

# Erosion Control Calculations

**401 WEST GANNON AVENUE**  
Zebulon, North Carolina

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## **INTRODUCTION AND GENERAL INFORMATION**

This report presents the erosion control design for the land disturbance for the proposed 401 West Gannon Avenue project located at the intersection of West Gannon Avenue and North Rotary Drive in Zebulon, North Carolina.

## **BACKGROUND**

The proposed development is comprised of approximately 0.99 acres of land and is currently vacant. At full build out the project will consist of 11 condominium units with infrastructure to support the proposed development as well as one SCM to meet the Town's stormwater requirements.

## **EROSION CONTROL REQUIREMENTS**

All sediment and erosion control devices were designed in accordance with requirements and specifications by Wake County and in the North Carolina Erosion and Sediment Control Planning and Design Manual.

Sediment and erosion control devices used for this project include sediment traps with a Faircloth skimmer, silt fence, silt fence outlets, diversion berms, and construction entrances. Diversion berms should be located as shown on the plans to divert surface flow into the sediment traps. Silt fence will be utilized where the drainage area is less than  $\frac{1}{4}$  acre per 100 linear feet of silt fence. Silt fence outlets will be located at low points along the silt fence to prevent high flow velocities from blowing out the silt fence while still allowing for the sediment to settle out of the surface flow. The construction entrances will be located at various locations for access to the areas of the project.

### **CALCULATIONS**

The following equations were used in developing the calculated values in shown in the appendices to this report.

#### **Sediment Traps and Basins**

- Peak flow to the basins were determined using the rational formula
- The required volume was calculated based on 1800 cubic feet per acre draining to the basin
- The required surface area was calculated based on 435 square feet of surface area per cfs for the 10-year storm
- The sediment basin volume provided was calculated by the average end area method applied vertically
- The sediment basin surface area was taken from contour areas taken from the proposed basin grading, or interpolated between contours if necessary.
- The volume to dewater was set equal to the required volume of the sediment basin
- Skimmer orifice sizes were determined by the orifice equation based on the head available for the selected skimmer size
- The depth of flow over the spillway was calculated with the weir equation using the selected spillway length and peak flow from the 10-year storm

## SEDIMENT TRAP DRAINAGE AREA MAP



## SEDIMENT TRAP CALCULATIONS

### SKIMMER BASIN DESIGN

 Location ID           SCM/Trap          
**Drainage Area Data**

Disturbed area	0.6 acres
Design storm for surface area calcs	10-year
Runoff for surface area calcs	2.0 cfs
Design storm for spillway calcs	10-year
Runoff for spillway calcs	4.0 cfs
Calculation methodology	Software/rational calcs

**Skimmer Basin Design Requirements**

Sediment storage per disturbed acre	1800 cubic feet/acre
Surface area per inflow CFS	435 square feet/cfs
Minimum trap dewater time	2 days
Maximum trap dewater time	5 days
Maximum sediment storage depth	3.5 feet
Minimum freeboard from sediment	1.5 feet
Minimum spillway length	10.0 feet
Maximum spillway flow depth	0.50 feet
Minimum freeboard for spillway flow	1.00 feet
Volume to dewater	Required

**Skimmer Basin Design Data**

Sediment storage depth	2.0 feet
Height to spillway	2.0 feet
Spillway length	20.0 feet
Skimmer size	1.50 in.
Orifice size	0.50 in.

**Sediment Trap Contour and Volume Data**

Elevation	Stage	Contour area	Incremental Volume	Cumulative volume
342.0	0.0	78 sf	0	0
343.0	1.0	691 sf	385	385
344.0	2.0	1,914 sf	1,303	1,687
345.0	3.0	3,746 sf	2,830	4,517
345.5	3.5	4,123 sf	1,967	6,484

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**SKIMMER BASIN DESIGN**


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 Location ID SCM/Trap


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**Sediment Trap Dewatering**


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Sediment Trap Dewatering Calculations	
Volume to dewater	990 CF
Skimmer size	1.5 in.
Orifice diameter	0.50 in.
Coefficient of discharge	0.60
Orifice area	0.0014 SF
Head on orifice	0.125 ft
Flowrate	0.002 cfs
Dewatering time	4.9 days

Head on skimmer orifice	
Skimmer size	Head
1.5 in.	0.125 ft.
2.0 in.	0.167 ft.
2.5 in.	0.208 ft.
3.0 in.	0.250 ft.
4.0 in.	0.333 ft.
5.0 in.	0.333 ft.
6.0 in.	0.417 ft.
8.0 in.	0.500 ft.

**CHECK DESIGN REQUIREMENTS**


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Design criteria	Required	Provided
Sediment storage volume	990 CF	1,687 CF
Sediment surface area	870 CF	1,914 CF
Sediment storage depth	3.5 ft. (max)	2.0 feet
Sediment freeboard	1.5 ft. (min)	1.5 ft
Spillway length	10.0 ft. (min)	20.0 ft
Flow depth over spillway	0.50 ft. (max)	0.15 ft
Freeboard at design discharge	1.0 ft. (min)	1.35 ft
Trap dewatering time	2 to 5 days	4.9 days

Note: data and methodology taken from NC Erosion Control Manual

## **RATIONAL RUNOFF CALCULATIONS TO TRAPS**

# Hydrograph Report

Project Name: New Project

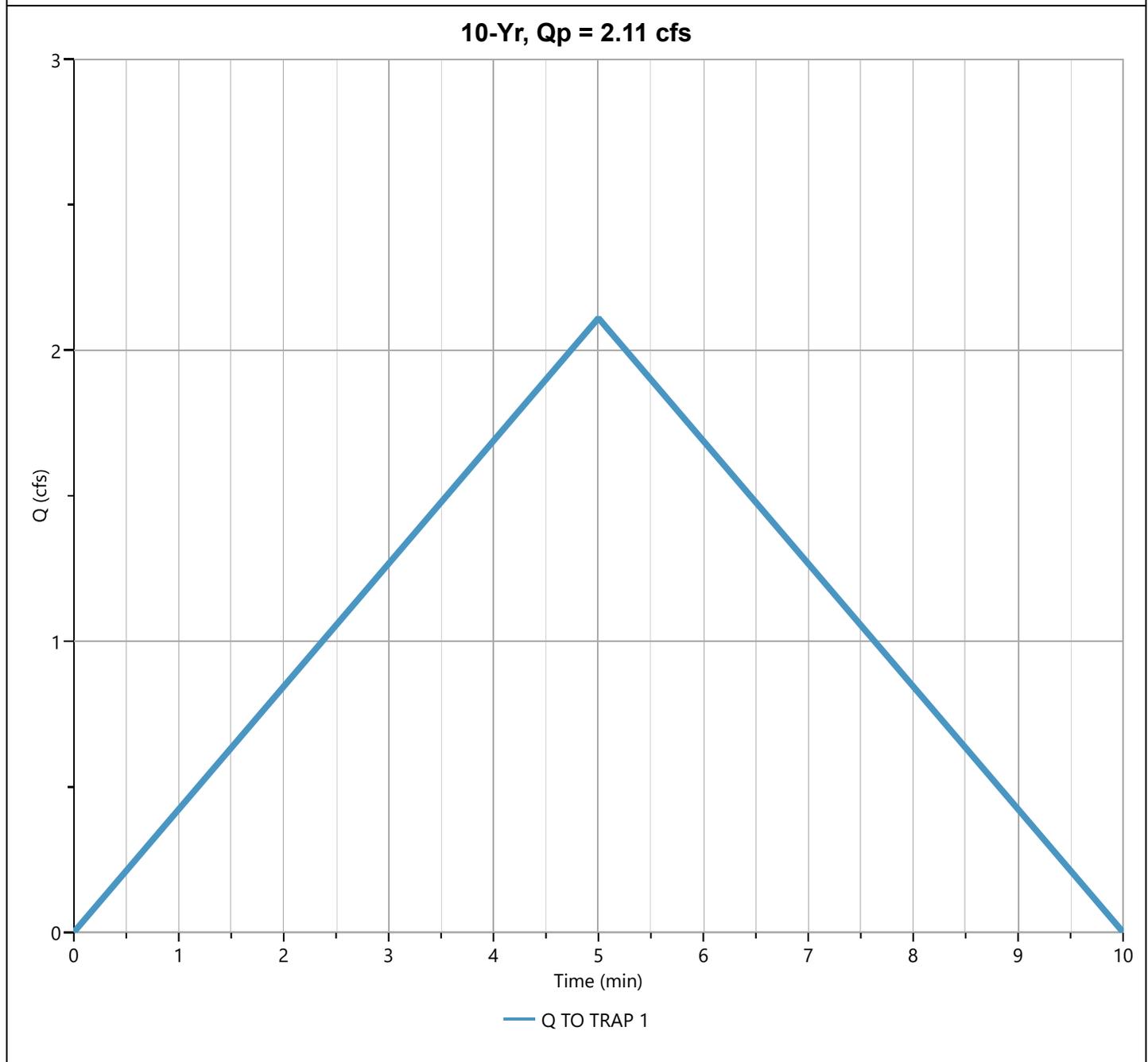
Studio Express by Hydrology Studio v 1.0.0.14

07-10-2023

## Q TO TRAP 1

## Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 2.111 cfs
Storm Frequency	= 10-yr	Time to Peak	= 5 min
Time Interval	= 1 min	Runoff Volume	= 633 cft
Drainage Area	= 0.6 ac	Runoff Coeff.	= 0.5
Tc Method	= User-Defined	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= RDU-Rainfall Intensity.idf	Intensity	= 7.036 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1



**OUTLET PROTECTION/RIPRAP APRON CALCULATIONS**

OUTLET FLOWRATE 1.0 cfs  
 PIPE DIAMETER 12 inches  
 OUTLET PIPE SLOPE 1.00 %  
 NUMBER OF PIPES 1  
 PIPE SEPARATION 0 feet

ZONE FROM GRAPH 1

PIPE AREA 0.79 sq. ft.  
 FLOW VELOCITY 1.3 ft/sec

MATERIAL NCDOT Class A riprap  
 LENGTH 4.00 feet  
 WIDTH 3.00 feet  
 STONE DIAMETER 3 inches  
 THICKNESS 9 inches

Zone	Material	Diameter	Thickness	Length	Width
1	Class A	3	9	4 x D(o)	3 x D(o)
2	Class B	6	22	6 x D(o)	3 x D(o)
3	Class I	13	22	8 x D(o)	3 x D(o)
4	Class I	13	22	8 x D(o)	3 x D(o)
5	Class II	23	27	10 x D(o)	3 x D(o)
6	Class II	23	27	10 x D(o)	3 x D(o)
7	Special study required				

1. Calculations based on NY DOT method - Pages 8.06.05 through 8.06.06 in NC Erosion Control Manual
2. Outlet velocity based on full-flow velocity

