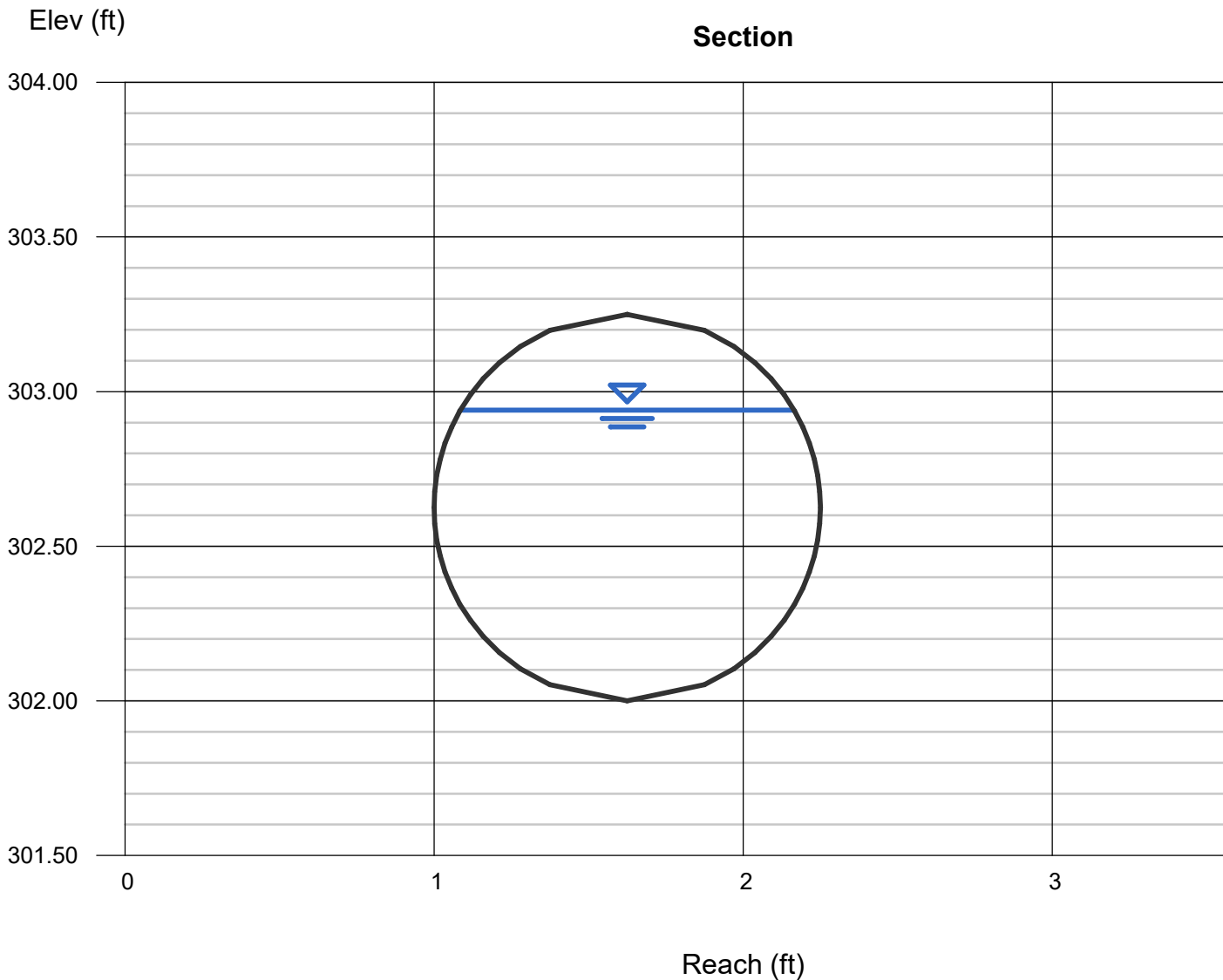
Velocity (ft/s) = 18.63

Wetted Perim (ft) = 2.63

Crit Depth,  $Y_c$  (ft) = 1.25

Top Width (ft) = 1.08

EGL (ft) = 6.33



# Channel Report

## TD-16

### Circular

Diameter (ft) = 1.25

Invert Elev (ft) = 299.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 13.30

### Highlighted

Depth (ft) = 0.74

Q (cfs) = 13.30

Area (sqft) = 0.76

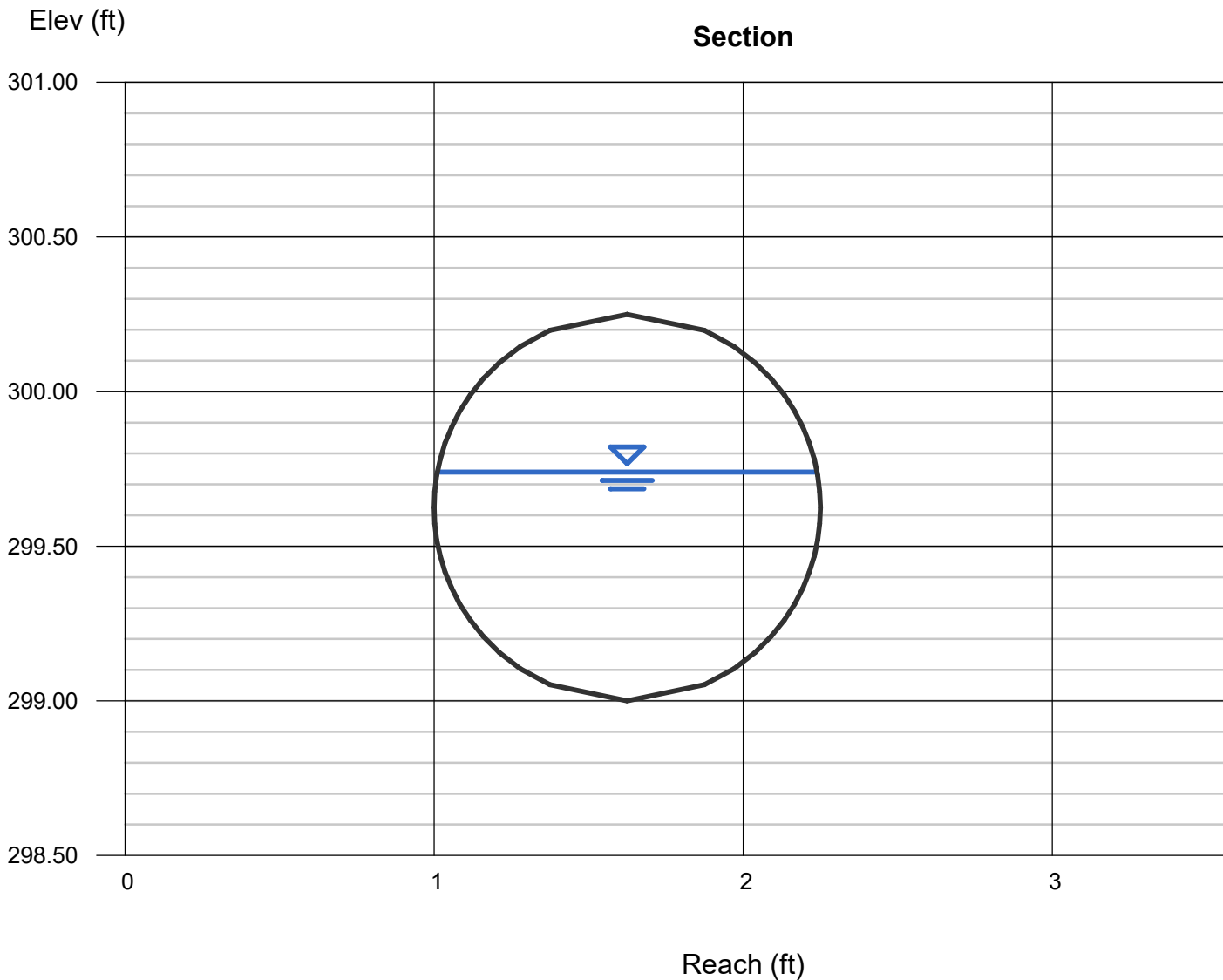
Velocity (ft/s) = 17.51

Wetted Perim (ft) = 2.20

Crit Depth,  $Y_c$  (ft) = 1.23

Top Width (ft) = 1.23

EGL (ft) = 5.51



# Channel Report

## TD-17

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 299.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 6.46

### Highlighted

Depth (ft) = 0.55

Q (cfs) = 6.460

Area (sqft) = 0.44

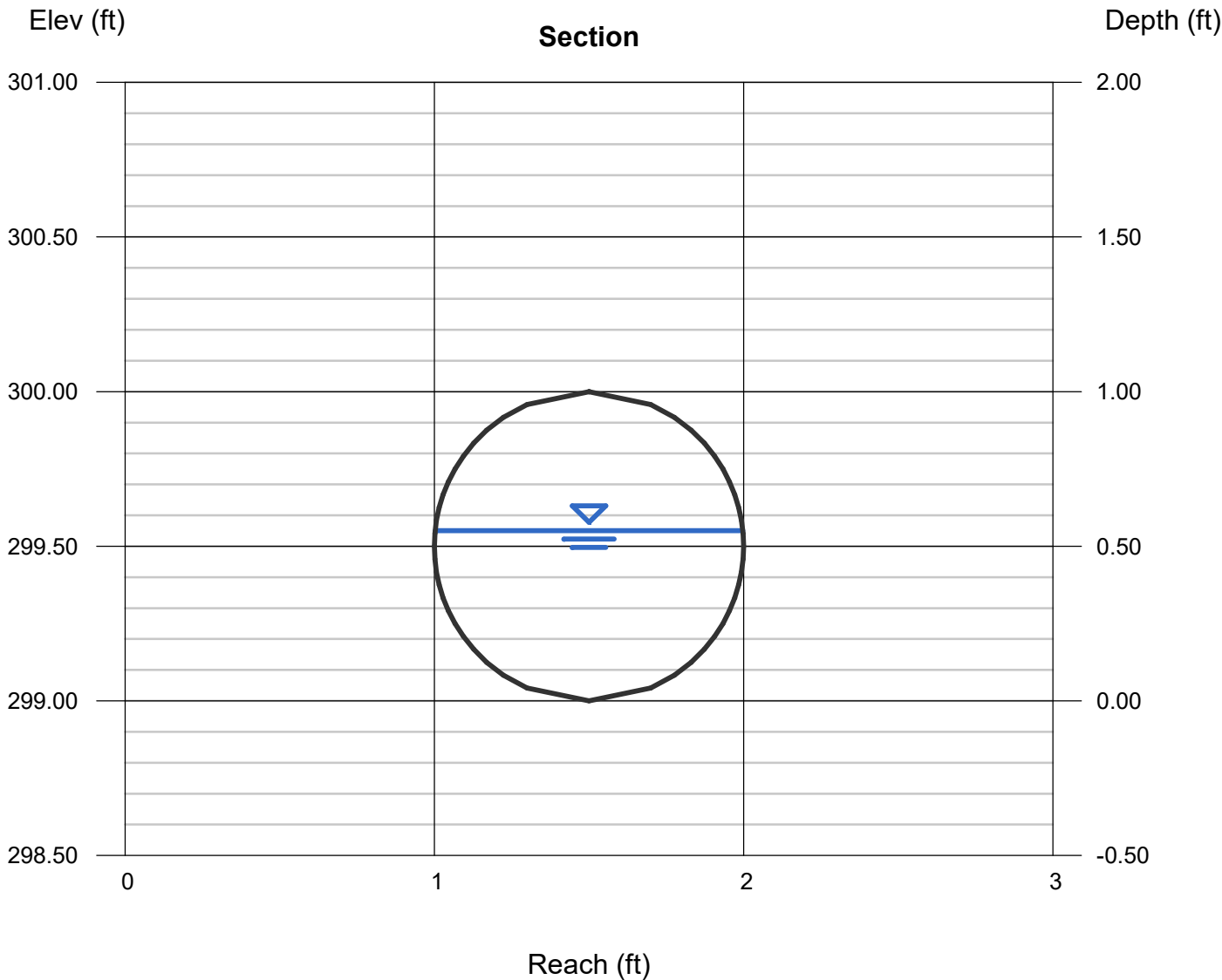
Velocity (ft/s) = 14.53

Wetted Perim (ft) = 1.67

Crit Depth, Yc (ft) = 0.97

Top Width (ft) = 0.99

EGL (ft) = 3.83



# Channel Report

## TD-18

### Circular

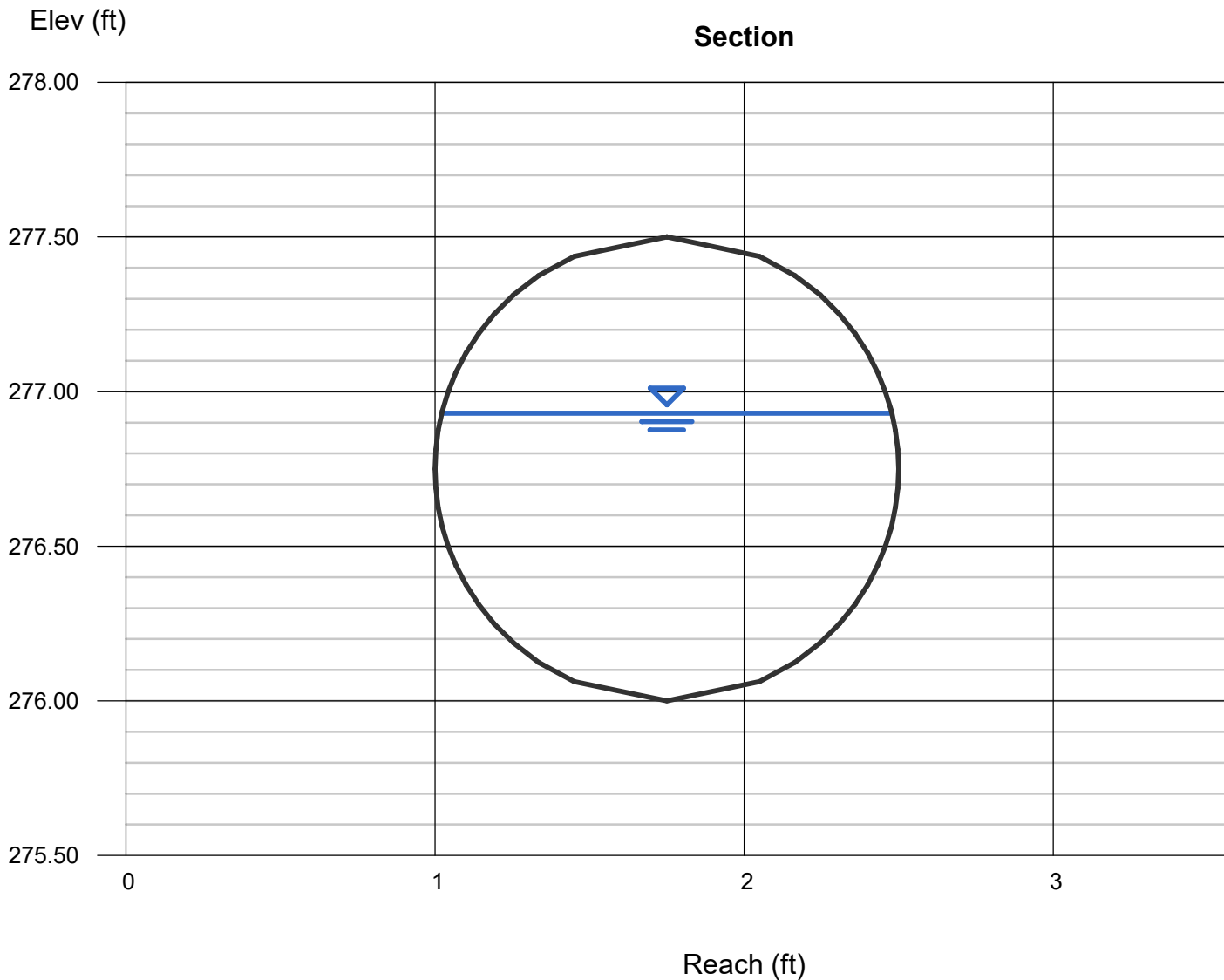
Diameter (ft) = 1.50  
  
Invert Elev (ft) = 276.00  
Slope (%) = 33.33  
N-Value = 0.024

### Highlighted

Depth (ft) = 0.93  
Q (cfs) = 22.97  
Area (sqft) = 1.15  
Velocity (ft/s) = 19.92  
Wetted Perim (ft) = 2.72  
Crit Depth, Yc (ft) = 1.49  
Top Width (ft) = 1.46  
EGL (ft) = 7.10

### Calculations

Compute by: Known Q  
Known Q (cfs) = 22.97



# Channel Report

## TD-19

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 310.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 2.11

### Highlighted

Depth (ft) = 0.30

Q (cfs) = 2.110

Area (sqft) = 0.20

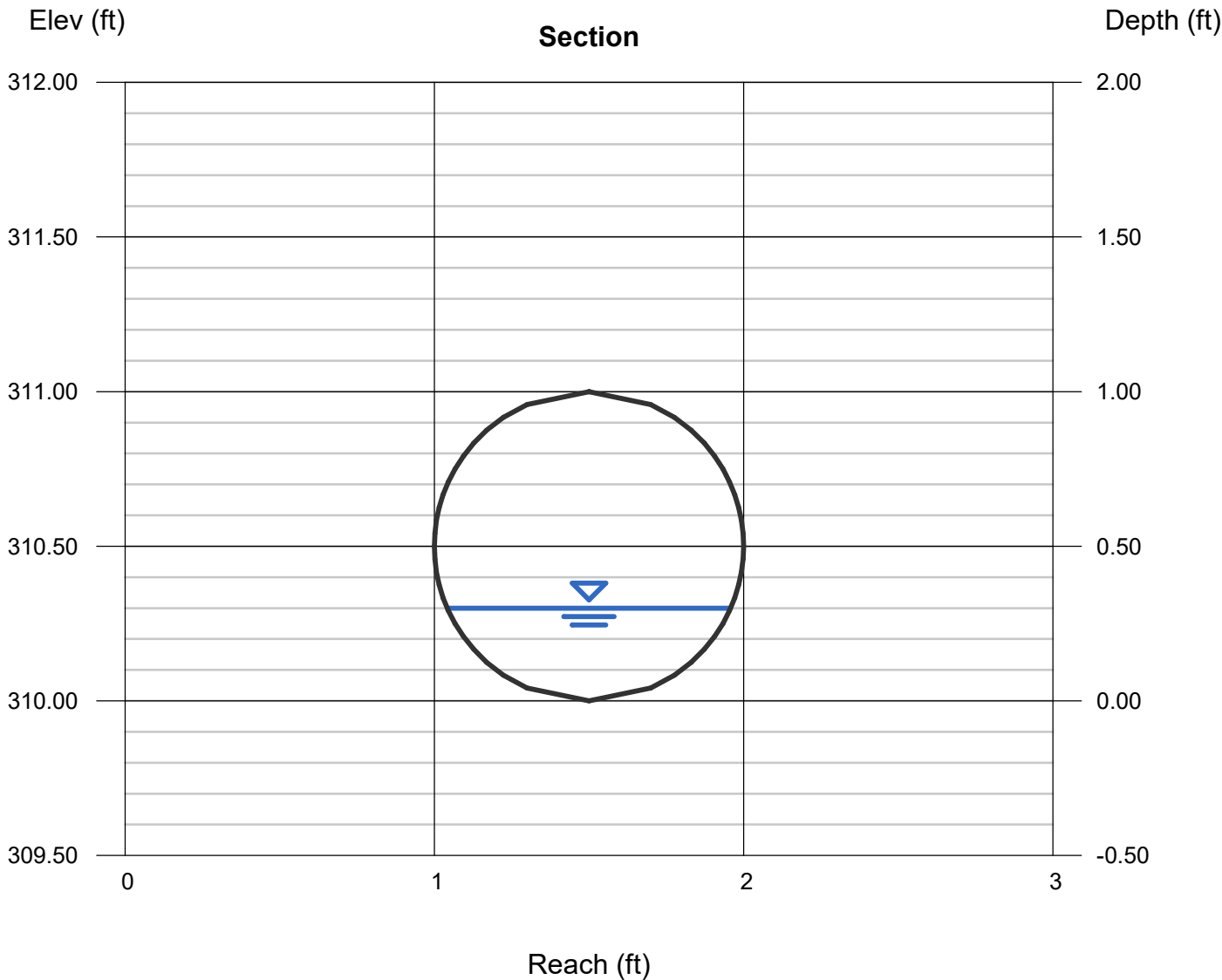
Velocity (ft/s) = 10.63

Wetted Perim (ft) = 1.16

Crit Depth,  $Y_c$  (ft) = 0.62

Top Width (ft) = 0.92

EGL (ft) = 2.06



# Channel Report

## TD-20

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 313.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 10.43

### Highlighted

Depth (ft) = 0.77

Q (cfs) = 10.43

Area (sqft) = 0.65

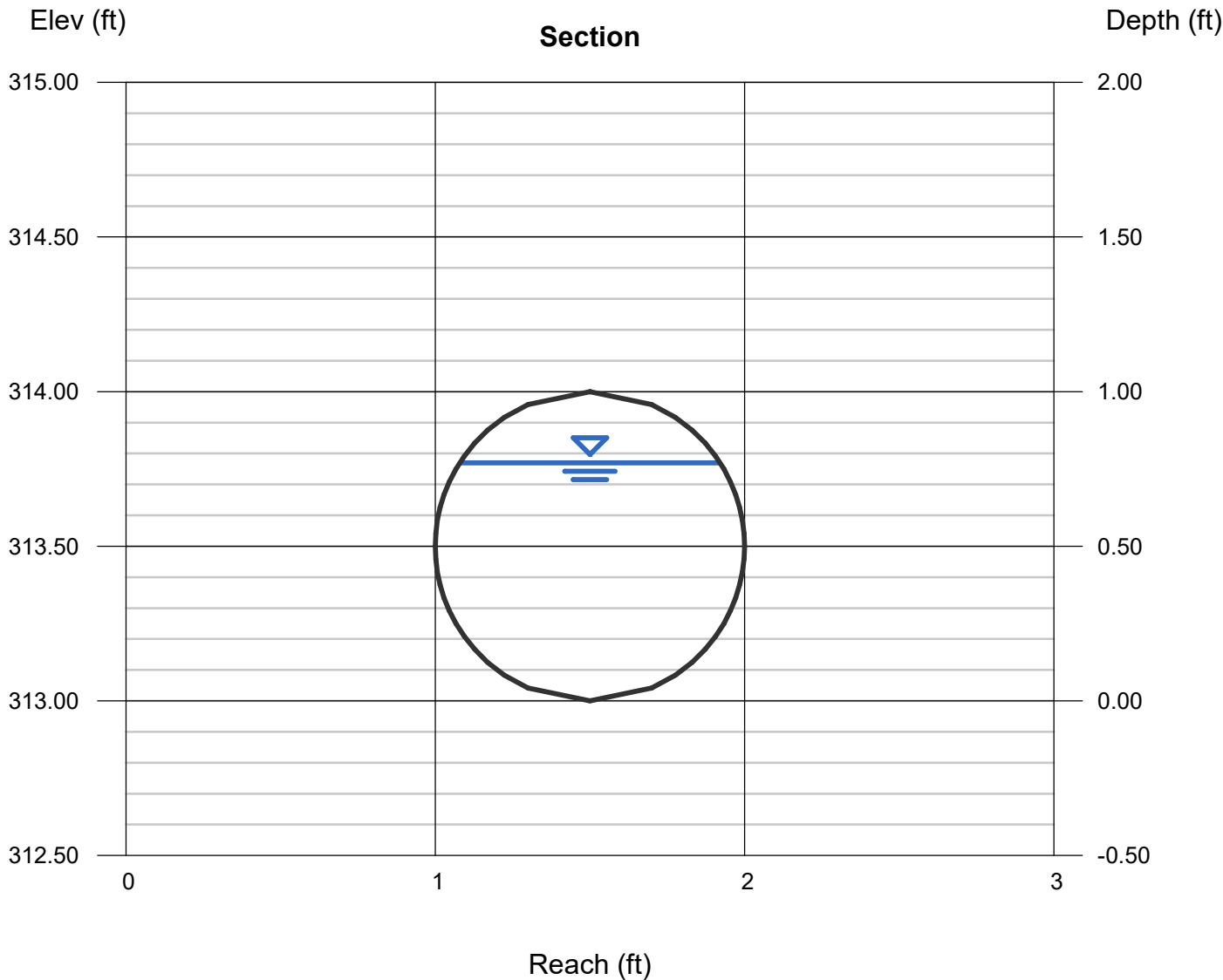
Velocity (ft/s) = 16.04

Wetted Perim (ft) = 2.15

Crit Depth,  $Y_c$  (ft) = 1.00

Top Width (ft) = 0.84

EGL (ft) = 4.77





# Channel Report

## TD-21

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 318.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 3.87

### Highlighted

Depth (ft) = 0.41

Q (cfs) = 3.870

Area (sqft) = 0.30

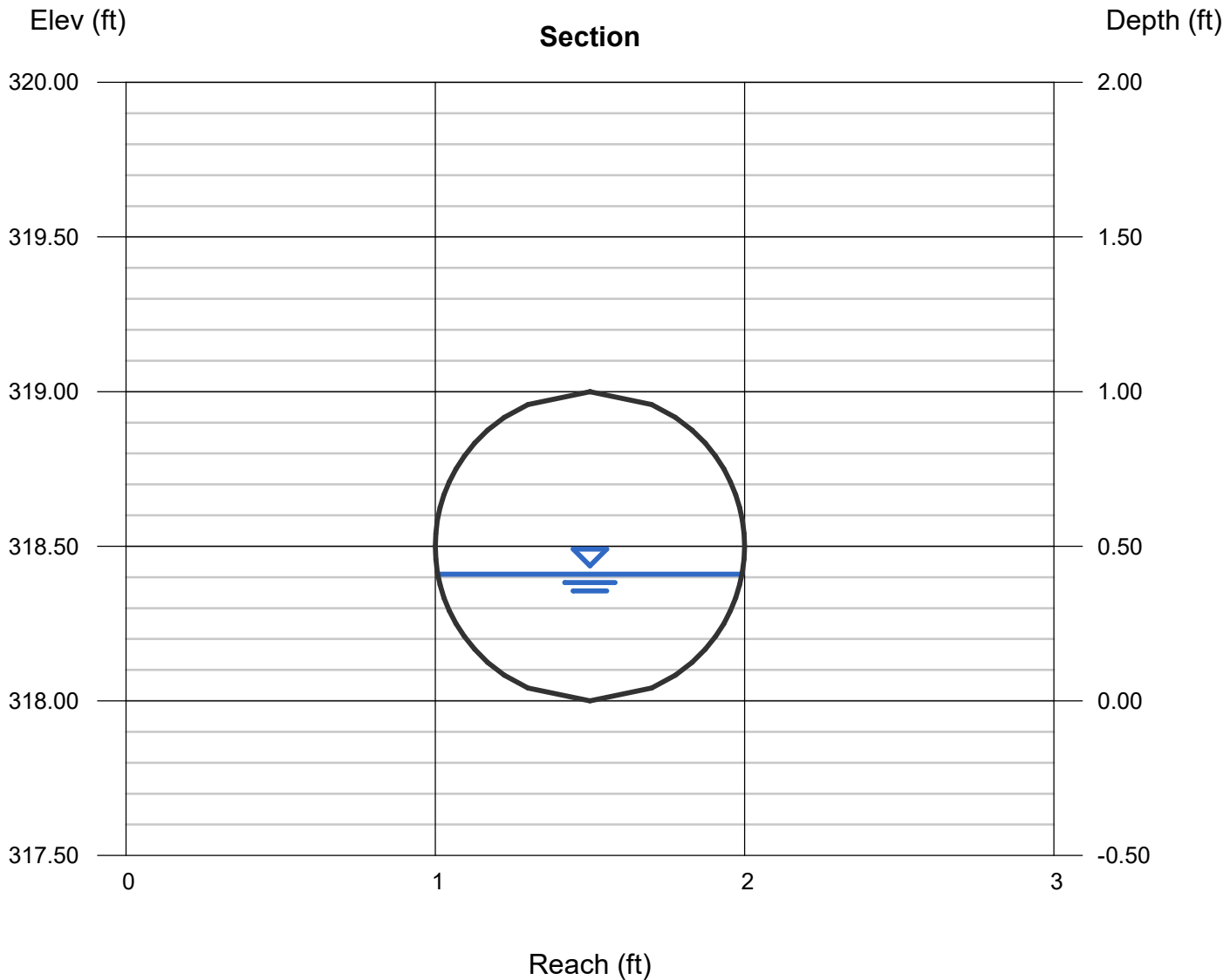
Velocity (ft/s) = 12.76

Wetted Perim (ft) = 1.39

Crit Depth,  $Y_c$  (ft) = 0.84

Top Width (ft) = 0.98

EGL (ft) = 2.94



# Channel Report

## TD-22

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 318.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 4.89

### Highlighted

Depth (ft) = 0.47

Q (cfs) = 4.890

Area (sqft) = 0.36

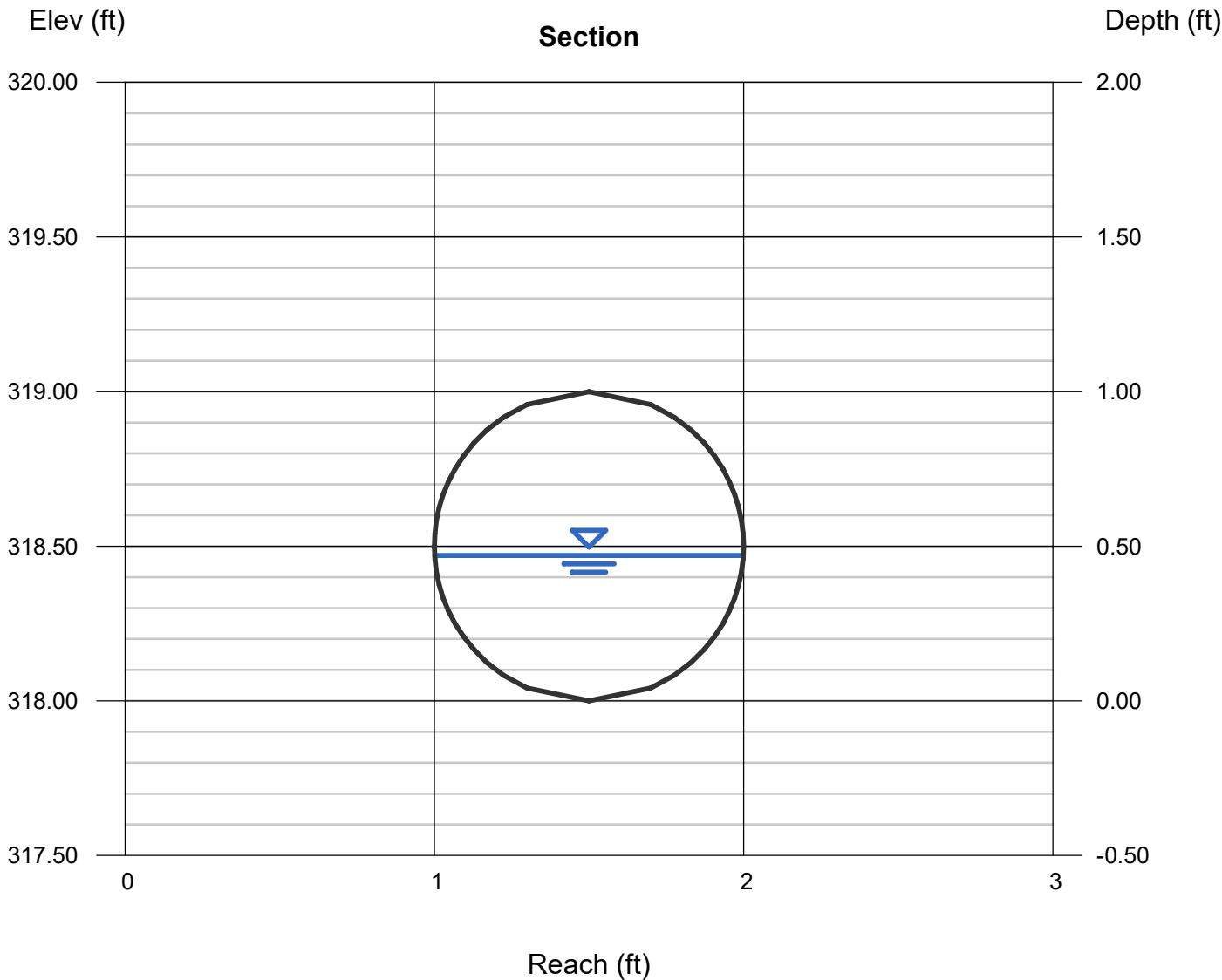
Velocity (ft/s) = 13.40

Wetted Perim (ft) = 1.51

Crit Depth,  $Y_c$  (ft) = 0.92

Top Width (ft) = 1.00

EGL (ft) = 3.26



# Channel Report

## TD-23

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 313.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 8.33

### Highlighted

Depth (ft) = 0.65

Q (cfs) = 8.330

Area (sqft) = 0.54

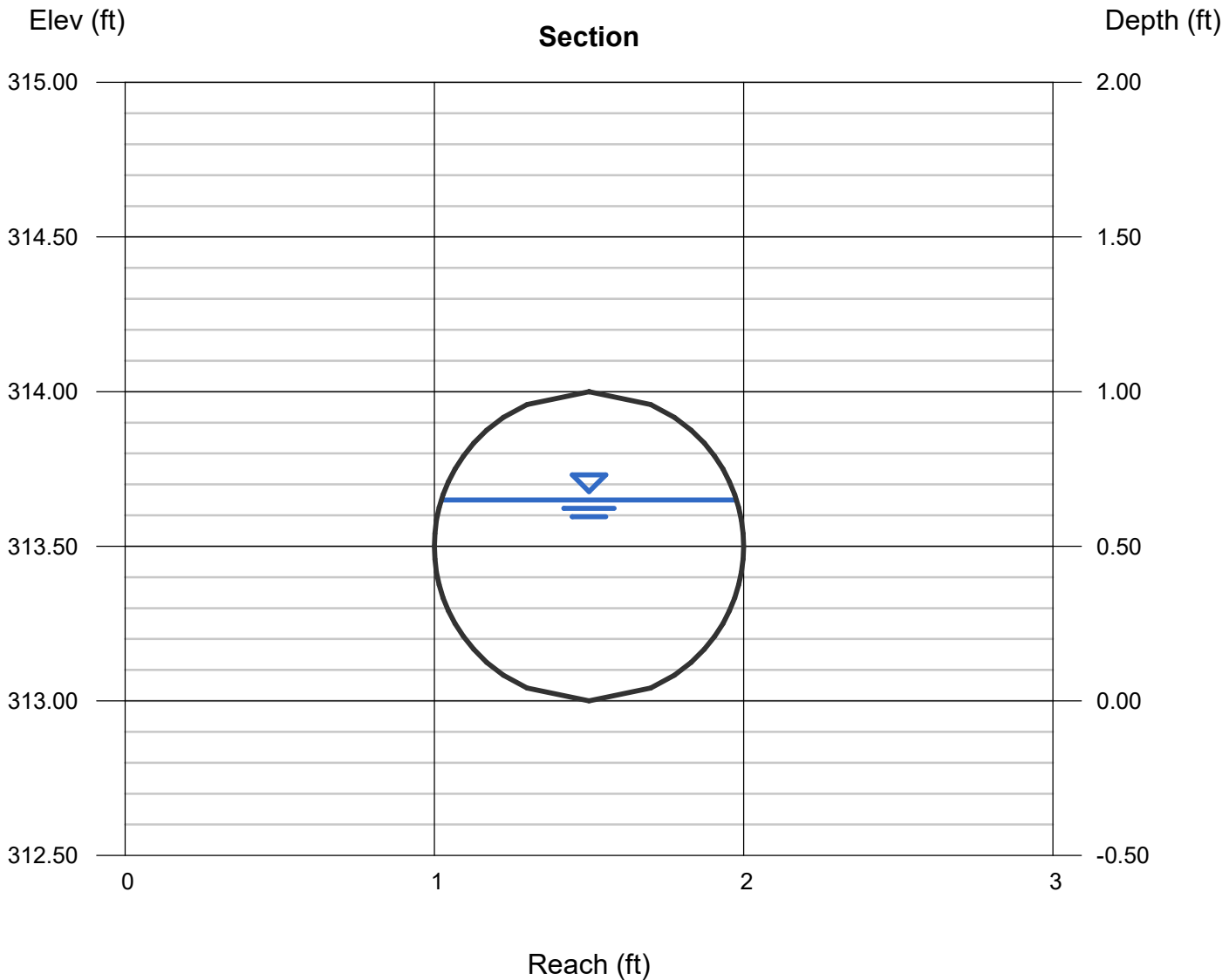
Velocity (ft/s) = 15.36

Wetted Perim (ft) = 1.88

Crit Depth, Yc (ft) = 0.99

Top Width (ft) = 0.95

EGL (ft) = 4.32



# Channel Report

## TD-24

### Circular

Diameter (ft) = 1.25

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 18.85

### Highlighted

Depth (ft) = 0.96

Q (cfs) = 18.85

Area (sqft) = 1.01

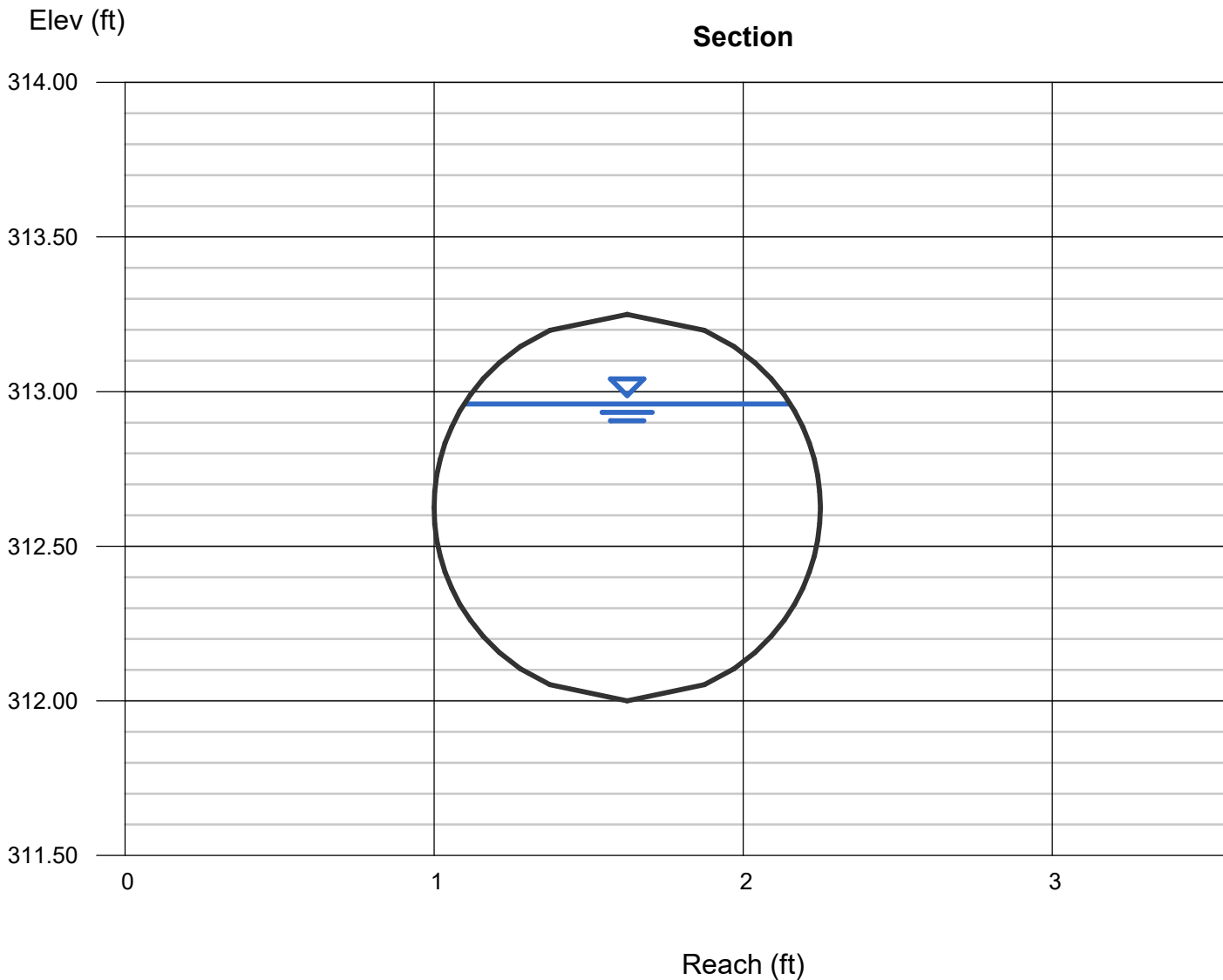
Velocity (ft/s) = 18.60

Wetted Perim (ft) = 2.68

Crit Depth,  $Y_c$  (ft) = 1.25

Top Width (ft) = 1.05

EGL (ft) = 6.34



# Channel Report

## TD-25

### Circular

Diameter (ft) = 1.25

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 13.23

### Highlighted

Depth (ft) = 0.74

Q (cfs) = 13.23

Area (sqft) = 0.76

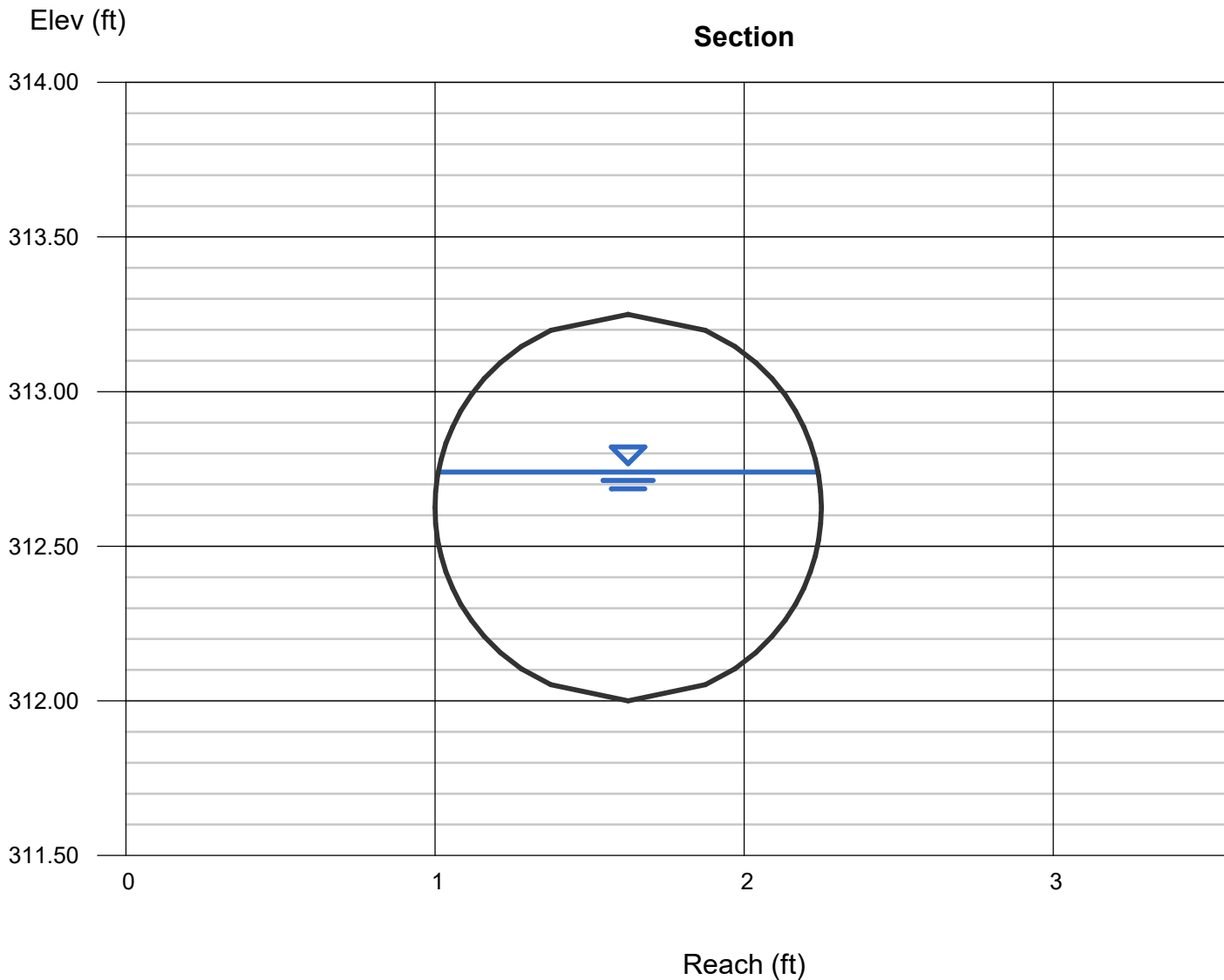
Velocity (ft/s) = 17.42

Wetted Perim (ft) = 2.20

Crit Depth,  $Y_c$  (ft) = 1.23

Top Width (ft) = 1.23

EGL (ft) = 5.46



# Channel Report

## TD-26

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 312.00

Slope (%) = 33.33

N-Value = 0.024

### Calculations

Compute by: Known Q

Known Q (cfs) = 6.23

### Highlighted

Depth (ft) = 0.54

Q (cfs) = 6.230

Area (sqft) = 0.43

Velocity (ft/s) = 14.33

Wetted Perim (ft) = 1.65

Crit Depth, Yc (ft) = 0.97

Top Width (ft) = 1.00

EGL (ft) = 3.73

