# Storm Water Impact Analysis 

Domino's Zebulon<br>1000 Hendricks Drive<br>Zebulon, NC 27597

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Project \#: 2023018


## Project Overview:

This narrative describes the routing analysis (pre- vs post-) as depicted on the sheets titled "PreDevelopment and Post-Development Drainage Area Map".

The proposed project site is located at 1000 Hendricks Drive, Zebulon, NC.
The existing property consists primarily of managed lawn. There is one existing dry detention pond that will remain. The new project consists of 1 commercial building (restaurant), parking lot, driveway connections and a slight road widening.

Refer to the attached Post Development DA Map for the routing study area.
The existing dry pond was initially designed to collect storm water from 3 developments in the immediate area. The purpose of this study is to make sure that our development will adequately work with the existing dry pond.

## Existing Conditions (Routing Analysis):

Refer to the Pre-Developed Drainage Area Map of this report. The existing property consists of managed lawn with a ditch near the south property corner.

The site generally slopes from north to south. The onsite project area is 0.93 acres for the Domino's site specifically. For this study, all 3 developments that discharge to the dry pond will be considered onsite equating to 2.84 acres. There are currently 2 other commercial developments that drain to the dry detention pond on our site.

There is offsite drainage area totaling 3.62 acres of existing agricultural crops from the north, which enters the property as shown on the Pre-Developed DA Map.

The Soils Survey mapping shows that the soils on the site are predominately Ur (Urban Land):
The USGS and soils survey maps indicate that there are no streams onsite. There are no known wetlands on the property. FEMA FIRM 3720270600 K indicates that there is no regulatory floodplain on the property, and the site lies within Zone X.

## Post-Developed Conditions (Routing Analysis):

Refer to the Post-Developed Drainage Area Map of this report. The project includes 1 commercial building, parking lot, and an existing dry detention pond. Refer to the Site Plan sheet for all lot impervious allocations. The project includes minor road widening on E. Jones Street.

Total Proposed Onsite Impervious for this Study $=1.89 \mathrm{AC}$
Proposed Offsite Impervious = 0.42 AC

## Predeveloped Conditions:

Onsite:
$\mathrm{A}=2.84$ acres
$\mathrm{CN}=60$
Offsite:
$\mathrm{A}=3.62$ acres
$\mathrm{CN}=68$
$\mathrm{A}=0.42$ acres
$\mathrm{CN}=98$
Combined:
$\mathrm{A}=6.88$ acres
$\mathrm{CN}=67$
(SCS Method)
Predeveloped Runoff Summary

|  | Predeveloped Runoff |
| :---: | :---: |
| Year | CFS |
| 1 | 1.73 |
| 2 | 3.19 |
| 5 | 5.88 |
| 10 | 8.37 |
| 25 | 12.11 |
| 100 | 18.93 |

## FINAL DEVELOPED CONDITIONS

| TO POND | BYPASS |
| :---: | :---: |
| ONSITE: | ONSITE |
| $A=0.92$ ACRES | $A=0.03$ ACRES |
| $\mathrm{CN}=68$ | $\mathrm{CN}=68$ |
| $\mathrm{A}=1.87$ ACRES | $A=0.02$ ACRES |
| $\mathrm{CN}=98$ | $\mathrm{CN}=98$ |
| OFFSITE: | OFFSITE |
| $A=3.61$ ACRES | $A=0.01$ ACRES |
| $\mathrm{CN}=68$ | $\mathrm{CN}=68$ |
| $A=0.42$ ACRES | COMBINED |
| $\mathrm{CN}=98$ | $A=0.06$ ACRES |
| COMBINED: | $\mathrm{CN}=78$ |
| $A=6.82$ $C N=78$ | (SCS METHOD) |
| (RESERVOIR ROUTING) |  |

FINAL DEVELOPED
CONDITIONS
TWISTED $52^{\circ}$

## Final Developed Conditions:

Onsite to Pond:
$\mathrm{A}=0.92$ acres
$\mathrm{CN}=68$
$\mathrm{A}=1.87$ acres
$\mathrm{CN}=98$
Offsite to Pond:
$\mathrm{A}=3.61$ acres
$\mathrm{CN}=68$
$\mathrm{A}=0.42$ acres
$\mathrm{CN}=98$
Combined:
$\mathrm{A}=6.82$ acres
$\mathrm{CN}=78$
(Reservoir Routing)
Onsite Bypass:
$\mathrm{A}=0.03$ acres
$\mathrm{CN}=68$
$\mathrm{A}=0.02$ acres
$\mathrm{CN}=98$
Offsite Bypass:
$\mathrm{A}=0.01$ acres
$\mathrm{CN}=68$
Combined:
$\mathrm{A}=0.06$ acres
$\mathrm{CN}=78$
(SCS Method)

|  | Predeveloped Runoff | Final Developed Runoff |
| :---: | :---: | :---: |
| Year | CFS | CFS |
| 1 | 1.73 | 3.08 |
| 2 | 3.19 | 3.59 |
| 5 | 5.88 | 6.31 |
| 10 | 8.37 | 9.22 |
| 25 | 12.11 | 13.33 |
| 100 | 18.93 | 20.28 |



EXISTING CONDITIONS
TWISTED $52^{\circ}$

## Methodology:

Hydrologic and hydraulic routing calculations were performed using Hydraflow Hydrographs software. Time of concentrations were calculated using the TR-55 segmented approach. Hydrologic calculations utilized the NRCS SCS Method within the Hydraflow software. The stormwater dry detention pond was routed within the Hydraflow software, which uses the Storage Indication Method.

## Conclusions:

Through the use of the existing stormwater dry detention pond, this project detains the $100-\mathrm{yr}$ storm onsite.

## List of Appendices:

- Maps
- Pre-Developed Drainage Area Map
- Post-Developed Drainage Area Map
- Soils Survey Map
- FEMA Firmette
- USGS Map
- Calculations
- Table of Subareas, Land Uses, \& Curve Numbers
- Peak Flow Summary
- Hydraflow Routing Report

NOAA Atlas 14, Volume 2, Version 3
Location name: Zebulon, North Carolina, USA*
Latitude: $35.8383^{\circ}$, Longitude: $-78.3233^{\circ}$
Elevation: $335 \mathrm{ft}^{* *}$

* source: ESRI Maps
** source: USGS


## POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland
PF tabular | PF graphical | Maps \& aerials

## PF tabular

| PDS-based point precipitation frequency estimates with $90 \%$ confidence intervals (in inches) ${ }^{1}$ |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Duration | Average recurrence interval (years) |  |  |  |  |  |  |  |  |  |
|  | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | $\begin{gathered} 0.405 \\ (0.369-0.444) \end{gathered}$ | 0.468 <br> $(0.428-0.512)$ | 0.530 <br> $(0.485-0.580)$ | 0.601 <br> $(0.548-0.657)$ <br> 0.561 | $\begin{gathered} \mathbf{0 . 6 7 0} \\ (0.609-0.732) \end{gathered}$ | 0.728 <br> $(0.658-0.795)$ | $\mathbf{0 . 7 8 0}$ <br> $(0.701-0.851)$ | $\begin{gathered} 0.827 \\ (0.739-0.904) \end{gathered}$ | $\begin{gathered} \hline 0.880 \\ (0.780-0.963) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 0.930 \\ (0.818-1.02) \\ \hline \end{array}$ |
| 10-min | $\begin{gathered} \mathbf{0 . 6 4 6} \\ (0.590-0.710) \end{gathered}$ | $\begin{gathered} \mathbf{0 . 7 4 8} \\ (0.685-0.819) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 8 4 9} \\ (0.777-0.929) \\ \hline \end{gathered}$ | 0.961 <br> $(0.877-1.05)$ | $\begin{gathered} 1.07 \\ (0.970-1.17) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.16 \\ (1.05-1.27) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{1 . 2 4} \\ (1.11-1.35) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.31 \\ (1.17-1.43) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.39 \\ (1.23-1.52) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline 1.46 \\ (1.29-1.61) \\ \hline \end{gathered}$ |
| 15-min | $\begin{gathered} \mathbf{0 . 8 0 8} \\ (0.737-0.887) \end{gathered}$ | (0.861-1.03) | $(0.983-1.18)$ | $\begin{gathered} \hline 1.22 \\ (1.11-1.33) \\ \hline \end{gathered}$ |  | $\begin{gathered} \hline 1.47 \\ (1.33-1.60) \\ \hline \end{gathered}$ |  |  |  | $\begin{gathered} \hline 1.84 \\ (1.62-2.02) \\ \hline \end{gathered}$ |
| 30-m | $\begin{gathered} \hline 1.11 \\ (1.01-1.22) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.30 \\ (1.19-1.42) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.53 \\ (1.40-1.67) \\ \hline \end{gathered}$ | $\begin{gathered} 1.76 \\ (1.61-1.92) \end{gathered}$ | $\begin{gathered} \hline 2.00 \\ (1.82-2.19) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 2 1} \\ (2.00-2.42) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 4 0} \\ (2.16-2.62) \end{gathered}$ | $\begin{array}{c\|} \hline 2.57 \\ (2.30-2.81) \\ \hline \end{array}$ | $\begin{gathered} 2.79 \\ (2.47-3.05) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2.98 \\ (2.62-3.27) \\ \hline \end{gathered}$ |
| 60-min | $\begin{gathered} 1.38 \\ (1.26-1.52) \end{gathered}$ | $\begin{gathered} \hline \hline 1.63 \\ (1.49-1.78) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.96 \\ (1.79-2.14) \\ \hline \end{gathered}$ | $\begin{gathered} 2.29 \\ (2.09-2.51) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 6 7} \\ (2.42-2.92) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.00 \\ (2.71-3.27) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.30 \\ (2.97-3.60) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 3.61 \\ (3.23-3.95) \\ \hline \end{array}$ | $\begin{gathered} \hline 4.00 \\ (3.55-4.38) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline 4.35 \\ (3.82-4.77) \end{gathered}$ |
| 2-hr | $\begin{gathered} \hline 1.62 \\ (1.46-1.79) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.91 \\ (1.74-2.10) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 3 2} \\ (2.11-2.56) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 7 6} \\ (2.50-3.03) \\ \hline \end{gathered}$ |  |  |  | $\begin{gathered} \hline 4.66 \\ (4.13-5.10) \\ \hline \end{gathered}$ | $\begin{gathered} 5.29 \\ (4.63-5.79) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.86 \\ (5.09-6.44) \\ \hline \end{gathered}$ |
| 3-hr | $\begin{gathered} \hline 1.71 \\ (1.55-1.90) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{2 . 0 3} \\ (1.85-2.24) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 4 7} \\ (2.25-2.74) \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{2 . 9 6} \\ (2.68-3.26) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.54 \\ (3.18-3.90) \\ \hline \end{gathered}$ |  |  |  | $\begin{gathered} \hline 5.97 \\ (5.20-6.57) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.71 \\ (5.77-7.39) \\ \hline \end{gathered}$ |
| 6-hr | $\begin{gathered} 2.05 \\ (1.87-2.27) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 4 3} \\ (2.22-2.68) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 9 7} \\ (2.70-3.27) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.55 \\ (3.23-3.91) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.27 \\ (3.85-4.68) \end{gathered}$ | $\begin{gathered} \hline 4.94 \\ (4.43-5.41) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.62 \\ (4.99-6.14) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.34 \\ (5.58-6.93) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.34 \\ (6.37-8.02) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.28 \\ (7.10-9.07) \\ \hline \end{gathered}$ |
| 12-h | $\begin{gathered} \hline \mathbf{2 . 4 1} \\ (2.20-2.66) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 8 6} \\ (2.62-3.15) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.51 \\ (3.21-3.86) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline 4.22 \\ (3.84-4.64) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.11 \\ (4.62-5.60) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{5 . 9 6} \\ (5.34-6.50) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.82 \\ (6.05-7.43) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 7.76 \\ (6.80-8.44) \\ \hline \end{array}$ | $\begin{gathered} \hline 9.06 \\ (7.82-9.86) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 10.3 \\ (8.77-11.2) \\ \hline \end{gathered}$ |
| 24-hr | $\begin{gathered} \hline \mathbf{2 . 8 5} \\ (2.65-3.09) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.46 \\ (3.21-3.74) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.38 \\ (4.06-4.74) \\ \hline \end{gathered}$ | $\begin{gathered} 5.14 \\ (4.75-5.55) \end{gathered}$ | $\begin{gathered} \hline \mathbf{6 . 2 0} \\ (5.71-6.69) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.07 \\ (6.48-7.64) \end{gathered}$ | $\begin{gathered} \hline 8.00 \\ (7.29-8.64) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 8.99 \\ (8.14-9.73) \\ \hline \end{array}$ | $\begin{gathered} \hline \mathbf{1 0 . 4} \\ (9.34-11.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 11.6 \\ (10.3-12.6) \\ \hline \end{gathered}$ |
| 2-day | $\begin{gathered} \hline 3.30 \\ (3.07-3.56) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.98 \\ (3.71-4.30) \\ \hline \end{gathered}$ | $\begin{gathered} 5.02 \\ (4.66-5.41) \end{gathered}$ | $\begin{gathered} 5.85 \\ (5.42-6.31) \end{gathered}$ | $\begin{gathered} 7.02 \\ (6.47-7.58) \end{gathered}$ | $\begin{gathered} \hline 7.98 \\ (7.32-8.61) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.99 \\ (8.21-9.71) \\ \hline \end{gathered}$ | (9.13-10.9) | (10.4-12.6) | $\begin{gathered} \hline 12.9 \\ (11.4-14.0) \\ \hline \end{gathered}$ |
| 3-day | $\begin{gathered} \hline 3.51 \\ (3.27-3.77) \end{gathered}$ | $\begin{gathered} \hline 4.22 \\ (3.93-4.54) \end{gathered}$ | $\begin{gathered} 5.29 \\ (4.92-5.68) \end{gathered}$ | $\begin{gathered} \hline \mathbf{6 . 1 4} \\ (5.70-6.60) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.35 \\ (6.79-7.90) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.33 \\ (7.66-8.96) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 9.36 \\ (8.57-10.1) \end{gathered}$ | $\begin{array}{c\|} \hline 10.5 \\ (9.51-11.3) \\ \hline \end{array}$ | $\begin{gathered} \hline 12.0 \\ (10.8-13.0) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 13.3 \\ (11.9-14.4) \\ \hline \end{gathered}$ |
| 4- | $\begin{gathered} \hline 3.71 \\ (3.46-3.98) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.46 \\ (4.16-4.78) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.56 \\ (5.18-5.95) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{6 . 4 4} \\ (5.99-6.89) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.67 \\ (7.11-8.22) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.68 \\ (8.00-9.30) \\ \hline \end{gathered}$ | $\begin{gathered} 9.73 \\ (8.93-10.4) \\ \hline \end{gathered}$ | $\begin{gathered} 10.8 \\ (9.89-11.7) \\ \hline \end{gathered}$ | $\begin{gathered} 12.4 \\ (11.2-13.4) \end{gathered}$ | $\begin{array}{c\|} \hline 13.7 \\ (12.3-14.8) \\ \hline \end{array}$ |
| 7-day | $\begin{gathered} \hline 4.31 \\ (4.03-4.61) \end{gathered}$ | $\begin{gathered} \hline \mathbf{5 . 1 6} \\ (4.82-5.52) \end{gathered}$ | $\begin{gathered} \hline 6.35 \\ (5.93-6.80) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.31 \\ (6.81-7.82) \end{gathered}$ | $\begin{gathered} \hline 8.64 \\ (8.02-9.24) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 9.71 \\ (8.99-10.4) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 10.8 \\ (9.97-11.6) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 12.0 \\ (11.0-12.9) \end{gathered}$ | $\begin{gathered} \hline 13.6 \\ (12.4-14.7) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{1 5 . 0} \\ (13.5-16.2) \\ \hline \end{gathered}$ |
| 10-day | $\begin{gathered} \hline 4.92 \\ (4.61-5.25) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.87 \\ (5.50-6.26) \\ \hline \end{gathered}$ | $\begin{gathered} 7.12 \\ (6.67-7.59) \end{gathered}$ | $\begin{gathered} \hline 8.11 \\ (7.59-8.64) \\ \hline \end{gathered}$ | $\begin{gathered} 9.47 \\ (8.83-10.1) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 10.6 \\ (9.81-11.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 11.7 \\ (10.8-12.5) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 12.8 \\ (11.8-13.7) \\ \hline \end{gathered}$ | $\begin{gathered} 14.4 \\ (13.2-15.5) \end{gathered}$ | $\begin{array}{c\|} \hline 15.7 \\ (14.3-16.9) \\ \hline \end{array}$ |
| 20-c | $\begin{gathered} \hline \mathbf{6 . 6 0} \\ (6.21-7.03) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.82 \\ (7.36-8.32) \\ \hline \end{gathered}$ | $\begin{gathered} 9.33 \\ (8.77-9.93) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 10.5 \\ (9.88-11.2) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 12.2 \\ (11.4-12.9) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 13.5 \\ (12.6-14.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 14.8 \\ (13.8-15.8) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 16.2 \\ (15.0-17.2) \\ \hline \end{array}$ | $\begin{gathered} \hline \mathbf{1 8 . 0} \\ (16.6-19.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline 19.5 \\ (17.8-20.9) \\ \hline \end{gathered}$ |
| 30-day | $\begin{gathered} \hline 8.20 \\ (7.74-8.70) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 9.67 \\ (9.13-10.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 11.4 \\ (10.7-12.0) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline 12.7 \\ (11.9-13.4) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{1 4 . 4} \\ (13.5-15.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline 15.8 \\ (14.8-16.8) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{1 7 . 1} \\ (16.0-18.2) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{1 8 . 5} \\ (17.2-19.7) \\ \hline \end{array}$ | $\begin{gathered} \hline \mathbf{2 0 . 4} \\ (18.9-21.7) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{2 1 . 8} \\ (20.1-23.3) \\ \hline \end{gathered}$ |
| 45-day | $\begin{gathered} \hline 10.4 \\ (9.90-11.0) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 12.3 \\ (11.6-12.9) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 14.2 \\ (13.4-14.9) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{1 5 . 6} \\ (14.8-16.5) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{1 7 . 6} \\ (16.6-18.6) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 19.0 \\ (18.0-20.1) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 0 . 5} \\ (19.3-21.7) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{2 2 . 0} \\ (20.6-23.3) \\ \hline \end{array}$ | $\begin{gathered} \hline \mathbf{2 3 . 9} \\ (22.3-25.4) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 5 . 4} \\ (23.6-27.0) \\ \hline \end{gathered}$ |
| 60-day | $\begin{gathered} \hline 12.5 \\ (11.9-13.2) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 14.7 \\ (13.9-15.4) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 16.7 \\ (15.9-17.6) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{1 8 . 4} \\ (17.4-19.3) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 20.4 \\ (19.4-21.5) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 2 . 0} \\ (20.8-23.2) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 3 . 6} \\ (22.2-24.9) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{2 5 . 1} \\ (23.6-26.5) \end{array}$ | $\begin{gathered} \hline \mathbf{2 7 . 1} \\ (25.4-28.7) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{2 8 . 6} \\ (26.7-30.4) \\ \hline \end{array}$ |

[^0]Back to Top
PF graphical

NOAA Atlas 14, Volume 2, Version 3
Location name: Zebulon, North Carolina, USA*
Latitude: $35.8383^{\circ}$, Longitude: $-78.3233^{\circ}$
Elevation: $335 \mathrm{ft}^{* *}$

* source: ESRI Maps
** source: USGS


## POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland
PF tabular | PF graphical | Maps \& aerials

## PF tabular

| PDS-based point precipitation frequency estimates with $\mathbf{9 0 \%}$ confidence intervals (in inches/hour) ${ }^{\mathbf{1}}$ |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Duration | Average recurrence interval (years) |  |  |  |  |  |  |  |  |  |
|  | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 50 | 1000 |
| 5-m | $\begin{gathered} \hline 4.86 \\ (4.43-5.33) \end{gathered}$ | $\begin{gathered} \hline \hline 5.62 \\ (5.14-6.14) \end{gathered}$ | $\begin{gathered} \hline 6.36 \\ (5.82-6.96) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 7.21 \\ (6.58-7.88) \end{gathered}$ |  | $\begin{gathered} 8.74 \\ (7.90-9.54) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 9.36 \\ (8.41-10.2) \\ \hline \end{gathered}$ | $\begin{gathered} 9.92 \\ (8.87-10.8) \\ \hline \end{gathered}$ | $\begin{gathered} 10.6 \\ (9.36-11.6) \\ \hline \end{gathered}$ | $\begin{gathered} 11.2 \\ (9.82-12.3) \end{gathered}$ |
| 10-1 | $3.88$ | $4.49$ | $\begin{gathered} \hline 5.09 \\ (4.66-5.57) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 5.77 \\ (5.26-6.30) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6.41 \\ (5.82-7.00) \\ \hline \end{gathered}$ |  | $\begin{gathered} \hline 7.43 \\ (6.68-8.11) \\ \hline \end{gathered}$ | $6$ | $\begin{gathered} \hline 8.35 \\ (7.40-9.14) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.79 \\ (7.73-9.64) \\ \hline \end{gathered}$ |
| 15 | $\begin{gathered} 3.23 \\ (2.95-3.55 \end{gathered}$ | (3.44-4.12) |  | $(4.44-5.32)$ |  |  |  |  |  | $\begin{gathered} \hline 7.36 \\ (6.46-8.07) \end{gathered}$ |
| 30- | $\begin{gathered} \hline \mathbf{2 . 2 2} \\ (2.02-2.43) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 6 0} \\ (2.38-2.84) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.05 \\ (2.79-3.34) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{3 . 5 2} \\ (3.21-3.85) \\ \hline \end{gathered}$ |  |  |  | $\begin{gathered} \hline 5.15 \\ (4.60-5.63) \\ \hline \end{gathered}$ |  | $\begin{gathered} \hline 5.96 \\ (5.23-6.53) \\ \hline \end{gathered}$ |
| 60-m | $\begin{gathered} \hline 1.38 \\ (1.26-1.52) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.63 \\ (1.49-1.78) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.96 \\ (1.79-2.14) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{2 . 2 9} \\ (2.09-2.51) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{2 . 6 7} \\ (2.42-2.92) \\ \hline \end{gathered}$ | $\begin{aligned} & \hline \hline 3.00 \\ & 71-3.27) \\ & \hline \hline \end{aligned}$ | $\begin{gathered} \hline 3.30 \\ (2.97-3.60) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.61 \\ (3.23-3.95) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.00 \\ (3.55-4.38) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \hline 4.35 \\ (3.82-4.77) \\ \hline \end{array}$ |
| 2- | $\begin{gathered} \mathbf{0 . 8 0 7} \\ (0.731-0.894 \end{gathered}$ | $\begin{gathered} 0.956 \\ (0.870-1.05) \end{gathered}$ |  |  |  |  |  |  |  | $\begin{gathered} \hline 2.93 \\ (2.54-3.22) \\ \hline \end{gathered}$ |
| 3-hr | 0.569 <br> $(0.516-0.633)$ | $(0.615-0.746)$ | $(0.748-0.911)$ | (0.892-1.09) | $\begin{gathered} \hline 1.18 \\ (1.06-1.30) \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline 1.73 \\ (1.52-1.90) \\ \hline \end{gathered}$ |  | $\begin{gathered} 2.23 \\ (1.92-2.46) \end{gathered}$ |
| 6-hr | (0.311-0.379) | 0.405 <br> $(0.370-0.447)$ | 0.495 <br> $(0.451-0.545)$ | 0.593 <br> $(0.538-0.652)$ | 0.712 <br> $(0.643-0.781)$ | 0.825 <br> $(0.739-0.903)$ | $\begin{gathered} \mathbf{0 . 9 3 8} \\ (0.833-1.03) \end{gathered}$ | $\begin{gathered} \hline 1.06 \\ (0.931-1.16) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.22 \\ (1.06-1.34) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.38 \\ (1.18-1.51) \\ \hline \end{gathered}$ |
| 12 | $\begin{gathered} \hline \mathbf{0 . 2 0 0} \\ (0.182-0.220) \end{gathered}$ | $\begin{gathered} 0.237 \\ (0.217-0.261) \end{gathered}$ | $\begin{gathered} \mathbf{0 . 2 9 1} \\ (0.266-0.320) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 3 5 0} \\ (0.319-0.385) \\ \hline \end{gathered}$ | $\\|(0.383-0.464)$ | $\begin{gathered} \mathbf{0 . 4 9 4} \\ (0.443-0.539) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{0 . 5 6 5} \\ (0.502-0.616) \\ \hline \end{array}$ | $\begin{gathered} \hline 0.643 \\ (0.564-0.700) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{0 . 7 5 1} \\ (0.649-0.818) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{0 . 8 5 6} \\ (0.727-0.932) \\ \hline \end{gathered}$ |
| 24-hr | $\begin{array}{c\|} \hline \mathbf{0 . 1 1 8} \\ (0.110-0.128) \\ \hline \end{array}$ | $(0.133-0.155)$ | (0.169-0.197) | $(0.197-0.231)$ | $(0.237-0.278)$ | $(0.270-0.318)$ | $(0.303-0.360)$ | $(0.339-0.405)$ | $(0.389-0.470)$ | (0.429-0.525) |
| 2-day | $(0.063-0.074)$ | $\mid(0.077-0.089)$ | $(0.097-0.112)$ | $(0.112-0.131)$ | $\\|(0.134-0.157)\\|$ | $(0.152-0.179)$ | $(0.170-0.202)$ | $(0.190-0.226)$ | $(0.217-0.262)$ | $\begin{gathered} \hline \mathbf{0 . 2 6 7} \\ (0.238-0.292) \end{gathered}$ |
| 3-day | $(0.045-0.052)$ | 0.058 <br> $(0.054-0.063)$ | $\begin{gathered} \mathbf{0 . 0 7 3} \\ (0.068-0.078) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{0 . 0 8 5} \\ (0.079-0.091) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{0 . 1 0 2} \\ (0.094-0.109) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 1 1 5} \\ (0.106-0.124) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 1 2 9} \\ (0.119-0.140) \\ \hline \end{gathered}$ | $\left\lvert\, \begin{gathered} 0.145 \\ (0.132-0.156) \\ \hline \end{gathered}\right.$ | $(0.150-0.180)$ | $\begin{gathered} \hline \mathbf{0 . 1 8 4} \\ (0.164-0.200) \\ \hline \end{gathered}$ |
| 4-day | $(0.036-0.041)$ | $(0.043-0.049)$ | $(0.053-0.062)$ | 0.067 <br> $(0.062-0.071)$ | $(0.074-0.085)$ | $\begin{gathered} \mathbf{0 . 0 9 0} \\ (0.083-0.096) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{0 . 1 0 1} \\ (0.092-0.108) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 1 1 2} \\ (0.102-0.121) \\ \hline \end{gathered}$ | $\|(0.116-0.139)\|$ | $\begin{array}{\|c\|} \hline \mathbf{0 . 1 4 2} \\ (0.127-0.154) \\ \hline \end{array}$ |
| 7-day | $\begin{gathered} 0.025 \\ (0.023-0.027) \end{gathered}$ | $\begin{gathered} \mathbf{0 . 0 3 0} \\ (0.028-0.032) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \mathbf{0 . 0 3 7} \\ (0.035-0.040) \\ \hline \end{array}$ | $\begin{array}{c\|} \mathbf{0 . 0 4 3} \\ (0.040-0.046) \\ \hline \end{array}$ | $\begin{array}{\|c\|} \mathbf{0 . 0 5 1} \\ (0.047-0.055) \end{array}$ | $\begin{gathered} 0.057 \\ (0.053-0.061) \end{gathered}$ | $\begin{array}{\|c\|} \hline 0.064 \\ (0.059-0.069) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{0 . 0 7 1} \\ (0.065-0.076) \\ \hline \end{gathered}$ | $(0.073-0.087)$ | $(0.080-0.096)$ |
| 10-day | $(0.019-0.021)$ | $(0.022-0.026)$ | $\begin{array}{c\|} \mathbf{0 . 0 2 9} \\ (0.027-0.031) \\ \hline \end{array}$ | $(0.031-0.036)$ | $(0.036-0.042)$ | $(0.040-0.046)$ | $(0.045-0.051)$ | $(0.049-0.057)$ | $(0.055-0.064)$ | $\begin{gathered} \mathbf{0 . 0 6 5} \\ (0.059-0.070) \\ \hline \end{gathered}$ |
| 20-day | $\begin{gathered} 0.013 \\ (0.012-0.014) \\ \hline \end{gathered}$ | $\begin{gathered} 0.016 \\ (0.015-0.017) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \mathbf{0 . 0 1 9} \\ (0.018-0.020) \\ \hline \end{array}$ | $\begin{array}{c\|} \mathbf{0 . 0 2 1} \\ (0.020-0.023) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{0 . 0 2 5} \\ (0.023-0.026) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \mathbf{0 . 0 2 8} \\ (0.026-0.029) \\ \hline \end{array}$ | $\begin{array}{\|c\|} \hline \mathbf{0 . 0 3 0} \\ (0.028-0.032) \\ \hline \end{array}$ | $\begin{array}{c\|} 0.033 \\ (0.031-0.035) \\ \hline \end{array}$ | $(0.034-0.040)$ | $\begin{array}{\|c\|} \mathbf{0 . 0 4 0} \\ (0.037-0.043) \\ \hline \end{array}$ |
| 30-day | $\left\lvert\, \begin{gathered} 0.011 \\ (0.010-0.012) \\ \hline \end{gathered}\right.$ | $\begin{gathered} 0.013 \\ (0.012-0.014) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \mathbf{0 . 0 1 5} \\ (0.014-0.016) \\ \hline \end{array}$ | $\begin{array}{c\|} \mathbf{0 . 0 1 7} \\ (0.016-0.018) \\ \hline \end{array}$ | $\begin{gathered} 0.020 \\ (0.018-0.021) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 0 2 1} \\ (0.020-0.023) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 0 2 3} \\ (0.022-0.025) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \mathbf{0 . 0 2 5} \\ (0.023-0.027) \\ \hline \end{gathered}$ | $(0.026-0.030)$ | $\begin{gathered} \mathbf{0 . 0 3 0} \\ (0.027-0.032) \\ \hline \end{gathered}$ |
| 45-day | $\left\lvert\, \begin{gathered} 0.009 \\ (0.009-0.010) \end{gathered}\right.$ | $(0.010-0.011)$ | $\begin{array}{c\|} \mathbf{0 . 0 1 3} \\ (0.012-0.013) \\ \hline \end{array}$ | $\begin{array}{c\|} \mathbf{0 . 0 1 4} \\ (0.013-0.015) \\ \hline \end{array}$ | $(0.015-0.017)$ | $(0.016-0.018)$ | $(0.017-0.020)$ | $\begin{gathered} \mathbf{0 . 0 2 0} \\ (0.019-0.021) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \mathbf{0 . 0 2 2} \\ (0.020-0.023) \\ \hline \end{array}$ | $\begin{array}{c\|} \hline \mathbf{0 . 0 2 3} \\ (0.021-0.025) \\ \hline \end{array}$ |
| 60-day | $\begin{gathered} 0.008 \\ (0.008-0.009) \end{gathered}$ | $\left\lvert\, \begin{gathered} \mathbf{0 . 0 1 0} \\ (0.009-0.010) \end{gathered}\right.$ | $\begin{array}{\|c\|} \mathbf{0 . 0 1 1} \\ (0.011-0.012) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{0 . 0 1 2} \\ (0.012-0.013) \\ \hline \end{gathered}$ | $\begin{gathered} 0.014 \\ (0.013-0.014) \end{gathered}$ | $\begin{gathered} 0.015 \\ (0.014-0.016) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 0 1 6} \\ (0.015-0.017) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 0 1 7} \\ (0.016-0.018) \end{gathered}$ | $\begin{gathered} 0.018 \\ (0.017-0.019) \\ \hline \end{gathered}$ | 0.019 <br> $(0.018-0.021)$ |

[^1]

## Hydrograph Report

## Hyd. No. 1

Pre-Developed Total

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=1.729 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=1 \mathrm{yrs}$ | Time to peak | $=744 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=12,674 \mathrm{cuft}$ |
| Drainage area | $=6.880$ ac | Curve number | $=67$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=$ User | Time of conc. (Tc) | $=45.00 \mathrm{~min}$ |
| Total precip. | $=2.85 \mathrm{in}$ | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

## Hydrograph Report

## Hyd. No. 3

Post Development (To Pond)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=4.437 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=1 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=25,107 \mathrm{cuft}$ |
| Drainage area | $=6.820 \mathrm{ac}$ | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=2.85 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |



## Hydrograph Report

## Hyd. No. 4

Post Development (Bypass)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=0.039 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=1 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=221 \mathrm{cuft}$ |
| Drainage area | $=0.060$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=$ User | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=2.85 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Post Development (Bypass)


## Hydrograph Report

## Hyd. No. 6

Pond Route

| Hydrograph type | $=$ Reservoir | Peak discharge | $=3.053 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=1 \mathrm{yrs}$ | Time to peak | $=764 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=25,107 \mathrm{cuft}$ |
| Inflow hyd. No. | $=3-$ Post Development (To Pon\&Jax. Elevation | $=33.78 \mathrm{ft}$ |  |
| Reservoir name | $=$ Dry Detention Pond | Max. Storage | $=2,580 \mathrm{cuft}$ |

Storage Indication method used.


## Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024
Tuesday, 01 / 9 / 2024

## Pond No. 1 - Dry Detention Pond

Pond Data
Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation $=31.70 \mathrm{ft}$
Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |
| :---: | :---: | :---: | :---: | :---: |
| 0.00 | 31.70 | 00 | 0 |  |
| 1.30 | 33.00 | 338 | 220 | 0 |
| 1.80 | 33.50 | 3,378 | 929 | 220 |
| 2.30 | 34.00 | 6,755 | 2,533 | 1,149 |
| 2.80 | 34.50 | 9,093 | 3,962 | 3,682 |
| 3.30 | 35.00 | 11,386 | 5,120 | 1,644 |
| 3.80 | 36.00 | 13,208 | 5,915 | 18,679 |
| 4.30 |  | 6,371 | 25,050 |  |

## Culvert / Orifice Structures

|  | [A] | [B] | [C] | [PrfRsr] |  | [A] | [B] | [C] | [D] |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rise (in) | $=24.00$ | 10.00 | 0.00 | 0.00 | Crest Len (ft) | $=3.50$ | 0.00 | 0.00 | 0.00 |
| Span (in) | = 24.00 | 10.00 | 0.00 | 0.00 | Crest El. (ft) | = 34.50 | 0.00 | 0.00 | 0.00 |
| No. Barrels | = 1 | 1 | 0 | 0 | Weir Coeff. | = 3.33 | 3.33 | 3.33 | 3.33 |
| Invert El. (ft) | = 31.70 | 31.70 | 0.00 | 0.00 | Weir Type | $=$ Rect | --- | --- | --- |
| Length (ft) | $=0.00$ | 0.00 | 0.00 | 0.00 | Multi-Stage | = Yes | No | No | No |
| Slope (\%) | $=0.00$ | 0.00 | 0.00 | n/a |  |  |  |  |  |
| N -Value | $=.013$ | . 013 | . 013 | n/a |  |  |  |  |  |
| Orifice Coeff. | $=0.60$ | 0.60 | 0.60 | 0.60 | Exfil.(in/hr) | $=0.000$ | ontour) |  |  |
| Multi-Stage | = n/a | Yes | No | No | TW Elev. (ft) | $=0.00$ |  |  |  |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).


## Hydrograph Report

## Hyd. No. 8

Post Development (Detained)

| Hydrograph type | $=$ Combine | Peak discharge | $=3.080 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=1 \mathrm{yrs}$ | Time to peak | $=762 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=25,327 \mathrm{cuft}$ |
| Inflow hyds. | $=4,6$ | Contrib. drain. area | $=0.060 \mathrm{ac}$ |




## Hydrograph Report

## Hyd. No. 1

Pre-Developed Total

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=3.193 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=2 \mathrm{yrs}$ | Time to peak | $=744 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=20,487 \mathrm{cuft}$ |
| Drainage area | $=6.880 \mathrm{ac}$ | Curve number | $=67$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=3.46 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Pre-Developed Total
Hyd. No. 1 -- 2 Year


Hyd No. 1

Q (cfs)

Time (min)

## Hydrograph Report

## Hyd. No. 3

Post Development (To Pond)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=6.548 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=2 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=35,995 \mathrm{cuft}$ |
| Drainage area | $=6.820 \mathrm{ac}$ | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=3.46 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |



## Hydrograph Report

## Hyd. No. 4

Post Development (Bypass)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=0.058 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=2 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=317 \mathrm{cuft}$ |
| Drainage area | $=0.060$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=$ User | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=3.46 \mathrm{in}$ | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Post Development (Bypass)


## Hydrograph Report

## Hyd. No. 6

Pond Route

| Hydrograph type | $=$ Reservoir | Peak discharge | $=3.560 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=2 \mathrm{yrs}$ | Time to peak | $=770 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=35,994 \mathrm{cuft}$ |
| Inflow hyd. No. | $=3-$ Post Development (To Pondlax. Elevation | $=34.33 \mathrm{ft}$ |  |
| Reservoir name | $=$ Dry Detention Pond | Max. Storage | $=6,321 \mathrm{cuft}$ |

Storage Indication method used.

## Hydrograph Report

## Hyd. No. 8

Post Development (Detained)

| Hydrograph type | $=$ Combine | Peak discharge | $=3.592 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=2 \mathrm{yrs}$ | Time to peak | $=768 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=36,311 \mathrm{cuft}$ |
| Inflow hyds. | $=4,6$ | Contrib. drain. area | $=0.060 \mathrm{ac}$ |




## Hydrograph Report

## Hyd. No. 1

Pre-Developed Total

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=5.879 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=5 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=34,286 \mathrm{cuft}$ |
| Drainage area | $=6.880 \mathrm{ac}$ | Curve number | $=67$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=U s e r$ | Time of conc. $($ Tc $)$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=4.38 \mathrm{in}$ | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |



## Hydrograph Report

## Hyd. No. 3

Post Development (To Pond)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=9.972 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=5 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=53,834 \mathrm{cuft}$ |
| Drainage area | $=6.820 \mathrm{ac}$ | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=4.38 \mathrm{in}$ | Distribution | $=7 y p e \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

## Hydrograph Report

## Hyd. No. 4

Post Development (Bypass)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=0.088 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=5 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=474 \mathrm{cuft}$ |
| Drainage area | $=0.060$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=$ User | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=4.38 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Post Development (Bypass)


## Hydrograph Report

## Hyd. No. 6

Pond Route

| Hydrograph type | $=$ Reservoir | Peak discharge | $=6.255 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=5 \mathrm{yrs}$ | Time to peak | $=764 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=53,833 \mathrm{cuft}$ |
| Inflow hyd. No. | $=3-$ Post Development (To Pon\&Jax. Elevation | $=34.86 \mathrm{ft}$ |  |
| Reservoir name | $=$ | Dry Detention Pond | Max. Storage |

Storage Indication method used.


## Hydrograph Report

## Hyd. No. 8

Post Development (Detained)

| Hydrograph type | $=$ Combine | Peak discharge | $=6.311 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=5 \mathrm{yrs}$ | Time to peak | $=764 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=54,307 \mathrm{cuft}$ |
| Inflow hyds. | $=4,6$ | Contrib. drain. area | $=0.060 \mathrm{ac}$ |




## Hydrograph Report

## Hyd. No. 1

Pre-Developed Total

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=8.367 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=10 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=47,057 \mathrm{cuft}$ |
| Drainage area | $=6.880 \mathrm{ac}$ | Curve number | $=67$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. (Tc) | $=45.00 \mathrm{~min}$ |
| Total precip. | $=5.14 \mathrm{in}$ | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Pre-Developed Total
Q (cfs) Hyd. No. 1 -- 10 Year


## Hydrograph Report

## Hyd. No. 3

Post Development (To Pond)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=12.94 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=10 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=69,459 \mathrm{cuft}$ |
| Drainage area | $=6.820 \mathrm{ac}$ | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=5.14 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

## Hydrograph Report

## Hyd. No. 4

Post Development (Bypass)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=0.114 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=10$ yrs | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=611 \mathrm{cuft}$ |
| Drainage area | $=0.060$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $==$ User | Time of conc. (Tc) | $=45.00 \mathrm{~min}$ |
| Total precip. | $=5.14$ in | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Post Development (Bypass)

| Q (cfs) |
| :--- |
| 0.50 |
| 0.5 |${ }^{2}$

## Hydrograph Report

## Hyd. No. 6

Pond Route

| Hydrograph type | $=$ Reservoir | Peak discharge | $=9.139 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=10 \mathrm{yrs}$ | Time to peak | $=760 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=69,458 \mathrm{cuft}$ |
| Inflow hyd. No. | $=3-$ Post Development (To Pon\&Jax. Elevation | $=35.10 \mathrm{ft}$ |  |
| Reservoir name | $=$ Dry Detention Pond | Max. Storage | $=13,928 \mathrm{cuft}$ |

Storage Indication method used.


## Hydrograph Report

## Hyd. No. 8

Post Development (Detained)

| Hydrograph type | $=$ Combine | Peak discharge | $=9.220 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=10 \mathrm{yrs}$ | Time to peak | $=760 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=70,070 \mathrm{cuft}$ |
| Inflow hyds. | $=4,6$ | Contrib. drain. area | $=0.060 \mathrm{ac}$ |




## Hydrograph Report

## Hyd. No. 1

Pre-Developed Total

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=12.11 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=25 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=66,381 \mathrm{cuft}$ |
| Drainage area | $=6.880 \mathrm{ac}$ | Curve number | $=67$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=U s e r$ | Time of conc. (Tc) | $=45.00 \mathrm{~min}$ |
| Total precip. | $=6.20$ in | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Pre-Developed Total


## Hydrograph Report

## Hyd. No. 3

Post Development (To Pond)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=17.20 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=25 \mathrm{yrs}$ | Time to peak | $=740 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=92,159 \mathrm{cuft}$ |
| Drainage area | $=6.820 \mathrm{ac}$ | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=6.20 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

## Hydrograph Report

## Hyd. No. 4

Post Development (Bypass)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=0.151 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=25$ yrs | Time to peak | $=740 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=811 \mathrm{cuft}$ |
| Drainage area | $=0.060$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=$ User | Time of conc. (Tc) | $=45.00 \mathrm{~min}$ |
| Total precip. | $=6.20$ in | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Post Development (Bypass)


## Hydrograph Report

## Hyd. No. 6

Pond Route

| Hydrograph type | $=$ Reservoir | Peak discharge | $=13.21 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=25 \mathrm{yrs}$ | Time to peak | $=756 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=92,158 \mathrm{cuft}$ |
| Inflow hyd. No. | $=3-$ Post Development (To Pon\&Jax. Elevation | $=35.38 \mathrm{ft}$ |  |
| Reservoir name | $=$ Dry Detention Pond | Max. Storage | $=17,220 \mathrm{cuft}$ |

Storage Indication method used.


## Hydrograph Report

## Hyd. No. 8

Post Development (Detained)

| Hydrograph type | $=$ Combine | Peak discharge | $=13.33 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=25 \mathrm{yrs}$ | Time to peak | $=756 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=92,969 \mathrm{cuft}$ |
| Inflow hyds. | $=4,6$ | Contrib. drain. area | $=0.060 \mathrm{ac}$ |



## Hydrograph Report

## Hyd. No. 1

Pre-Developed Total

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=18.93 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=100 \mathrm{yrs}$ | Time to peak | $=742 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=102,007 \mathrm{cuft}$ |
| Drainage area | $=6.880 \mathrm{ac}$ | Curve number | $=67$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=8.00 \mathrm{in}$ | Distribution | $=\mathrm{Type} \mathrm{II}$ |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |

Pre-Developed Total
Hyd. No. 1 -- 100 Year


Hyd No. 1

## Hydrograph Report

## Hyd. No. 3

Post Development (To Pond)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=24.64 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=100 \mathrm{yrs}$ | Time to peak | $=740 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=132,272 \mathrm{cuft}$ |
| Drainage area | $=6.820$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=\mathrm{User}$ | Time of conc. $(\mathrm{Tc})$ | $=45.00 \mathrm{~min}$ |
| Total precip. | $=8.00$ in | Distribution | $=$ Type II |
| Storm duration | $=24 \mathrm{hrs}$ | Shape factor | $=484$ |



## Hydrograph Report

## Hyd. No. 4

Post Development (Bypass)

| Hydrograph type | $=$ SCS Runoff | Peak discharge | $=0.217 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=100$ yrs | Time to peak | $=740 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=1,164 \mathrm{cuft}$ |
| Drainage area | $=0.060$ ac | Curve number | $=78$ |
| Basin Slope | $=0.0 \%$ | Hydraulic length | $=0 \mathrm{ft}$ |
| Tc method | $=$ User | Time of conc. (Tc) | $=45.00 \mathrm{~min}$ |
| Total precip. | $=8.00$ in | Distribution | $=$ Type II |
| Storm duration | $=24$ hrs | Shape factor | $=484$ |

Post Development (Bypass)


## Hydrograph Report

## Hyd. No. 6

Pond Route

| Hydrograph type | $=$ Reservoir | Peak discharge | $=20.11 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=100 \mathrm{yrs}$ | Time to peak | $=754 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=132,271 \mathrm{cuft}$ |
| Inflow hyd. No. | $=3-$ Post Development (To PondJax. Elevation | $=35.79 \mathrm{ft}$ |  |
| Reservoir name | $=$ | Dry Detention Pond | Max. Storage |
|  |  |  |  |

Storage Indication method used.


## Hydrograph Report

## Hyd. No. 8

Post Development (Detained)

| Hydrograph type | $=$ Combine | Peak discharge | $=20.28 \mathrm{cfs}$ |
| :--- | :--- | :--- | :--- |
| Storm frequency | $=100 \mathrm{yrs}$ | Time to peak | $=754 \mathrm{~min}$ |
| Time interval | $=2 \mathrm{~min}$ | Hyd. volume | $=133,435 \mathrm{cuft}$ |
| Inflow hyds. | $=4,6$ | Contrib. drain. area | $=0.060 \mathrm{ac}$ |





## National Flood Hazard Layer FIRMette



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

|  | Without Base Flood Elevation (BFE) <br> Zone A, $V$, A99 <br> With BFE or Depth Zone AE, AO, AH, VE, AR |
| :--- | :--- |
| SPECIAL FLOOD |  |
| HAZARD AREAS |  |$\quad$| Regulatory Floodway |
| :--- |

Without Base Flood Elevation (BFE)
Zone $A, V$, A99 With BFE or Depth Zone AE, AO, AH, VE, AR Regulatory Floodway
of $1 \%$ annual chance flood with average depth less than one foot or with drainage eas of less than one square mile zone $X$

Future Conditions 1\% Annual
Area with Reduced Flood Risk due to Levee. See Notes. Zone $X$
no screen Area of Minimal Flood Hazard Zone $X$ THER AREAS
$\qquad$ Levee, Dike, or Floodwal

B $-\quad \begin{array}{r}\mathbf{2 0 . 2} \\ \text { 17.5 } \\ \text { Cross Sections with 1\% Annual Chance }\end{array}$ 17.5 Water Surface Elevation Coastal Transect mun 513 mm Base Flood Elevation Line (BFE) $工$ Limit of Study —— Jurisdiction Boundary -- --- Coastal Transect Baseline OTHER FEATURES $\qquad$ Profile Baseline
$\qquad$ Hydrographic Feature

MAP PANELS

## $\therefore$ Digital Data Available

 an authoritative property location.This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/18/2024 at 8:32 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

United States Department of Agriculture


Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Wake County, North Carolina


# Custom Soil Resource Report <br> Soil Map 



## MAP LEGEND

| Area of Interest (AOI) |  |
| :--- | :--- |
| $\square$ | Area of Interest (AOI) |
| Soils |  |
| $\square$ | Soil Map Unit Polygons |
| $\square$ | Soil Map Unit Lines |
| $\square$ | Soil Map Unit Points |

Special Point Features
(c) Blowout

B Borrow Pit
次 Clay Spot
$\diamond$ Closed Depression
Gravel Pit
$\therefore \quad$ Gravelly Spot
(4) Landfill
A. Lava Flow
A. Marsh or swamp
\& Mine or Quarry
(-) Miscellaneous Water

- Perennial Water
- Rock Outcrop
+ Saline Spot
$\because \quad$ Sandy Spot
을 Severely Eroded Spot
- Sinkhole

3) Slide or Slip
(6) Sodic Spot

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Wake County, North Carolina Survey Area Data: Version 25, Oct 2, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 24, 2022—May 9, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background magery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend 

| Map Unit Symbol |  |  | Map Unit Name |
| :--- | :--- | ---: | ---: |
| Ur | Urban land | Acres in AOI | Percent of AOI |
| Totals for Area of Interest |  | 1.1 | $100.0 \%$ |

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.
A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.
Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.
Soils that have profiles that are almost alike make up a soil series. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into soil phases. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.
Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.
A complex consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.
An undifferentiated group is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.
Some surveys include miscellaneous areas. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Wake County, North Carolina

Ur—Urban land<br>Map Unit Setting<br>National map unit symbol: 2qwpc<br>Elevation: 70 to 1,400 feet<br>Mean annual precipitation: 39 to 51 inches<br>Mean annual air temperature: 54 to 63 degrees F<br>Frost-free period: 190 to 250 days<br>Farmland classification: Not prime farmland<br>Map Unit Composition<br>Urban land: 100 percent<br>Estimates are based on observations, descriptions, and transects of the mapunit.<br>Description of Urban Land<br>Setting<br>Parent material: Impervious layers over human-transported material<br>Interpretive groups<br>Land capability classification (irrigated): None specified<br>Land capability classification (nonirrigated): 8<br>Hydric soil rating: No

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[^0]:    ${ }^{1}$ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
    Numbers in parenthesis are PF estimates at lower and upper bounds of the $90 \%$ confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is $5 \%$. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
    Please refer to NOAA Atlas 14 document for more information.

[^1]:    ${ }^{1}$ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
    Numbers in parenthesis are PF estimates at lower and upper bounds of the $90 \%$ confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is $5 \%$. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
    Please refer to NOAA Atlas 14 document for more information.

